



SALT LAKE COUNTY

**EASTSIDE
CANAL STUDY**

JORDAN AND SALT LAKE CANAL
EAST JORDAN CANAL
EAST JORDAN CANAL EXTENSION
UPPER CANAL

FINAL REPORT

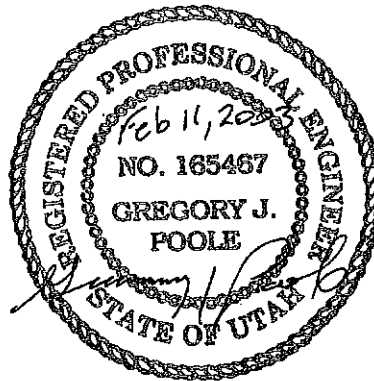
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Project Manager

Prepared by:

HANSEN, ALLEN & LUCE, INC.
Consulting Engineers
6771 South 900 East
Midvale, Utah 84047
(801)566-5599

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Salt Lake County

Neil Stack - Director of Engineering
Brent Beardall - Project Manager

City Representatives

Dennis Larkin - Holladay City Mayor
Wendell Rigby - Draper City Director of Public Works
Judith Bell - Sandy City Public Utilities Director
Keith Ludwig - Midvale City Engineer

Canal Representatives

Jeff Niermeyer - Salt Lake City Department of Public Utilities Deputy Director
Tim O'Hara - Salt Lake City Department of Public Utilities Canals Manager
Bill Marcoveccio - East Jordan Canal Company President
Richard Oldroyd - Upper Canal Company President

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EXECUTIVE SUMMARY

INTRODUCTION

The major canals on the east side of the Jordan River Valley (Salt Lake County) were constructed in the late 1800's and early 1900's to convey Utah Lake water from the Jordan Narrows to various irrigation systems. Today, urban development has increased the storm drainage use of the canals. At the same time, the actual demand for canal irrigation water has decreased as farms have been urbanized. Also, in recent years, canal capacity has been adversely affected by pond weed, bank instability, seepage, and encroachment.

Presently, the canals on the east side of the Jordan River Valley play key roles in the conveyance of storm runoff. The canals accept storm drainage and convey flows to overflow structures connected to County storm drain systems which convey flows to the Jordan River.

This study includes three major tasks: 1) Evaluation of the tributary storm runoff flows to the canals and creeks, 2) Evaluation of the existing canal capacities, and 3) Evaluation of alternatives for improving the storm drainage capacity of the canals. The following canal segments were evaluated as part of this study and will hereafter be referred to as the Eastside Canals:

- East Jordan Canal from 14600 South to 7000 South.
- East Jordan Canal Extension from 7000 South to Walker Lane.
- Jordan & Salt Lake Canal from 14600 South to 800 South.
(Hydraulic capacity was evaluated from 14600 South to 3300 South.
The canal is piped north of 3300 South).
- Upper Canal from Big Cottonwood Creek to Mill Creek.

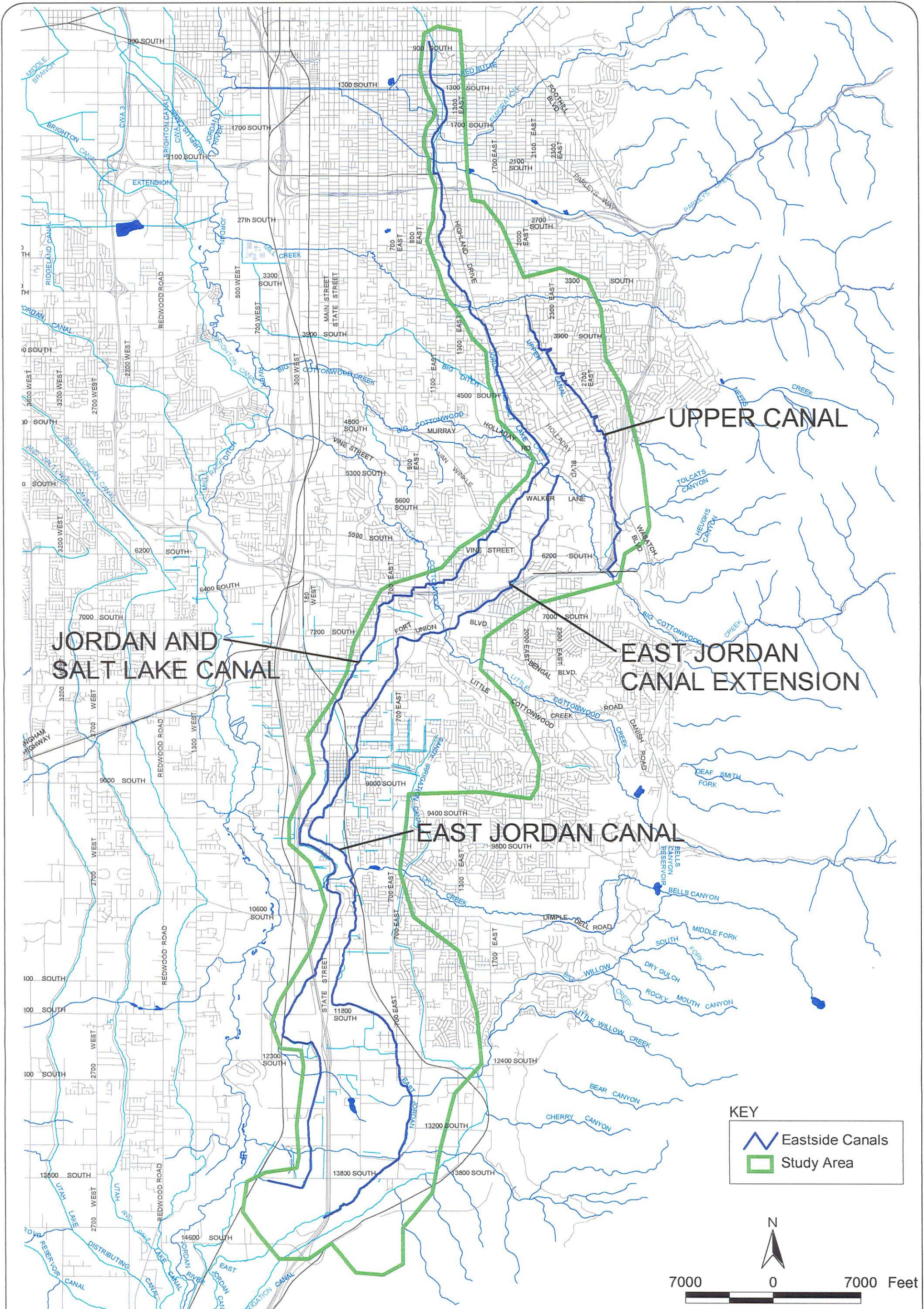
Figure ES-1 is a map of the study area.

STORM DRAINAGE MODEL

The study includes the evaluation of storm runoff from urban areas in Salt Lake County tributary to the above canal segments. To accomplish this, a computer model was developed to simulate runoff during a storm event in the urban areas of Salt Lake County east of the canals. The storm drainage model operates within ArcView GIS and uses the HEC-1 Army Corps of Engineers Flood Hydrograph Package for computing the storm runoff hydrographs.

STORM DRAINAGE CAPACITY

The study also includes the evaluation of the Eastside Canals ability to convey the storm drainage runoff calculated by the canal storm drainage model. The ability of the Eastside Canals to convey the storm drainage runoff is dependent on the canal base irrigation flows, canal capacity above base flow deliveries, and location of storm drain inflows and overflows along the canals. It





JORDAN AND SALT LAKE CANAL

UPPER CANAL

EAST JORDAN CANAL EXTENSION

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KEY

-  Eastside Canals
-  Study Area



was assumed in determining the storm drainage capacity of the Eastside Canals that the overflows release all excess storm water over the canal base flow amount. The U.S. Army Corps of Engineers Hydrologic Engineering Center - River Analysis System (HEC-RAS) software was used to calculate water surface profiles for each of the Eastside Canals. The water surface profiles were used to estimate the capacity of different reaches within the canals.

The models were vital tools in analyzing the storm drainage runoff to the canals and defining the storm drainage capacity of the canals. The models will allow Salt Lake County to continue to update the model, analyze potential drainage deficiencies, and facilitate the analysis of mitigation measures.

PREFERRED SOLUTION FOR IMPROVING STORM DRAINAGE CAPACITY

The study includes an evaluation of the canal storm drainage capacity inadequacies and development of a preferred solution for improving the storm drainage capacity of the Eastside Canals. The Eastside Canals storm drainage capacity improvement projects are presented in Table ES-1.

**TABLE ES-1
CONCEPTUAL COSTS FOR RECOMMENDED IMPROVEMENTS TO
THE EASTSIDE CANALS**

LOCATION	DESCRIPTION	CONCEPTUAL COST ¹
East Jordan Canal - 8720 South to 8750 South	Build up canal bank	\$8,000
East Jordan Canal - 6800 South	Build up canal bank	\$56,000
East Jordan Canal - 11900 South 625 East	Build up canal bank or install overflow at Van Winkle	\$22,000
Jordan and Salt Lake Canal - 5250 South	Build up canal bank	\$48,000
Jordan and Salt Lake Canal - 3900 South, Big and Little Cottonwood Creeks, 7200, 10300, and 12400 South	Modify overflows to provide capacity to discharge storm flow	\$100,000
Upper Canal - Downstream of 3660 S	Replace headgate with automatic overflow	\$30,000
TOTAL		\$264,000

1) Costs are in 2002 dollars.

RECOMMENDATIONS

The following is a summary of the recommendations presented in the Salt Lake County Eastside Canal Study:

1. It is recommended that the Eastside Canals Storm Drainage Capacity Improvement Projects be included in the Salt Lake County Flood Control Capital Improvement Plan.
2. It is recommended that a more detailed study of the Jordan and Salt Lake Canal overflow and outfall systems be completed.
3. The Sandy City Storm Drain Master Plan projects have a significant impact on the storm drain capacity in the East Jordan Canal and the Jordan and Salt Lake Canal. It is vital that these projects be implemented. It is recommended that Salt Lake County work with Sandy City in monitoring the progress of the projects.
4. It is recommended that Salt Lake County continue to work with canal owners to help ensure that the Eastside Canals are properly maintained and able to convey the storm drainage runoff discharged into the canals.
5. Model data should be updated as further land use, conveyance, capacity, and storm drainage inflows data become available.
6. It is recommended that the Eastside Canal Study be periodically reviewed and updated.
7. It is recommended that the impact of a proposed inflow into a canal be adequately analyzed to insure it will not create an increased risk of flooding before it is allowed.
8. It is recommended that a feasibility study be conducted to determine the cost and legal issues related to granting Salt Lake County recorded easements for the canals to allow Salt Lake County to protect rights-of-way from encroachment.
9. It is recommended that the County maintain an action plan to optimize the use of the Eastside Canals during a major storm event. For example, have a crew on-call who are trained to operate head gates and monitor key locations in anticipation of and during a major storm event.

CHAPTER I

PROJECT OVERVIEW

This chapter provides an overview of the Salt Lake County Eastside Canal Study. The following topics are presented in this chapter:

- Background
- Authority
- Study Activities
- Study Area

BACKGROUND

The major canals on the east side of the Jordan River Valley (Salt Lake County) were constructed in the late 1800's and early 1900's to convey Utah Lake water from the Jordan Narrows to various irrigation systems. Today, urban development has increased the storm drainage use of the canals. At the same time, the actual demand for canal irrigation water has decreased as farms have been urbanized. Also, in recent years, canal capacity has been adversely affected by pond weed, bank instability, seepage, and encroachment.

Presently, the canals on the east side of the Jordan River Valley play key roles in the conveyance of storm runoff. The canals accept storm drainage and convey flows to overflow structures connected to County storm drain systems which convey flows to the Jordan River.

This study includes three major tasks: 1) Evaluation of the tributary storm runoff flows to the canals and creeks, 2) Evaluation of the existing canal capacities, and 3) Evaluation of alternatives for improving the storm drainage capacity of the canals. The following canal segments were evaluated as part of this study and will hereafter be referred to as the Eastside Canals:

- East Jordan Canal from 14600 South to 7000 South.
- East Jordan Canal Extension from 7000 South to Walker Lane.
- Jordan & Salt Lake Canal from 14600 South to 800 South.
(Hydraulic capacity was evaluated from 14600 South to 3300 South.
The canal is piped north of 3300 South).
- Upper Canal from Big Cottonwood Creek to Mill Creek.

The study includes the evaluation of storm runoff from urban areas in Salt Lake County tributary to the above canal segments. To accomplish this, a computer model was developed to simulate runoff during a storm event in the urban areas of Salt Lake County east of the canals. The storm drainage model operates within ArcView GIS and uses the HEC-1 Army Corps of Engineers Flood Hydrograph Package for computing the storm runoff hydrographs.

The study also includes the evaluation of the Eastside Canals ability to convey the storm drainage runoff calculated by the canal storm drainage model. The ability of the Eastside Canals to convey the storm drainage runoff is dependent on the canal base irrigation flows, canal capacity above base flow deliveries, and location of storm drain inflows and overflows along the canals. The U.S. Army Corps of Engineers Hydrologic Engineering Center - River Analysis System (HEC-RAS) software was used to calculate water surface profiles for each of the Eastside Canals. The water surface profiles were used to estimate the capacity of different reaches within the canals.

Not only were the models vital tools in analyzing the storm drainage runoff to the canals and the storm drainage capacity of the canals for the Salt Lake County Eastside Canal Study, but the models will allow Salt Lake County to continue to update the model, analyze potential drainage deficiencies, and facilitate the analysis of mitigation measures.

AUTHORITY

Salt Lake County selected Hansen, Allen & Luce, Inc. (HAL) to assist them in preparation of an Eastside Canal Study. This study has been completed in accordance with an agreement between the Salt Lake County and HAL dated October 2000. Development of the Eastside Canal Study was completed under the direction of, and in cooperation with County staff.

STUDY ACTIVITIES

Specific tasks performed for this storm drainage study included:

Task 1 - Review of Existing Master Plans, Drainage Data, and Canal Data

Information contained in available reports and data was reviewed and summarized to make use of these prior studies to the extent possible. Study activities for this task included the following:

1. Met with County Staff to review project objectives and obtain available drainage studies and standards.
2. Reviewed available drainage studies and data and prepared a summary of the pertinent information available in each document.
3. Met with representatives of each canal company to review irrigation flow requirements and define design irrigation flow requirements.
4. Met with the cities of Draper, Sandy, Midvale, Holladay, and Salt Lake to present study objectives, review and obtain available data, and receive comments.

Task 2 - Data Gathering

Available canal and storm drainage system information was obtained. Study activities for this task included the following:

1. Used available storm drainage facilities mapping, available "as built", contour mapping and information summarized in Task 1 to prepare the existing facilities GIS coverages and mapping in ArcView GIS.
2. Available mapping and field reconnaissance were used to identify locations where field survey was needed to define canal cross sections and flow control.
3. Surveyed canal cross sections, bridges, overflows, and other identified flow controls.
4. Input survey data into ArcView GIS.
5. Reviewed canal data coverage with Salt Lake County staff.

Task 3 - Definition of Existing Canal Storm Drainage Capacity

Existing canal capacity was defined for the Jordan and Salt Lake Canal, East Jordan Canal, and Upper Canal.

1. Minimum freeboard requirements were selected with Salt Lake County staff.
2. HEC-RAS water surface profile models were prepared for the Eastside Canals.
3. Field reconnaissance was performed during the irrigation season to provide data for calibrating the HEC-RAS models.
4. The HEC-RAS models were calibrated based on field reconnaissance to define capacity available for storm drainage in each canal reach.
5. Predicted canal capacities and irrigation flow requirements were compared to define capacity available for storm drainage in each canal reach.
6. Met with Salt Lake County personnel to present results and receive comments.

Task 4 - Canal Storm Drainage Computer Model Development

A storm drainage computer model was prepared to define tributary storm drainage flowrates and areas of deficient canal capacity. Study activities for this task included the following:

1. Assisted the County in selection of the storm drainage model. The advantages and disadvantages of available storm drainage models were reviewed.
2. Using aerial photography mapping and contour data provided by the County, and drainage basin maps from previous studies; drainage basin and subbasin boundaries and flow paths were delineated.
3. Typical hydrologic characteristics by land use type were defined using data summarized in Task 1.
4. Met with County staff to review drainage basin boundaries and existing hydrologic characteristics. Subbasin boundaries were modified based on input from the County.

5. Using available storm drainage model data and mapping, hydrologic characteristics for each subbasin were developed.
6. Hydrologic characteristics were developed for all model features including conveyances, detentions, confluences, and diversions.
7. Runoff hydrographs at key locations were computed. The storm drainage model was calibrated qualitatively through evaluation with County personnel.
8. Predicted storm drainage flows and irrigation flow requirements were compared with canal capacities. Identified areas of deficiency.
9. Predicted creek flows were compared with creek master plans (Mill Creek, Big Cottonwood Creek, Little Cottonwood Creek, Willow Creek, and Corner Canyon Creek).

Task 5 - Canal Storm Drainage Plan

A canal storm drainage plan was prepared, including recommendations for immediate and future improvement recommendations. Study activities for this task included the following:

1. A workshop with County personnel was held to review modeling results and develop conceptual level solution alternatives.
2. The alternative plans were compared on the basis of conceptual level construction costs, maintenance requirements, and public acceptability.
3. A workshop was held with County personnel to evaluate the alternatives and select the preferred alternative.
4. The preferred drainage plan was refined and recommendations were prioritized.
5. Workshops were held with Salt Lake City, Midvale City, Sandy City, Draper City, East Jordan Canal Company, and Upper Canal Company.

Task 6 - Master Plan Document Preparation

A report was prepared documenting the capability and needs of using the Eastside Canals to convey storm drainage. Study activities for this task included the following:

1. A draft table of contents and outline was prepared. The draft outline was reviewed by County staff. Suggestions received from County staff were used to revise the outline as needed to meet County needs.
2. A draft study report document was prepared.
3. The draft document was presented to County staff for review.
4. Comments from County staff on the draft document were received and the Final Master Plan Document was prepared.

STUDY AREA

The study area includes the tributary area within Salt Lake County east of the Jordan and Salt Lake Canal between 14600 South and 800 South (see Figure ES-1). The study area includes portions of the following Cities: Draper, Sandy, Murray, Salt Lake, Holladay, South Salt Lake, and Midvale.

CHAPTER II

STORM DRAINAGE MODEL DEVELOPMENT

This chapter describes the methodology and process related to the storm drainage model for the Salt Lake County Eastside Canal Study. First, the hydrology and basin characteristics will be discussed followed by a description of the model.

HYDROLOGY AND BASIN CHARACTERISTICS

Design Storm

A design storm is a distribution of rainfall depths over a time increment for a given duration and frequency. Each of the following components of the design storm used in this study are described in this section: frequency, precipitation depth, duration and distribution.

Frequency

Salt Lake County staff selected the 10-year storm event for the storm drain analysis of the Eastside canals because the 10-year event is the design frequency used by the cities with storm drain system tributary to the Eastside Canals and is the generally accepted standard for storm drainage analysis along the Wasatch Front. The 10-year storm by definition has a 10 percent chance of being equaled or exceeded in any given year.

Applying the 100-year or greater storm event in the cities with storm drain systems tributary to the Eastside Canals is a more complex issue. In most of the newer developments, roadways are lower in elevation than adjacent lots which allow the roadways to carry the runoff that exceeds the capacity of the initial storm drainage system. However, the older sections of the County were developed around an existing open channel irrigation system where ditches along roadways deliver irrigation water to adjacent lots and agricultural land. The ditches along the roadways must be higher than the lots, so the roadways are higher in elevation than adjacent properties. Runoff that exceeds the capacity of the initial storm drainage system will collect in low areas between the homes and the roadways, and in some cases may flow through lots between homes. Determining the local storage in existing lots and identifying flow patterns for the 100-year storm event, is not feasible with the topographic information that is currently available.

Precipitation Depth

The Rainfall Intensity Duration Analysis, Salt Lake County, Utah, (TRC North American Weather Consultants and Meteorological Solutions Incorporated, 1999), was used to develop design rainstorm depths for the study area. The TRC report gives design rainfall depths for storms with durations of 1-hour and 6-hours. A 3-hour design storm developed by Salt Lake County was selected by County staff for this study. Rainfall depths for the 3-hour design storm were determined using the precipitation depths for the 1-hour and 6-hour duration storms from the TRC report and

a technique developed by the National Oceanic and Atmospheric Administration (NOAA, 1973). Precipitation depths tend to increase with elevation and with proximity to mountains. The 10-year, 3-hour rainfall depths range from 1.19 inches adjacent to the Jordan and Salt Lake Canal to 1.22 inches in the higher elevations of the tributary area (see Table I-1). One average design rainfall value of 1.21 inches was used for the 3-hour duration, 10-year frequency design storm for the entire study area. Table I-1 is a summary of the precipitation depths for the study area from the TRC report.

**TABLE II-1
PRECIPITATION DEPTHS**

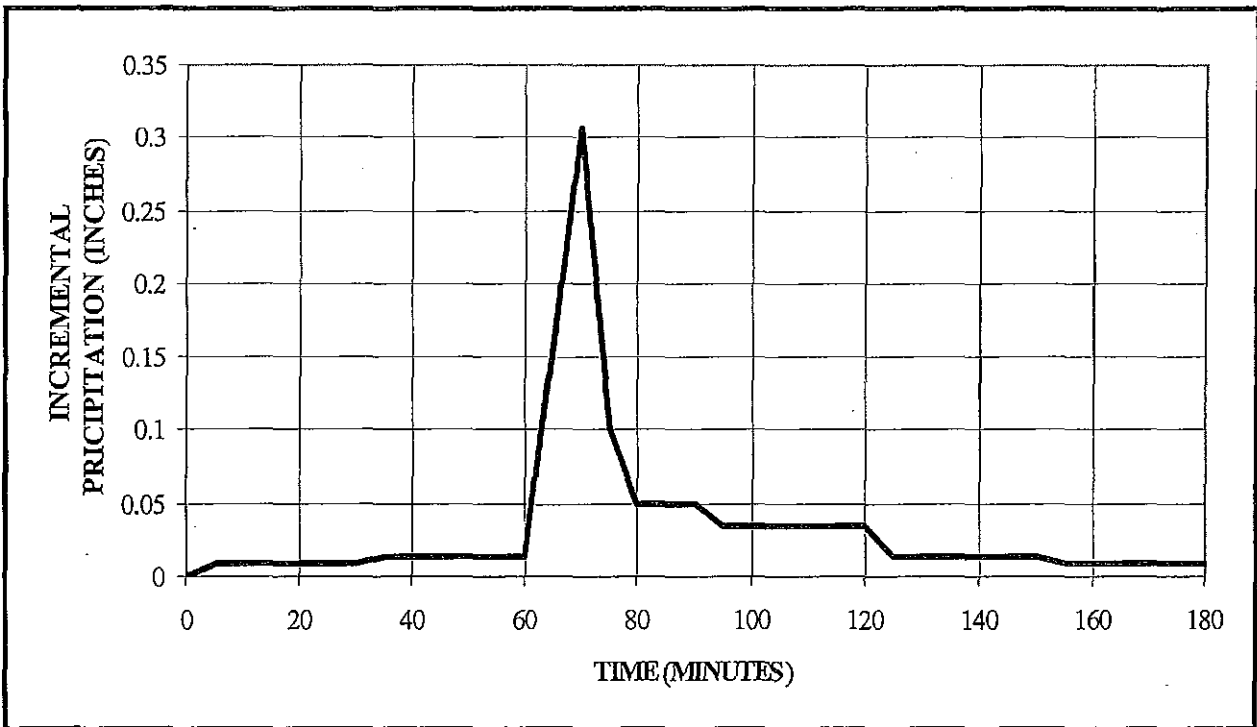
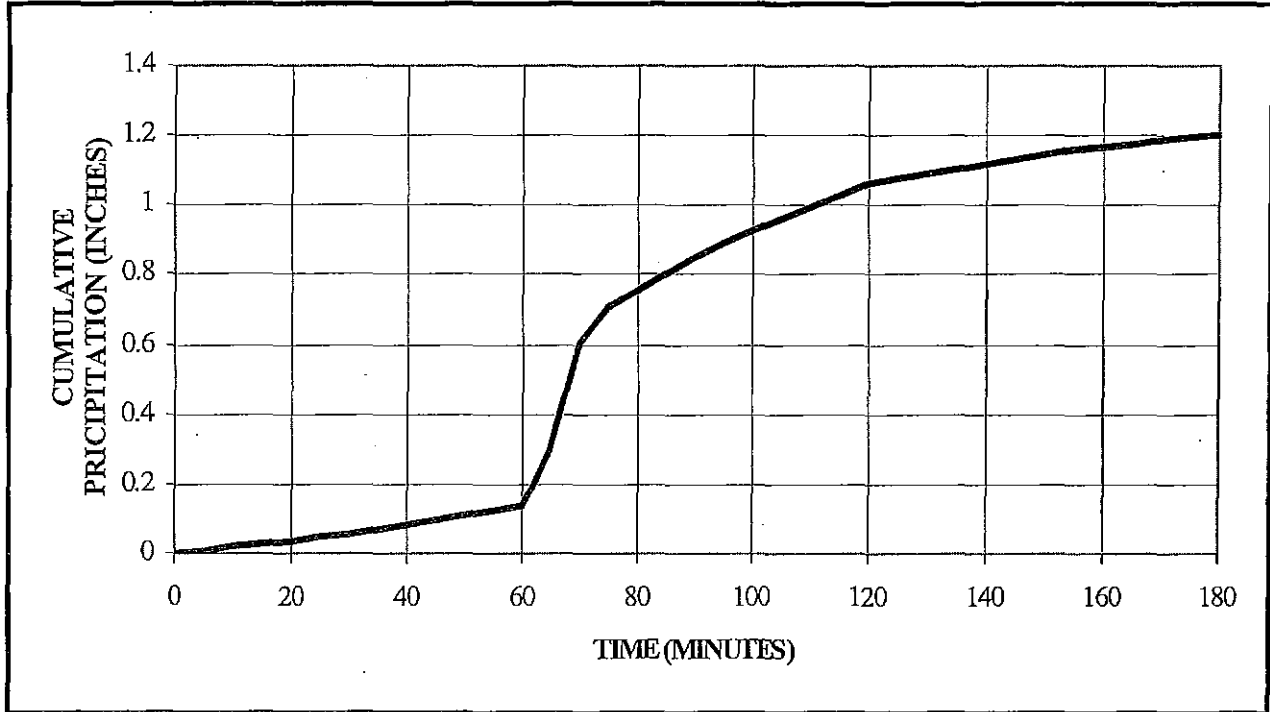
10-Year Storm Duration	Range of Precipitation in Study Area (inches)
1-Hour	0.88 to 0.92
3-Hour	1.19 to 1.22
6-Hour	1.40 to 1.50

Distribution and Duration

To compute runoff from a given storm, the distribution of the rainfall through time must be known. Critical runoff events from urban areas along the Wasatch Front are caused by cloudburst type storms, characterized by short periods of high intensity rainfall. During the 1960's and early 1970's, Dr. Eugene E. Farmer and Dr. Joel E. Fletcher completed a major study of the precipitation characteristics for storms in northern Utah. This effort has become the definitive source for rainfall distributions appropriate for the Wasatch Front area. In Davis County, Farmer and Fletcher (1971) examined rainfall gage records and classified storms based on whether the heaviest rainfall of the storm fell in the first, second, third, or fourth quarter of the storm period. Farmer and Fletcher found that "first and second quartile storms together comprise 76 percent of those storms containing a burst of 5-minute duration, with a 2-year recurrence interval and 92 percent of storms containing a burst of 10-minute duration, with a 10-year recurrence interval." Farmer and Fletcher developed model storms for first and second quartile storms.

The 3-hour design storm distribution developed by Salt Lake County that is based on the Farmer-Fletcher storm distribution was selected for this study. The 3-hour design storm duration utilizes a Farmer-Fletcher first quartile storm distribution for the central hour of the 3-hour distribution. The remaining two hours of the design storm distribution were distributed symmetrically around the central hour (see Figure II-1).

FIGURE II-1
SALT LAKE COUNTY 10-YEAR, 3-HOUR DESIGN STORM DISTRIBUTION



Drainage Basin Characteristics

A drainage basin is an area where rain or snow melt runoff within it will collect to a common point. Another name for a drainage basin is watershed or catchment. Subbasins are smaller drainage basins located within a larger drainage basin. Drainage subbasin boundaries depend upon both the topography and the location of storm drainage facilities. The drainage subbasin boundaries delineated for the storm drainage model are shown on Figure II-2.

Subbasins were delineated and subbasin characteristics were developed based on the following mapping and data sources:

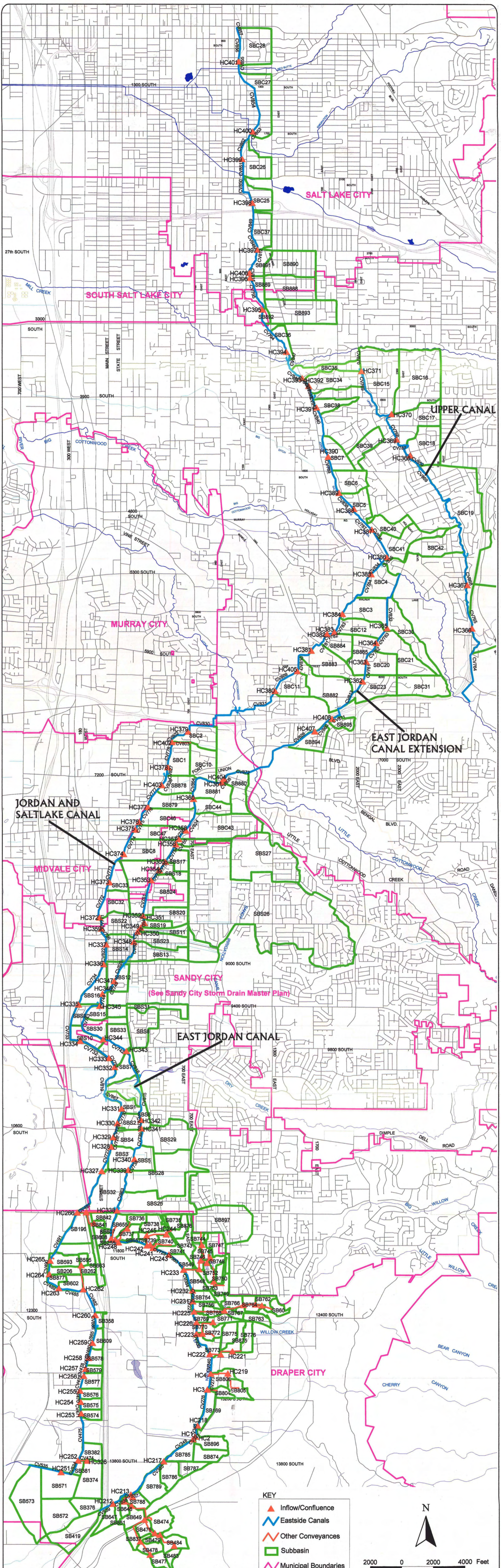
- Salt Lake County Storm Drainage Master Plan maps showing locations of storm drainage facilities.
- Digital base mapping of Salt Lake County consisting of physical features such as property lines, canals, and streets.
- Aerial photo and contour mapping of Salt Lake County
- The Sandy City Storm Drain Master Plan and HEC1 input files for areas in Sandy City tributary to the Eastside Canals (CH2MHill, 1998).
- The Draper City Storm Drainage Master Plan and Storm Drainage Model for areas in Draper City tributary to the Eastside Canals (HAL, 2001).
- "Soil Survey, Salt Lake County, Utah" (SCS, 1968).
- 7-½ Minute U.S. Geological Survey topographical maps.
- Curve number selection procedures provided by the Soils Conservation Service (SCS, 1972).
- Field reconnaissance.
- Discussions with County staff.

The subbasins characteristics for the area in Sandy City tributary to the Eastside Canals were not developed by HAL. Subbasin characteristics developed by CH2MHill for the Sandy City Storm Drain Master Plan were used directly in the storm drainage model developed for the Eastside Canal Study.

Subbasins for the remainder of the study area not in Sandy City were developed with the following characteristics:

- Subbasin area
- Hydrologic soil type
- Percentage of impervious area
- SCS curve number
- Conveyance characteristics

Hydrologic characteristics of each subbasin are given in the model output provided in Appendix A. Subbasin numbers are illustrated on the Figure II-2. Areas east of the canals not included in the



model indicate where storm drainage does not enter the canals but is conveyed directly west toward the Jordan River.

Subbasin Area

Subbasins were delineated within the GIS database using the mapping and data sources. Subbasins varied in size depending upon the level of development within the subbasin and the locations for which hydrographs were needed. Average subbasin size in developed areas was approximately 60-acres.

Hydrologic Soil Type

Hydrologic soil type is a general indication of the soil's infiltration capacity. Soils are assigned a hydrologic type of A, B, C, or D by the Natural Resource Conservation Service (NRCS). Soils of hydrologic soil type A have the highest infiltration rate, and therefore produce the least amount of runoff. Soils of hydrologic soil type D have the lowest infiltration rate, and therefore produce the highest amount of runoff. The soil type data were assigned to each subbasin. The calculation runoff is dependent on the soil hydrologic type.

Impervious Area

Impervious areas within each subbasins were estimated using the GIS model. The impervious area was divided into two components, directly connected impervious areas and unconnected impervious areas. Directly connected impervious areas provide a direct path for runoff from the impervious area to a conveyance such as a pipe, gutter, or channel. Directly connected impervious areas include roadways, parking lots, driveways, and sometimes the roofs of buildings. Runoff from unconnected impervious areas must cross a pervious area before reaching a conveyance. Examples of unconnected impervious areas include sidewalks that are not adjacent to the curb, patios, sheds, and usually some portion of the roof of a house.

Differentiation was made between directly connected and unconnected impervious areas. Runoff from directly connected impervious area reaches the drainage conveyance system quickly and usually determines the magnitude of the peak flow rate upstream from detention. Impervious areas such as back yard patios which drain to grassed or landscaped areas have much less impact on storm runoff peak flows. Based upon field observations, the directly contributing impervious area for a typical residential lot in Salt Lake County is assumed to include the driveway, and 50 percent of the home and garage area. It is assumed that runoff from the remaining 50 percent of the home and garage area flows over grassed areas before reaching the street. For large commercial structures, it was assumed that 100 percent of the roof area is directly connected impervious area.

SCS Curve Number

Each basin was assigned an SCS curve number. The curve number describes the relationship between precipitation and runoff for the pervious and unconnected impervious portions of the

subbasin. Curve numbers for each subbasin were estimated using a methodology presented by the Soil Conservation Service (SCS, 1972).

MODEL DESCRIPTION

The Salt Lake County Eastside Canal Study Storm Drainage Model is a combination of an ArcView GIS model and the Army Corps of Engineers (COE) "Flood Hydrograph Package (HEC-1)" linked by the Hydrologic Model Interface (HMI). HMI is a custom user interface developed by HAL to operate within the ArcView GIS environment. HMI provides an interactive link between the user, the GIS database, and HEC-1. A description of the model is contained in Appendix B.

The Eastside Canals Storm Drainage Model incorporates the Army Corps of Engineer's "Flood Hydrograph Package (HEC-1)" for calculation of runoff hydrographs. The kinematic wave modeling method for urban areas was used for all of the subbasins included in the model. The HEC-1 input file is written by HMI from the information within the GIS database.

Model Components

The Storm Drainage Model is comprised of five major components. Each of these model components is described below.

- **Subbasin Elements** - Subbasins are the basic elements for which runoff hydrographs are calculated. Subbasin elements represent a geographic area, and they are described by all of the hydrologic characteristics required by HEC-1 for calculation of a runoff hydrograph. Hydrologic characteristics of the subbasin elements are discussed in above.
- **Conveyance Elements** - Conveyance elements are used to represent routing of runoff through pipes, gutters, and channels. Conveyance elements are described by slope, length, hydraulic roughness, and cross section dimensions.
- **Confluence Elements** - Confluence elements are used to combine runoff hydrographs. Confluences are described by a single value which defines the number of hydrographs to be combined.
- **Diversion Elements** - Diversion elements are used to split runoff hydrographs. Diversions are described by a curve defining the flow of water being diverted for a given base flow. It is also defined by what hydrograph the diverted flow is being diverted to.
- **Detention Basin Elements** - Detention basin elements route runoff through a detention basin. Detention basin elements are described numerically by a stage-volume relationship, a stage discharge relationship, and an initial water level.

- **Connectivity Data** - Connectivity data defines the order in which the model elements are processed by HEC-1. Each model element is assigned a rank and the HEC-1 input file is written according to that rank. The order in which model elements are processed by HEC-1 establishes hydrograph routing and when hydrographs are combined.

Model Tools

All of the data associated with the model elements are contained within the GIS database. HMI accesses the GIS database, calculates hydrologic characteristics of the subbasins, and assembles the data to create input files for HEC-1. Input files are processed by HEC-1 to calculate the runoff hydrographs. Runoff hydrographs are calculated for subbasins, conveyances, confluences, and detention basins. HMI can then read the HEC-1 hydrograph files, identify peak flowrates, and write the hydrograph data to the GIS database. Some of the tools used to accomplish these tasks are as follows:

- **Connectivity Editor** - This tool allows the user to define hydrograph routing by assigning a rank to each of the model elements. The rank establishes the order that the element is written to the HEC-1 file. When a new element is inserted into the model, this tool automatically adjusts the rank of the subsequent elements.
- **Subbasin Analysis** - This tool uses information from the GIS database to calculate subbasin area, the percentages of connected and unconnected impervious area, and the representative SCS curve number for each subbasin.
- **HEC-1 Input File Writer** - The HEC-1 input file writer gathers the model data from the GIS database and writes the input file that is used by HEC-1 to calculate the runoff hydrographs.

Computation of Runoff Hydrographs

Hydrographs were computed for each subbasin, conveyance, confluence, detention basin inlet, and detention basin outlet. The maximum value from each hydrograph is the peak runoff flow rate. HMI determines the peak runoff flow rate for each hydrograph and adds the hydrographs to the GIS database.

Peak flows from the 10-year, 3-hour storm event are shown by reach for each Eastside Canal in Tables II-2, II-3, and II-4. Existing peak flows are flows without the Sandy City Storm Drain Master Plan Projects completed. The future peak flows are with the Sandy City Storm Drain Master Plan Projects completed. Model input and output is provided in Appendix A.

TABLE II-2
EAST JORDAN CANAL (INCLUDING EXTENSION)
10-YEAR, 3-HOUR STORM PEAK FLOW BY REACH

HMI ID	Upstream Station	Downstream Station	Existing 10-Year, 3-Hour Storm Peak Flow (cfs)	Future 10-Year, 3-Hour Storm Peak Flow (cfs) ¹
CV832	38+80	0+00	188.4	19.5
CV763	50+41	38+80	225.4	22.3
CV762	64+77	50+41	233.4	20.7
CV761	77+70	64+77	240.6	20.5
CV813	112+18	77+70	244.3	4.1
CV860	126+07	112+18	255.3	3.8
CV828	197+50	126+07	259.2	0.0
CV808	200+02	197+50	287.1	22.7
CV864	230+47	200+02	286.4	20.2
CV824	252+48	230+47	303.9	19.4
CV756	259+58	252+48	318.7	0.0
CV755	263+04	259+58	312.7	0.0
CV754	275+95	263+04	110.6	0.0
CV753	283+06	275+95	110.0	60.0
CV752	291+35	283+06	106.9	55.5
CV751	314+48	291+35	78.4	20.4
CV750	319+41	314+48	79.1	7.7
CV749	323+28	319+41	78.7	0.0
CV748	326+47	323+28	72.8	0.0
CV747	334+10	326+47	24.8	24.8
CV821	350+54	334+10	0.0	0.0
CV820	363+55	350+54	44.8	28.9
CV745	375+37	363+55	43.5	22.0
CV744	383+19	375+37	38.3	0.0
CV743	419+78	383+19	30.2	30.2
CV742	436+77	419+78	25.6	25.6
CV816	456+38	436+77	0.0	0.0
CV815	486+49	456+38	56.2	0.0

1. Future flows are less than existing flows because the implementation of the municipal storm drain master plans will decrease the flow of storm runoff to the canal.

TABLE II-2
EAST JORDAN CANAL (INCLUDING EXTENSION)
10-YEAR, 3-HOUR STORM PEAK FLOW BY REACH
(CONTINUED)

HMI ID	Upstream Station	Downstream Station	Existing 10-Year, 3-Hour Storm Peak Flow (cfs)	Future 10-Year, 3-Hour Storm Peak Flow (cfs) ¹
CV740	492+34	486+49	61.6	0.0
CV739	514+43	492+34	50.0	50.0
CV738	522+40	514+43	52.3	52.3
CV863	550+75	522+40	35.1	35.1
CV862	551+94	550+75	0.0	0.0
CV499	568+70	551+94	88.2	88.2
CV379	574+78	568+70	91.5	91.5
CV378	593+79	574+78	88.3	88.3
CV724	597+96	593+79	93.0	93.0
CV723	608+48	597+96	85.1	85.1
CV383	623+00	608+48	86.0	86.0
CV384	637+76	623+00	50.0	50.0
CV232	644+96	637+76	51.9	51.9
CV227	651+58	644+96	50.6	50.6
CV228	658+87	651+58	26.1	26.1
CV234	665+62	658+87	29.6	29.6
CV335	682+63	665+62	11.4	11.4
CV340	696+35	682+63	4.6	4.6
CV273	706+65	696+35	2.0	2.0
CV276	732+37	706+65	1.8	1.8
CV345	738+68	732+37	57.5	57.5
CV347	741+17	738+68	56.8	56.8
CV348	764+54	741+17	55.2	55.2
CV385	793+84	764+54	58.8	58.8
CV537	799+73	793+84	34.2	34.2
CV538	808+66	799+73	37.4	37.4
CV539	818+14	808+66	41.8	41.8

1. Future flows are less than existing flows because the implementation of the municipal storm drain master plans will decrease the flow of storm runoff to the canal.

**TABLE II-3
JORDAN AND SALT LAKE CANAL
10-YEAR, 3-HOUR STORM PEAK FLOW BY REACH**

HMI ID	Upstream Station	Downstream Station	Existing 10-Year, 3-Hour Storm Peak Flow (cfs)	Future 10-Year, 3-Hour Storm Peak Flow (cfs) ¹
CV794	17+48	0+00	3.4	3.4
CV847	24+62	17+48	0.0	0.0
CV846	39+00	24+62	14.4	14.4
CV845	44+93	39+00	12.0	12.0
CV844	46+71	44+93	22.5	22.5
CV843	54+77	46+71	0.0	0.0
CV842	61+28	54+77	22.3	22.3
CV840	94+61	61+28	22.4	22.4
CV789	118+83	94+61	21.3	21.3
CV838	132+85	118+83	20.4	20.4
CV787	150+81	132+85	18.7	18.7
CV836	171+81	150+81	16.0	16.0
CV835	176+75	171+81	0.0	0.0
CV834	186+27	176+75	37.1	37.1
CV784	219+42	186+27	37.6	37.6
CV783	232+93	219+42	40.7	40.7
CV782	237+91	232+93	39.4	39.4
CV781	254+00	237+91	30.5	30.5
CV810	273+55	254+00	21.3	21.3
CV809	293+32	273+55	8.0	8.0
CV831	324+91	293+32	0.0	0.0
CV830	368+50	324+91	17.9	17.9
CV802	382+24	368+50	17.3	17.3
CV778	398+66	382+24	8.8	8.8
CV827	404+82	398+66	0.0	0.0
CV826	410+95	404+82	50.2	32.1
CV777	428+08	410+95	50.6	29.7
CV776	439+42	428+08	51.5	25.3
CV775	444+85	439+42	51.8	18.1
CV774	462+25	444+85	51.2	9.0

1. Future flows are less than existing flows because the implementation of the municipal storm drain master plans will decrease the flow of storm runoff to the canal.

**TABLE II-3
JORDAN AND SALT LAKE CANAL
10-YEAR, 3-HOUR STORM PEAK FLOW BY REACH
(CONTINUED)**

HMI ID	Upstream Station	Downstream Station	Existing 10-Year, 3-Hour Storm Peak Flow (cfs)	Future 10-Year, 3-Hour Storm Peak Flow (cfs) ¹
CV773	483+61	462+25	52.4	28.6
CV772	507+26	483+61	53.1	28.8
CV771	514+68	507+26	56.2	28.8
CV736	525+71	514+68	36.2	19.7
CV822	538+82	525+71	20.1	0.0
CV734	569+76	538+82	11.1	0.0
CV733	599+07	569+76	10.8	10.8
CV732	620+12	599+07	7.3	7.3
CV757	628+16	620+12	0.3	0.3
CV819	650+90	628+16	0.0	0.0
CV867	664+96	650+90	38.7	10.4
CV730	674+90	664+96	38.0	7.3
CV729	684+60	674+90	36.5	18.7
CV728	691+09	684+60	35.0	16.0
CV727	707+80	691+09	32.9	11.7
CV497	740+15	707+78	29.0	0.0
CV491	776+06	740+15	20.2	20.2
CV490	784+83	776+06	19.7	19.7
CV489	797+72	784+83	17.6	17.6
CV485	814+17	797+72	14.8	14.8
CV482	833+04	814+17	1.6	1.6
CV866	841+70	833+04	0.0	0.0
CV865	861+03	841+70	77.2	77.1
CV480	861+03	851+04	77.8	77.8
CV479	868+09	861+03	78.0	78.0
CV478	872+70	868+09	79.0	79.0
CV472	882+40	872+70	79.8	79.8
CV470	889+66	882+40	82.1	82.1
CV471	897+18	889+66	85.2	85.2
CV475	928+85	897+18	89.1	89.1
CV534	944+82	928+85	75.0	75.0
CV535	965+64	944+82	41.0	41.1

1. Future flows are less than existing flows because the implementation of the municipal storm drain master plans will decrease the flow of storm runoff to the canal.

**TABLE II-4
UPPER CANAL
10-YEAR, 3-HOUR STORM PEAK FLOW BY REACH**

HMI ID	Upstream Station	Downstream Station	10-Year, 3-Hour Storm Peak Flow (cfs) ¹
CV870	17+48	0+00	3.4
CV769	24+62	17+48	0.0
CV768	39+00	24+62	14.4
CV767	44+93	39+00	12.0
CV859	46+71	44+93	22.5
CV859	54+77	46+71	0.0
CV869	61+28	54+77	22.3
CV868	94+61	61+28	22.4
CV765	118+83	94+61	21.3
CV764	132+85	118+83	20.4

1. The Upper Canal does not have separate existing and future peak flows because the tributary area is mostly developed and no master plan projects affect the Upper Canal.

CHAPTER III

STORM DRAINAGE CAPACITY

The Eastside Canals play a key role in conveying storm drainage water to overflows where the storm water can be diverted to the Jordan River. The ability of the canals to convey storm water is dependent on canal base flows (irrigation delivery requirements), canal capacity above base flow deliveries, and location of storm drain inflows and overflows along the canals.

HEC-RAS CANAL MODELS

The U.S. Army Corps of Engineers Hydrologic Engineering Center - River Analysis System (HEC-RAS) software can be used to perform one-dimensional steady and unsteady flow channel hydraulics calculations. This software was developed by the United States Government and is available for free download at the Internet site www.hec.usace.army.mil. The most recent version of HEC-RAS is version 3.0 which was released in January of 2001.

The HEC-RAS software was used to generate steady flow models of the Eastside canals. The canal models were used to calculate water surface profiles at given flow rates for each of the canals. The water surface profiles were used to estimate the capacity of different reaches within the canals.

HEC-RAS has the ability to calculate water surface profiles. The input data for a HEC-RAS model consists of geometric and flow data. Geometric data include reach lengths, loss/roughness coefficients, cross sections, and data for structures such as bridges, weirs, and culverts. Steady flow data include flow rates and flow change locations for different reaches along the canals. HEC-RAS has the ability to calculate water surface profiles for subcritical, supercritical, and mixed flow conditions using the energy equation, momentum equation, Yarnell equation, and WSPRO method. For a detailed description of the model methods and capabilities please refer to "HEC-RAS River Analysis System User's Manual", U.S. Army Corps of Engineers, Version 3.0, January 2001.

Model Data and Development

A HEC-RAS model was developed for the Eastside Canals. Data for the model were obtained by conducting a survey of each of the canals including cross sections and structures, contacting canal water masters and taking flow measurements to obtain flow rate inputs, and conducting field visits to sites along the canals to measure canal water surface elevation at structures for the calibration of the models. Each of these steps is described in more detail below.

Canal Surveys

A survey was conducted of the Eastside Canals by Robinson Biehn & Biehn Inc., Professional Land Surveyors, Salt Lake City, Utah. Each structure within the study area for each of the canals was surveyed along with cross sections at various locations along the canals. Cross section survey locations were selected by viewing aerial photos, contour mapping, and computer

generated cross sections along each of the canals and selecting locations that appeared to typify reaches of the canal. A survey report, including the survey data (cross section and structure survey points with location and elevation), canal structure sketches, and photographs of the canal structures was prepared for each canal. An AutoCAD file containing each of the survey points was also generated for each canal. Figures C-1 through C-3 of Appendix C are plan and profile drawings of the Eastside Canals.

Geometric Data

Each of the canal models were developed in the same manner. First, an ArcView Shape file of the canal centerline was imported into HEC-RAS. Next, AutoCAD files containing the centerline and survey data for each of the canals were generated. The AutoCAD drawings were used to determine the stationing and distance along the canals to each surveyed cross section and structure. The cross section and structure data were then input into the HEC-RAS model at the correct location along the canal.

A Manning's n value was assigned to each reach of the canal. An initial Manning's n value of 0.04, representing a canal with a clean bottom and brush on the sides, was used for each of the canals. This value was selected by observing canal conditions during a field visit to the canals, and by viewing photographs of the canals at canal structures.

Field Reconnaissance

Field reconnaissance was performed on August 16 and 17, 2001 in order to provide data for calibration of the HEC-RAS canal models. The East Jordan Canal was visited on August 16, 2001 and water level measurements were taken at various structures along the canal. A flow measurement was also taken in Draper at the old overflow to Corner Canyon Creek near the intersection of Stokes Avenue and Fort Street. The section of the Jordan and Salt Lake canal between 14600 South and I-15 was also visited on August 16, and water level measurements were taken at various structures along this section of the canal. On August 17, 2001, water level measurements were taken at various structures along the remainder of the Jordan and Salt Lake Canal, along the Upper Canal, and along the East Jordan Canal Extension. A summary of the field notes is included in Appendix D.

Flow Data

Flow data for the East Jordan Canal on the day of the field visit were obtained from the water master for the East Jordan Canal as 220 cfs at the diversion and 40 cfs at the East Jordan Canal Extension. During the field visit to the East Jordan Canal, the flow was measured at 138 cfs near the old Corner Canyon Creek overflow. For purposes of the model, a flow of 140 cfs was assumed from the beginning of the study at 14600 South to the old Corner Canyon Creek overflow. It was assumed based on information provided by the water master, that from the old Corner Canyon Creek overflow to the beginning of the East Jordan Canal Extension at about 7000 South, there was a linear decrease in flow from 138 cfs to 40 cfs in increments of about 2,000 feet along the canal.

Upon speaking with the water master for the Jordan and Salt Lake Canal it was determined that around the time of the field visit to the canal, flow was approximately 13 cfs. Upon speaking with the water master for the Upper Canal it was determined that around the time of the field visit to the Upper Canal, flow was also approximately 13 cfs.

Assumptions

The assumptions that were made in modeling the canals are as follows:

- Conveyance facilities downstream of lateral weirs and overflow structures are capable of conveying the full amount of overflow.
- All adjustable head gates and diversion structures are closed.
- The canals are free of major obstructions such as fallen trees, beaver dams, etc.
- Automatic overflow weirs are releasing all excess storm water over the canal base flow amount.
- Proposed Sandy City Storm Drain Master Plan improvements to the East Jordan Canal and Jordan and Salt Lake Canal are in place. (For example, new overflows and storm drain bypasses.)

Model Calibration

Calibration of the HEC-RAS canal models can be accomplished by entering measured water surface elevations into HEC-RAS for comparison to water surface profiles generated by the model. The model can be calibrated by adjusting the Manning's n roughness values until the model generated water surface profiles are close to the measured water surface elevations. Calibration information for each of the canal models is included below.

East Jordan Canal

The East Jordan Canal model was used to generate a water surface profile using the flow data as given in the previous section. This water surface profile was compared to the measured water surface elevations to verify the accuracy of the model. The model in most cases generated a water surface profile within 1 foot of the measured water surface elevations. If there appeared to be an excessive difference between the modeled and actual water surface elevation, the Manning's n values downstream of the area of interest were adjusted in order to calibrate the model. There were only a couple of areas along the canal where adjustments were made to the model.

Jordan and Salt Lake Canal

The Jordan and Salt Lake Canal was carrying very low flows during the summer of 2001 when water level measurements were taken. It was determined that due to the low flows and a beaver dam in the canal, an accurate calibration could not be accomplished. It was also determined that reasonable Manning's n values would be selected for the canal based on field observation and canal photographs. A Manning's n value of 0.04 was used for the canal which represents a canal

with a clean bottom and brush on the sides. This is the condition for the majority of the canal, however, there were some areas along the canal where pond weed was growing on the bottom of the canal. Due to the difficulties in associating a Manning's n value with the sections of the canal containing pond weed, and due to the fact that pond weed growth would probably be limited under higher flow conditions, it was assumed that all sections of the canal could be modeled with a Manning's n of 0.04.

Upper Canal

Similar to the Jordan and Salt Lake Canal, it was determined that due to low flows in the Upper Canal a calibration would not be performed, but the canal would be modeled using a value for Manning's n of 0.04.

Model Results

In order to evaluate the capacity of the canals, the canal models were used to generate water surface profiles at flows ranging from very low flows to high flows that exceeded the capacity of the canals. Canal reaches were selected for capacity evaluation based on storm drain inflow and overflow locations. It was assumed that the automatic overflows along the canal discharge all storm flow and that the water surface elevation was set at each overflow slightly above the overflow elevation. Limiting sections were selected for each canal reach by using the canal profile and cross section data to select the section within a reach that would be the first to overflow under high flow conditions. The results from HEC-RAS were used to obtain the capacity for each reach of the Eastside Canals at different amounts of freeboard. The maximum capacity was tabulated reach by reach for a freeboard of 0, 0.5, and 1 feet. After reviewing the results of the analysis with Salt Lake County it was determined that 1 feet of freeboard would be used to represent the capacity of the Eastside Canals. Canal sections with less freeboard than 0.5 feet would be considered problem areas. The canal capacity reach by reach at 0.5 feet of freeboard is presented for all three canals in Table III-1, Table III-2, and Table III-3. Figures C-1 through C-3 of Appendix C are plan and profile drawings of the Eastside Canals with water surface profiles from the model results.

Tables III-1 through III-3 show the capacity of each canal reach by reach. The capacity varies from one reach to the next and is based on a limiting cross section for each reach. Most portions of a reach will have a greater capacity than what is shown in the table, but the capacity of the limiting section is presented.

**TABLE III-1
EAST JORDAN CANAL (INCLUDING EXTENSION) TOTAL CAPACITY BY REACH**

Downstream Station ¹	Upstream Station ¹	South Ordinate		Capacity (cfs)	
		Downstream	Upstream	0.5 ft Freeboard	0 ft Freeboard
0+00	115+60	5450 ¹	6650	52	60
115+60	173+74	6650	7125	26	70
173+74	275+00	7125	8000	92	88 ← 113 cfs
275+00	350+54	8000	9000	51	75
350+54	456+38	9000	10250	115	140
456+38	550+75	10250	11400	300	330
550+75	732+37	11400	13520	175	220
732+37	865+53	13520	14600	225	260

1. HEC-RAS stationing is from downstream to upstream and the beginning of the study (Walker Lane) is at Station 19+41.

**TABLE III-2
JORDAN AND SALT LAKE CANAL TOTAL CAPACITY BY REACH**

Downstream Station ¹	Upstream Station ¹	South Ordinate		Capacity (cfs)	
		Downstream	Upstream	0.5 ft Freeboard	0 ft Freeboard
0+00	24+62	3345	Mill Creek	16	22
24+62	54+77	Mill Creek	3900	50	65
54+77	176+75	3900	Big Cottonwood	48	52
176+75	325+08	Big Cottonwood	Little Cottonwood	50	82
325+08	404+82	Little Cottonwood	7200	70	80
404+82	650+90	7200	10300	60	80
650+90	841+70	10300	12400	68	90
841+70	1032+10	12400	14600	160	200

1. HEC-RAS stationing is from downstream to upstream and begins at about 3345 South 1300 East.

**TABLE III-3
UPPER CANAL TOTAL CAPACITY BY REACH**

Downstream Station ¹	Upstream Station ¹	South Ordinate		Capacity (cfs)	
		Downstream	Upstream	0.5 ft Freeboard	0 ft Freeboard
0+00	16+60	3515	3660	4.5	9.5
16+60	107+15	3660	4500	60	80
107+15	282+61	4500	6200	45	62

1. HEC-RAS stationing is from downstream to upstream and begins at about 3515 South 2000 East.

CANAL BASE FLOWS

Canal base flows are very important in determining canal storm drainage capacity. The amount of storm drainage water that can be conveyed by the canal can be calculated by subtracting the canal base flow from the maximum capacity of the canal reach by reach. Canal base flows were determined by reviewing "Salt Lake City Corporation, Department of Public Utilities, Jordan & Salt Lake Canal and East Jordan Canal Study", Hansen, Allen, & Luce, Inc., Salt Lake City, Utah, November 1996, by contacting the water master or canal manager for each of the canals to discuss base flows, and by using the canal models to predict base flows based on overflow elevations. These flows are normal current operating base flows. Base flows to satisfy all water rights would be greater than these flows. The canal base flow data for each of the canals is presented in Table III-4 below.

**TABLE III-4
NORMAL IRRIGATION BASE FLOWS FOR THE EASTSIDE CANALS**

Canal	Reach	Base Flow (cfs)
East Jordan ¹	14600 South - 13520 South	140
	13520 South - 11400 South	130
	11400 South - 9000 South	63
	9000 South - 7125 South	40
	7125 South - 6200 South	26
	6200 South - Walker Lane	13
Jordan and Salt Lake ²	14600 South - Mill Creek	30
	Mill Creek - 3345 South	20
Upper Canal ³	Beginning - 3660 South	13
	3660 South - 3515 South	4.5

1. East Jordan Canal base flows were taken from the canal model as flows due to overflow settings, and were verified by the Salt Lake City Department of Public Utilities Canal Manager.
2. The Jordan and Salt Lake Canal base flow was determined by reviewing previous year flows in the canal and consulting with the Salt Lake City Department of Public Utilities Canal Manager.
3. The Upper Canal base flow was determined upon discussion with the canal Water Master and was assumed to be at the capacity of the canal downstream of 3660 South.

Table III-4 shows that base flows in the East Jordan canal range from 13 cfs in the lower portion of the East Jordan Canal Extension to 140 cfs at 14600 South. The Jordan and Salt Lake canal is shown to have a relatively consistent base flow along the length of the canal of about 30 cfs, and the Upper Canal base flow was assumed to be 4.5 cfs in the downstream section of the canal to about 3660 South and 13 cfs in the rest of the canal. The exact base flow is unknown in the lower section of the Upper Canal due to the use of an adjustable head gate at 3660 South.

STORM DRAINAGE CAPACITY

Each of the canal base flows as presented in Table III-4 can be subtracted from the total canal capacities of the canals from Tables III-1 through III-3 to determine the canal storm drainage capacity. The available storm drainage capacity for each canal is presented in Tables III-5 through III-7 below.

TABLE III-5
EAST JORDAN CANAL (INCLUDING EXTENSION)
STORM DRAINAGE CAPACITY WITH 0.5 FOOT FREEBOARD BY REACH

0 Free board
↓

Downstream Station ¹	Upstream Station ¹	South Ordinate		Storm Drainage Capacity (cfs)	
		Downstream	Upstream		
0+00	115+60	5450 ¹	6650	60-52 +8 26 x 8	34
115+60	173+74	6650	7125	0 x 31	34
173+74	275+00	7125	8000	52 x 19	71
275+00	350+54	8000	9000	11 x 24	39
350+54	456+38	9000	10250	52 x 25	77
456+38	550+75	10250	11400	237 x 30	267
550+75	732+37	11400	13520	45 x 45	90
732+37	865+53	13520	14600	85 x 35	120

1. HEC-RAS stationing is from downstream to upstream and the beginning of the study (Walker Lane) is at Station 19+41.

TABLE III-6
JORDAN AND SALT LAKE CANAL
STORM DRAINAGE CAPACITY WITH 0.5 FOOT FREEBOARD BY REACH

Downstream Station ¹	Upstream Station ¹	South Ordinate		Storm Drainage Capacity (cfs)	
		Downstream	Upstream		
0+00	24+62	3345	Mill Creek	0 x 6	6
24+62	54+77	Mill Creek	3900	20 x 15	35
54+77	176+75	3900	Big Cottonwood	18 x 4	22
176+75	325+08	Big Cottonwood	Little Cottonwood	20 x 32	52
325+08	404+82	Little Cottonwood	7200	40 x 10	50
404+82	650+90	7200	10300	30 x 20	50
650+90	841+70	10300	12400	38 x 22	60
841+70	1032+10	12400	14600	130 x 40	170

1. HEC-RAS stationing is from downstream to upstream and begins at about 3345 South 1300 East.

**TABLE III-7
UPPER CANAL STORM
STORM DRAINAGE CAPACITY WITH 0.5 FOOT FREEBOARD BY REACH**

Downstream Station ¹	Upstream Station ¹	South Ordinate		Storm Drainage Capacity (cfs)
		Downstream	Upstream	
0+00	16+60	3515	3660	0 - 5 5
16+60	107+15	3660	4500	47 - 10 57
107+15	282+61	4500	6200	32 - 17 49

1. HEC-RAS stationing is from downstream to upstream and begins at about 3515 South 2000 East.

The calculated canal storm drainage capacity can then be compared to calculated storm drainage inflows to determine where the canals have insufficient capacity to convey expected storm drainage water. Areas along the canals that were identified as having insufficient capacity to convey storm water are presented in Chapter IV of this report.

CANAL OVERFLOW EVALUATION

One of the major assumptions that was made in the evaluation of the canals was that all automatic overflows will discharge all of the upstream storm flows and the flow downstream of the overflows will be reduced to the canal base flow. An evaluation of the existing overflows on the canals was conducted to determine if the current overflows are sufficient. The results of the overflow evaluation are presented in Table III-8 on the following page. Table III-8 also includes overflows that are proposed as a part of the "Storm Drain Master Plan" for Sandy City.

Existing Overflows

Most of the existing overflows on the East Jordan Canal are adequately sized to discharge storm water (See Table III-8). The overflow at 13520 South is the new Corner Canyon Creek overflow. The evaluation of the overflow showed that at the current overflow elevation of the overflow it would need to be about 30 feet wide to discharge all of the storm flow coming into the canal upstream. The East Jordan Canal downstream of the new overflow, however, appears to have sufficient capacity to convey the 10-year, 3-hour storm flows which carry over past the existing 18-foot overflow.

Due to a change in irrigation delivery/base flow in the Jordan and Salt Lake Canal, a preliminary evaluation showed that all of the overflows except the flume at Mill Creek will need to be modified to discharge upstream storm flows. The overflow elevation of the overflows along the Jordan and Salt Lake Canal is generally higher than the base flow water surface elevation. This is due to the decrease in flow carried by the Jordan and Salt Lake Canal in recent years. An in-depth study of how the overflows will need to be modified to discharge upstream storm flows is beyond

**TABLE III-8
EASTSIDE CANALS OVERFLOW EVALUATION**

Station	Location	Overflow Elevation	Base Flow Water Surface Elevation	Weir Width	Calibration Base Flow (cfs)	Upstream Storm Flow (cfs)	Sufficient	Recommendations/Comments
EAST JORDAN CANAL (INCLUDING EXTENSION)								
115+60	6650 South	4451.11	4450.38	8	26	4	yes	
173+74	Little Cottonwood Cr.	4451.60	4452.42	6.5	36	22	yes	Adjustable by check boards.
275+95	8000 South		4456.84		52	60	-	Sandy City Master Plan proposed overflow.
333+00	8760 South		4459.28		63	25	-	Sandy City Master Plan proposed overflow.
350+54	9000 South	4458.76	4460.04	16	67	29	yes	
383+19	9400 South		4461.74		74	30	-	Sandy City Master Plan proposed overflow.
456+38	10250 South	4465.92	4465.39	12	85	0	yes	
493+17	10600 South		4466.81		93	50	-	Sandy City Master Plan proposed overflow changed to a bypass.
550+75	11400 South	4466.05	4467.82	20	104	88	yes	Sandy Master Plan proposed bypass.
732+37	13520 South	4474.96	4475.36	18	138	58	no	Corner Canyon Creek overflow. Required width = 30 ft.
738+90	13550 South	4475.94	4475.49	6	138	NA	NA	Old overflow, water surface generally below overflow.
JORDAN AND SALT LAKE CANAL								
24+60	Mill Creek	4385.37	4385.47	85	20	14	yes	
54+77	3900 South	4390	4389.97	4	30	22	no	Modify overflow to provide capacity to discharge storm flow.
176+75	Big Cottonwood Cr.	4398.77	4398.36	7.5	30	37	no	Modify overflow to provide capacity to discharge storm flow.
324+90	Little Cottonwood Cr.	4402.58	4402.24	16	30	18	no	Modify overflow to provide capacity to discharge storm flow.
404+82	7200 South	4404.88	4404.28	9.5	30	32	no	Modify overflow to provide capacity to discharge storm flow.
469+10	8000 South		4407.07		30	28/0	-	Sandy City Master Plan proposed overflow.
511+02	8600 South		4407.72		30	28	-	Sandy City Master Plan proposed overflow.
569+76	9400 South		4409.88		30	11	-	Sandy City Master Plan proposed overflow.
650+90	10300 South	4412.73	4412.48	19	30	10	no	Modify overflow to provide capacity to discharge storm flow.
679+14	10600 South		4412.85		30	11	-	Sandy City Master Plan proposed overflow changed to a bypass.
740+15	11400 South		4414.44		30	20	-	Sandy City Master Plan proposed overflow changed to a bypass.
841+70	12400 South	4417.31	4417.40	6	30	77	no	Modify overflow to provide capacity to discharge storm flow
UPPER CANAL								
16+49	3660 South	4542.19	4542.25	4	13	4	no	Replace headgate with an automatic overflow
107+09	4500 South	4560.62	4560.50	17	13	21	yes	

the scope of this study. It is likely that the concrete overflows on the overflows will need to be saw-cut to lower the overflow elevations to the base flow water surface elevation.

The overflow at 4500 South on the Upper Canal is adequately sized to discharge storm water. The overflow located at 3660 South, however, is not adequately designed to automatically discharge excess water. A manual gate has to be opened to regulate the amount of water diverted out of the canal through the overflow. It is recommended that the headgate on the overflow be replaced with an automatic overflow to reduce the possibility of flooding downstream.

Proposed Overflows

As a part of the Sandy City "Storm Drain Master Plan", prepared by CH2MHILL, August 1998, several overflows and storm drain bypasses were proposed on the East Jordan and Jordan and Salt Lake Canals. The proposed overflows are included in Table III-8. An evaluation of the canals was conducted without the Sandy City improvements included in the model. The result was extensive flooding along the canals in the Sandy City area and downstream. Upon meeting with Sandy City staff it was determined that work is proceeding on the 8000 South and 8600 South overflows and the 9400 South overflow will be completed in the future. The overflow at 10250 South (Dry Creek) was recently replaced on the East Jordan Canal. Also, Sandy City indicated they do not plan to install the proposed overflows at 11400 South and 10600 South on the Jordan and Salt Lake Canal and the proposed overflow at 10600 South on the East Jordan Canal. Storm drain pipes that bypass the canals are planned at these three proposed overflow locations, but overflows are not. It was assumed in the models for the Eastside Canals that the proposed overflow locations at 11400 South and 10600 South on the Jordan and Salt Lake Canal and the proposed overflow location at 10600 South on the East Jordan Canal will be bypasses and not overflows.

CHAPTER IV

STORM DRAINAGE CAPACITY INADEQUACIES

The storm drainage inadequacies identified by the storm drainage and surface profile models are described in this chapter. The Eastside Canals were evaluated assuming that improvements suggested as part of the Sandy City Storm Drain Master Plan are implemented (except for the proposed overflows at 11400 South and 10600 South on the Jordan and Salt Lake Canal and 10600 South on the East Jordan Canal which are evaluated as bypasses consistent with current Sandy City planning). Improvements included a number of storm drainage bypasses and new overflows through the Sandy City area (see Table IV-1). Because the Sandy City Storm Drain Master Plan projects have a significant impact on the storm drain capacity in the East Jordan Canal and the Jordan and Salt Lake Canal, it is recommended that the County monitor the progress of Sandy City in implementing the projects. The Sandy City Storm Drain Master Plan improvements do not affect the Upper Canal. The Eastside Canals were evaluated assuming that improvements to overflows identified in Table III-8 are implemented. Without the Sandy City Storm Drain Master Plan improvements and overflow modifications, the East Jordan Canal and Jordan and Salt Lake Canal have many additional and more severe problem areas than the ones identified in this chapter.

**TABLE IV-1
SANDY CITY STORM DRAIN MASTER PLAN PROJECTS ASSOCIATED WITH THE
EASTSIDE CANALS**

PROJECT NUMBER ¹	PROJECT DESCRIPTION	ESTIMATED COST ¹
80S-P1(A-B)	8000 South Outfall from the East Jordan (EJ) Canal overflow to the Jordan and Salt Lake (JSL) Canal overflow to the Jordan River	\$4,017,600
86-P1(A-B)	8600 South Outfall from the JSL Canal overflow to the Jordan River	\$2,325,900
86S-P3	8760 South Outfall from the EJ Canal overflow to the JSL Canal	\$423,100
94S-P3	9400 South Outfall from the EJ Canal overflow to the Jordan River	\$354,100
94-P1(A-B)	9400 South Outfall from the JSL Canal overflow to the JSL Canal	\$1,103,700
106S-P1	10600 South Outfall from I-15 to the Jordan River	\$891,800
106S-P3	10600 South Outfall from the JSL Canal to I-15 (JSL Canal overflow not needed)	\$565,400

**TABLE IV-1
SANDY CITY STORM DRAIN MASTER PLAN PROJECTS ASSOCIATED WITH THE
EASTSIDE CANALS
(CONTINUED)**

PROJECT NUMBER ¹	PROJECT DESCRIPTION	ESTIMATED COST ¹
106S-P4	10600 South Outfall from the EJ Canal to I-15 (EJ Canal overflow not needed)	\$393,500
114S-P2	11400 South Outfall Casing Under I-15	\$609,900
114S-P1	11400 South Outfall from I-15 to Willow Creek (JSL Canal overflow not needed)	\$781,200
114S-P3	11400 South Outfall from the EJ Canal overflow to the Union Pacific Railroad	\$601,800
		\$12,068,000

1) Project Number and Estimated Costs are from Table 6-1 of Sandy City Storm Drain Master Plan prepared in 1998 by CH2MHill

Each identified problem area is described in the following paragraphs. Recommendations and preferred solutions for each of the inadequacies are discussed in Chapter V.

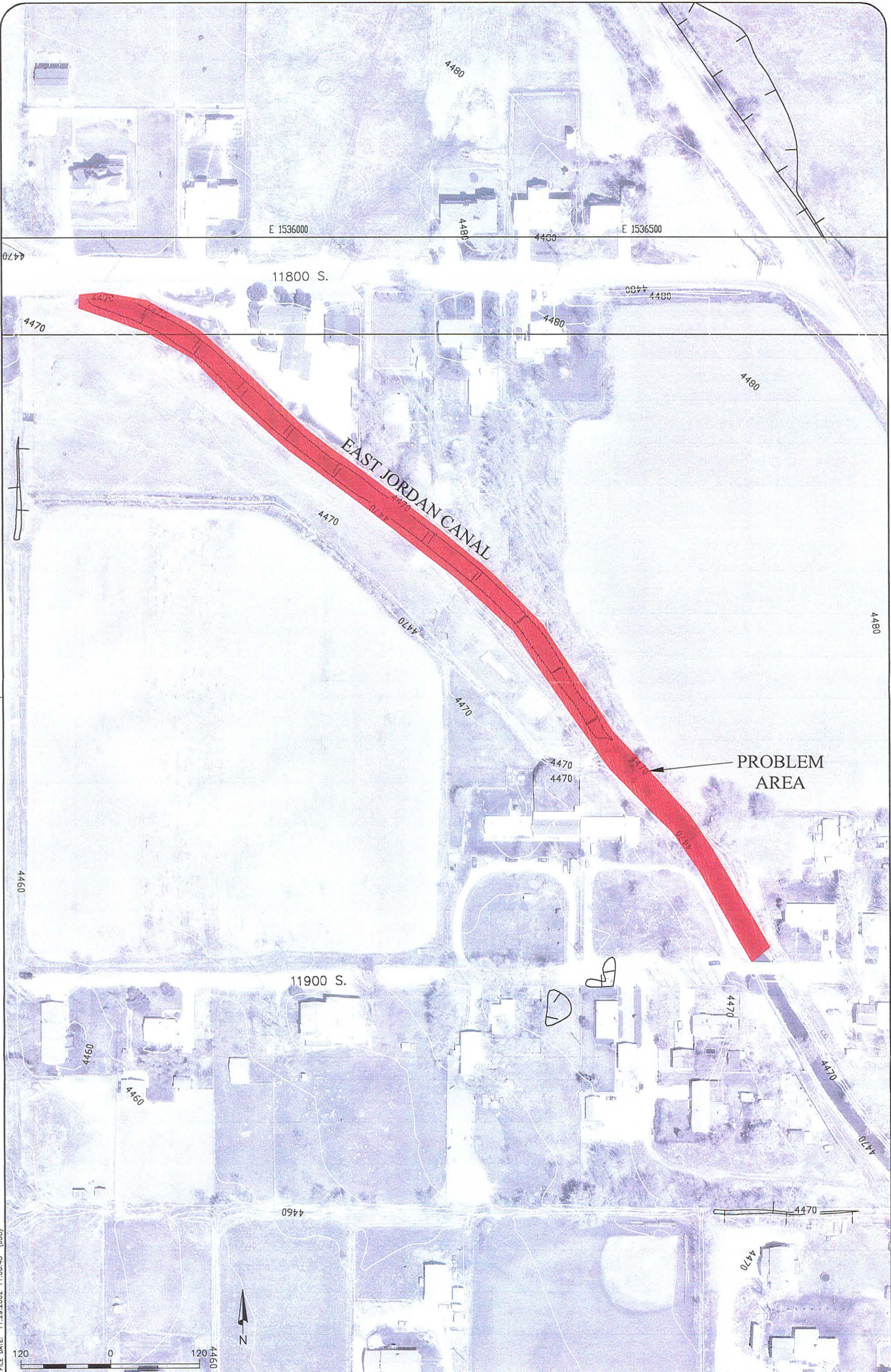
EAST JORDAN CANAL

11900 South 625 East

Downstream of the bridge at 11900 South, a limiting section was identified which has insufficient capacity for the 10-year, 3-hour storm runoff event (see Figure IV-1). The flow in this section with 10 year storm flow on top of base flow is about 216 cfs (130 cfs base and 86 cfs storm flow). The capacity through this section with 0.5 feet of freeboard is 175 cfs which is exceeded by 41 cfs during a 10-year, 3-hour storm. If the water were to overflow the downhill bank in this area, it appears that some driveways would be flooded and additional flooding could result downstream.

8720 South to 8750 South

The canal has a low east bank between the bridges at 8750 South and 8720 South (see Figure IV-2). A feed store is located on the bank at the bridge at 8750 South. City streets and a home are located just beyond this east bank. The total flow through this reach of the canal during a 10-year, 3-hour storm is about 65 cfs (40 cfs base and 25 cfs storm flow). The capacity of the canal through this section with 0.5 feet of freeboard is about 50 cfs.



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SALT LAKE COUNTY EASTSIDE CANAL STUDY - EAST JORDAN CANAL
STORM DRAINAGE PROBLEM - 11900 SOUTH 625 EAST

FIGURE
IV-1



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SALT LAKE COUNTY EASTSIDE CANAL STUDY - EAST JORDAN CANAL
 STORM DRAINAGE CAPACITY PROBLEM - 8720 SOUTH TO 8750 SOUTH

FIGURE
 IV-2

6800 South

The canal upstream and a short distance downstream of the foot bridge at 6800 South has a low east bank with a school, school playground, and a shed behind the bank (see Figure IV-3). The capacity of the canal at this section (based on the low east bank) is about 26 cfs at 0.5 feet of freeboard. The total flow in the canal through this reach under 10-year, 3-hour storm conditions is slightly above 26 cfs. The capacity of the canal using the west bank elevation is much higher.

JORDAN AND SALT LAKE CANAL

Overflows

The overflow elevation of the overflows along the Jordan and Salt Lake Canal is generally higher than the base flow water surface elevation. This is because flow carried by the Jordan and Salt Lake Canal has decreased in recent years. The following overflows on the Jordan and Salt Lake Canal do not allow the release of excess water over the canal base flow: 3900 South, Big Cottonwood Creek, Little Cottonwood Creek, 7200 South, 10300 South, and 12400 South.

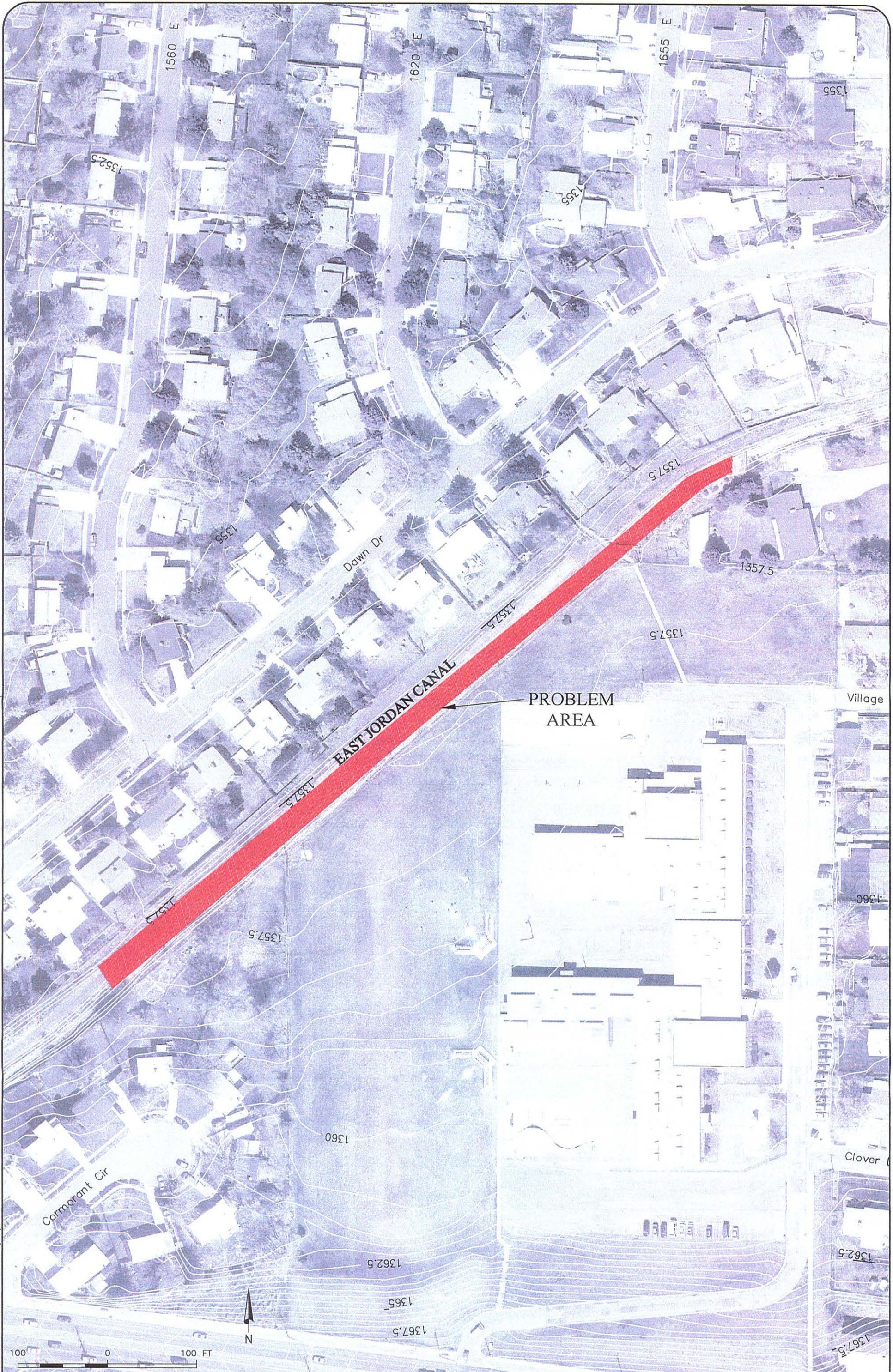
5250 South

The canal upstream of the bridge at 5250 South has a capacity with 0.5 feet of freeboard of 48 cfs (see Figure IV-4). The total flow in this reach of the canal during a 10-year, 3-hour storm is about 68 cfs (30 cfs base and 38 cfs storm flow). If the banks were overtopped at this section of the canal, water would flood a home.

UPPER CANAL

Downstream of 3660 South

The model of the 10-year, 3-hour storm flow on top of a base flow of 13 cfs indicates problems with flooding downstream of the culvert at 3660 South. A large headgate where water can be turned out is located at 3660 South. The water that is not turned out of the canal continues through a culvert across the intersection. Downstream of the intersection the canal turns into a small concrete lined roadside ditch. The model indicates flooding problems along the ditch downstream if the headgate at 3660 South is closed. The total capacity of the ditch downstream of the culvert is about 4.5 cfs to 9.5 cfs with 0.5 feet of freeboard. Predicted flow during a 10-year, 3-hour storm event upstream from the overflow would be about 17 cfs (13 cfs base flow and 4 cfs storm flow).

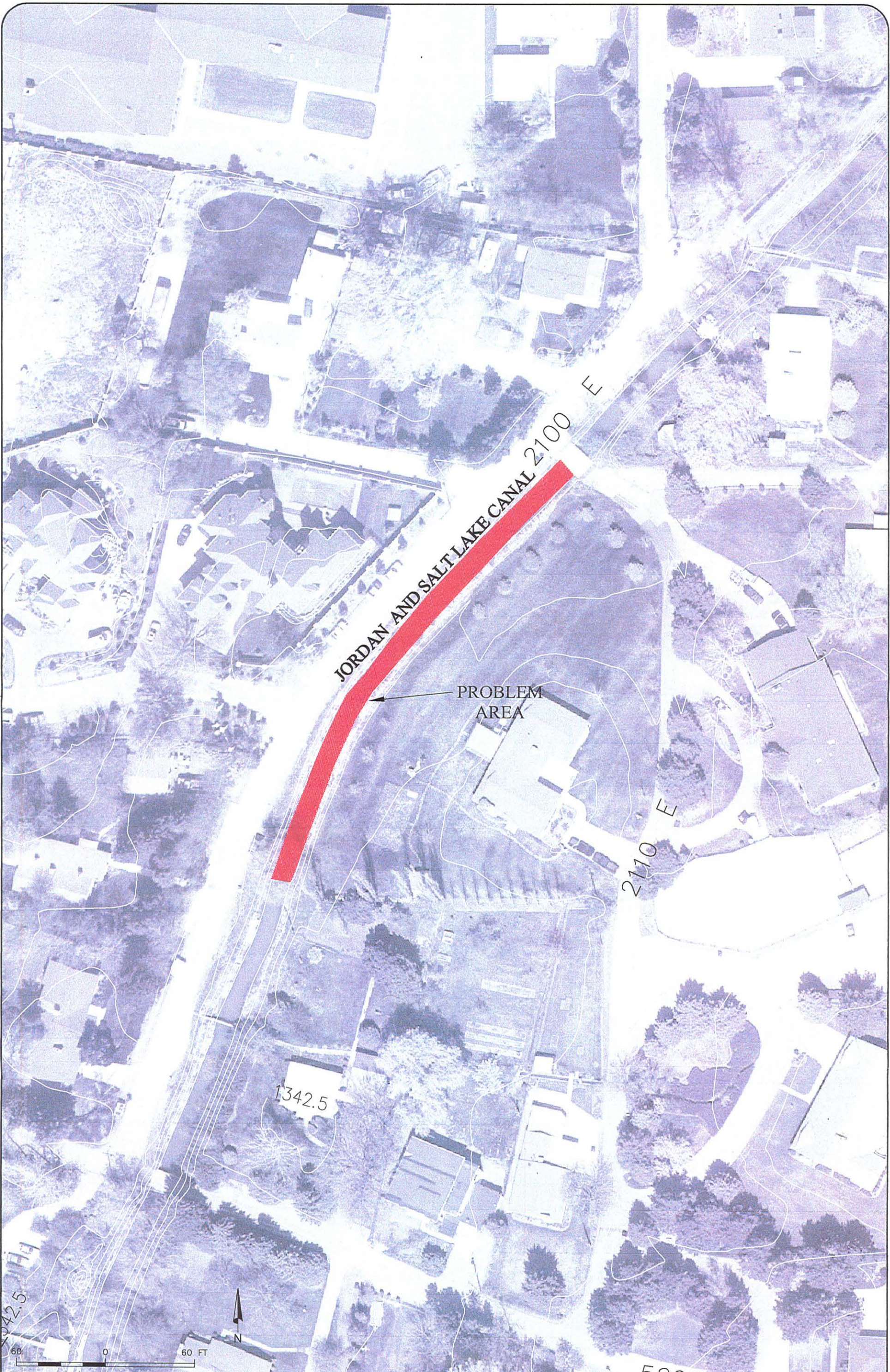


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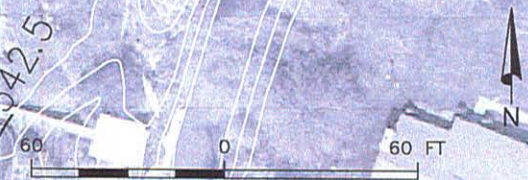


SALT LAKE COUNTY EASTSIDE CANAL STUDY - EAST JORDAN CANAL
 STORM DRAINAGE CAPACITY PROBLEM - 6800 SOUTH 1650 EAST

FIGURE
 IV-3



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SALT LAKE COUNTY EASTSIDE CANAL STUDY - JORDAN AND SALT LAKE CANAL
STORM DRAINAGE PROBLEM - 5250 SOUTH. 2100 EAST

FIGURE
IV-4

CHAPTER V

RECOMMENDATIONS

This chapter contains a discussion of the actions HAL recommends to Salt Lake County to improve the storm drainage capacity of the Eastside Canals. The following topics are included in this section:

- Eastside Canals Storm Drainage Capacity Improvement Projects
- Conceptual Construction Costs and Prioritization of Improvements
- Maintenance
- Summary of Recommendations

PREFERRED SOLUTION FOR IMPROVING STORM DRAINAGE CAPACITY

Meetings were held with Salt Lake County personnel to evaluate alternatives for improving the storm drainage capacity of the Eastside Canals. Selection of the preferred alternative was a process of evaluation and refinement, rather than a simple choice between alternatives. Input from County personnel has been very helpful in efficiently eliminating impractical solutions and in formulating practical solutions. The process of selecting a preferred alternative with County staff included: reviewing the list of canal storm drainage inadequacies, brainstorming possible solutions to the problems, screening alternatives based on feasibility and public acceptance, development of alternatives, comparison based on cost and function, and presentation of the alternatives analysis to County staff for selection of the preferred alternative. Preferred solutions were then presented to the respective canal companies and involved cities along with major alternatives for comments. The preferred solution for each inadequacy is described in the following paragraphs. All flows given are peak storm runoff flows in a 10-year, 3-hour storm event.

East Jordan Canal

11900 South 625 East

Downstream of the bridge at 11900 South a limiting section was identified which has insufficient capacity for the 10-year, 3-hour storm runoff event (see Figure IV-1). The preferred solution to the problem is to raise the west bank of the canal to increase the capacity through this reach. The water surface elevation, under 10-year, 3-hour storm conditions, is slightly below the elevation of the west bank. The east bank is significantly higher than the west bank in this area. It is recommended that the west bank be raised by about 1 foot which would give 1 foot of freeboard on the west bank during a 10-year, 3-hour storm. The bank could be raised when the property is developed. The approximate length of the bank needing to be raised is 1,400 feet between the bridges at 11900 South and 11800 South. Figure IV-1 is a location map for this inadequacy.

8720 South to 8750 South

The canal has a low east bank between the bridges at 8750 South and 8720 South (see Figure IV-2). The preferred solution is to raise the east bank to match the elevation of the west bank of the canal through this reach. The 10-year, 3-hour storm water surface elevation through this section is about 4,459.3 feet which is the same as the east bank elevation. If the east bank is raised to the same elevation as the west bank at about 4,460 feet then there will be more than 0.5 feet of freeboard in the canal through this section during a 10-year, 3-hour storm. The total length of the bank that would need to be raised through this section is about 250 feet.

6800 South

The canal upstream and a short distance downstream of the foot bridge at 6800 South has a low east bank with a school, school playground, and homeowners shed behind the bank (see Figure IV-3). The preferred solution is to raise the east bank of canal by about 1.7 feet to be slightly lower than the elevation of the west bank. The total length of bank that would need to be raised through this section is about 900 feet.

Jordan and Salt Lake Canal

Overflows

The overflow elevation of the overflows along the Jordan and Salt Lake Canal is generally higher than the base flow water surface elevation because the flow carried by the Jordan and Salt Lake Canal has decreased in recent years. Modifications to the following overflows on the Jordan and Salt Lake Canal are recommended to allow the release of excess water over the canal base flow: 3900 South, Big Cottonwood Creek, Little Cottonwood Creek, 7200 South, 10300 South, and 12400 South. It is recommended that a more detailed study be completed to determine how the overflows and outfalls will need to be modified.

5250 South

The canal upstream of the bridge at 5250 South has insufficient capacity. If the banks were overtopped at this location of the canal, a home would be flooded. It is recommended that the east bank of the canal be raised by about 1 foot and a curb be installed along 2100 East which parallels the canal along the west bank (see Figure IV-4). Alternatively, if a new overflow is installed on the Jordan and Salt Lake Canal at Van Winkle then the problem is taken care of. A new overflow at Van Winkle is currently a part of Salt Lake County's Master Plan.

Upper Canal

Downstream of 3660 South

If the headgate at 3660 South is modeled as being closed, flooding occurs along the ditch downstream. The preferred solution is to modify the overflow at this location to provide for automatic overflow of flows which exceed downstream capacity.

CONCEPTUAL CONSTRUCTION COSTS OF THE CANAL IMPROVEMENTS

Construction costs for improving the storm drainage capacity in the Eastside Canals were estimated for planning purposes (see Table V-1). Unit costs for the construction cost estimates are based on conceptual level engineering. Sources used to estimate construction costs included: The 2002 "Means Heavy Construction Cost Data", price quotes from equipment suppliers, and recent construction bids for similar work.

All costs are presented in 2002 dollars. Recent price and economic trends indicate that future costs are difficult to predict with certainty. Engineering cost estimates given in this study should be regarded as conceptual level as appropriate for use as a planning guide. Only during final design can a more definitive and accurate estimate be provided. A break-down of the cost estimate for each project is provided in Appendix E.

TABLE V-1
CONCEPTUAL COSTS FOR RECOMMENDED IMPROVEMENTS TO
THE EASTSIDE CANALS

LOCATION	DESCRIPTION	CONCEPTUAL COST ¹
East Jordan Canal - 8720 South to 8750 South	Build up canal bank	\$8,000
East Jordan Canal - 6800 South	Build up canal bank	\$56,000
East Jordan Canal - 11900 South 625 East	Build up canal bank or install overflow at Van Winkle	\$22,000
Jordan and Salt Lake Canal - 5250 South	Build up canal bank	\$48,000
Jordan and Salt Lake Canal - 3900 South, Big and Little Cottonwood Creeks, 7200, 10300, and 12400 South	Modify overflows to provide capacity to discharge storm flow	\$100,000
Upper Canal - Downstream of 3660 S	Replace headgate with automatic overflow	\$30,000
TOTAL		\$264,000

1) Costs are in 2002 dollars.

MAINTENANCE

It is impossible to overstate the importance of effective maintenance in the overall storm water capacity of the Eastside Canals. Without maintenance, the canals will deteriorate, and their storm drainage capacities will be reduced by accumulations of sediments, vegetation, and debris. Historically, the canals have been well maintained. Inadequate maintenance, as with any facility, transforms a productive resource into a multi-faceted liability. It is imperative that sufficient maintenance manpower and equipment are made available to ensure proper function and community acceptance.

Concern was expressed in meetings with Eastside Canal owners that encroachment is becoming a major hinderment to maintenance. Rights-of-way need to be protected from encroachment. A major tool in protecting the rights-of-way for the canals may be to grant recorded easements to the County. It is recommended that a feasibility study be conducted to determine the cost and legal issues related to establishing easements for the canals to Salt Lake County.

It is assumed that if the projects presented in this study for improving storm drainage capacity in the Eastside Canals are completed, the Eastside Canals would be able to convey a 10-year, 3-hour storm event automatically without opening any manually operated head gates. If the head gates were open in anticipation of a storm event, the canal could convey storm drainage runoff from a larger storm event. It is recommended that the County maintain an action plan to optimize the use of the Eastside Canals during a major storm event. For example, have a crew on-call who are trained to operate head gates and monitor key locations in anticipation of and during a major storm event.

NEW STORM DRAINAGE INFLOWS

The Eastside Canals were analyzed assuming the current storm drainage inflows. If a new storm drainage inflow into one of the Eastside Canals is considered, the storm drainage impact of the inflow should be adequately analyzed to insure it will not create an increased risk of flooding during a storm event.

SUMMARY OF RECOMMENDATIONS

1. It is recommended that the Eastside Canals Storm Drainage Capacity Improvement Projects be included in the Salt Lake County Flood Control Capital Improvement Plan.
2. It is recommended that a more detailed study of the Jordan and Salt Lake Canal overflow and outfall systems be completed.
3. The Sandy City Storm Drain Master Plan projects have a significant impact on the storm drain capacity in the East Jordan Canal and the Jordan and Salt Lake Canal. It is vital that these projects be implemented. It is recommended that Salt Lake County work with Sandy City in monitoring the progress of the projects.

4. It is recommended that Salt Lake County continue to work with canal owners to help ensure that the Eastside Canals are properly maintained and able to convey the storm drainage runoff discharged into the canals.
5. Model data should be updated as further land use, conveyance, capacity, and storm drainage inflows data become available.
6. It is recommended that the Eastside Canal Study be periodically reviewed and updated.
7. It is recommended that the impact of a proposed inflow into a canal be adequately analyzed to insure it will not create an increased risk of flooding before it is allowed.
8. It is recommended that a feasibility study be conducted to determine the cost and legal issues related to granting Salt Lake County recorded easements for the canals to allow Salt Lake County to protect rights-of-way from encroachment.
9. It is recommended that the County maintain an action plan to optimize the use of the Eastside Canals during a major storm event. For example, have a crew on-call who are trained to operate head gates and monitor key locations in anticipation of and during a major storm event.

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- Utah Administrative Code
- Utah Code Annotated

APPENDIX A

STORM DRAINAGE MODEL OUTPUT

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* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIME- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE PORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMET INFILTRATION KINEMATIC WAVE; NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

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2         ID
3         ID
4         IT      1              240
5         IO      3
6         KK      SBC19
7         BA      0.517
8         PB      1.21
9         IN      5
10        PI      0.009  0.009  0.009  0.009  0.009  0.009  0.014  0.014  0.014  0.014
11        PI      0.014  0.014  0.160  0.306  0.100  0.050  0.050  0.050  0.035  0.035
12        PI      0.035  0.035  0.035  0.035  0.014  0.014  0.014  0.014  0.014  0.014
13        PI      0.009  0.009  0.009  0.009  0.009  0.009
14        LS      0.1      98
15        UK      25      .02      .1      13.46
16        UK      70      .02      .4      86.54
17        RD      500     .02      .02      .01      TRAP      2      25
18        RD      2000    .02      .015     CIRC      2
19        KO      22
20        KK      CV859Upper Canal
21        RD      1058.60 0.00050  0.030     TRAP      8.00  1.00
22        KO      22
23        KK      SBC18
24        BA      0.1642
25        LS      0.1      98.000     67.635  0.1
26        UK      25      .02      .1      21.36
27        UK      70      .02      .4      78.64
28        RD      600     .02      .02      .013     TRAP      2      25
29        RD      2400    .038     .02      CIRC      2
30        KO      22
31        KK      HC368
32        HC      2
33        KO      22
34        KK      CV767Upper Canal
35        RD      2028.28 0.00050  0.030     TRAP      8.00  1.00
36        KO      22

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37	KK	SBC17							
38	BA	0.1927							
39	LS	0.1	98.000			69.166	0.1		
40	UK	25	.02	.1	26.74				
41	UK	70	.02	.4	73.26				
42	RD	1000	.02	.02	.03	TRAP	2	25	
43	RD	3500	.028	.02		TRAP	2	25	
44	KO					22			
45	KK	HC369							
46	HC	2							
47	KO					22			
48	KK	CV768Upper Canal							
49	RD	2043.84	0.00050	0.030		TRAP	8.00	1.00	
50	KO					22			
51	KK	SBC16							
52	BA	0.3566							
53	LS	0.1	98.000			69.047	0.1		
54	UK	25	.02	.1	26.03				
55	UK	70	.02	.4	73.97				
56	RD	900	.02	.02	.039	TRAP	2	25	
57	RD	5500	.028	.02		TRAP	2	25	
58	KO					22			
59	KK	HC370							
60	HC	2							
61	KO					22			
62	KK	CV769Upper Canal							
63	RD	4038.60	0.00050	0.030		TRAP	8.00	1.00	
64	KO					22			
65	KK	DH35							
66	DT	DR35							
67	DI	0	25	50	100	200	400	800	
68	DQ	0	25	50	100	200	400	800	
69	KO					22			
70	KK	SBC15							
71	BA	0.2984							
72	LS	0.1	98.000			70.613	0.1		
73	UK	25	.02	.1	29.09				
74	UK	70	.02	.4	70.9				
75	RD	2000	.018	.02		TRAP	2	25	
76	KO					22			
77	KK	HC37							
78	HC	2							
79	KO					22			
80	KK	CV870							
81	RD	1414.47	0.00050	0.030		TRAP	8.00	1.00	
82	KO					22			
83	KK	SB419							
84	BA	0.1373							
85	LS	0.1	98.000			86.000	0.1		
86	UK	20	.02	.1	60.00				
87	UK	300	.04	.4	40.00				
88	RD	2000	.04	.02	.018	TRAP	2	25	
89	RD	3800	.005	.04		TRAP	10	2	
90	KO					22			
91	KK	CV539East Jordan Canal							
92	RD	947.634	0.00050	0.030		TRAP	20.00	1.50	
93	KO					22			
94	KK	SB647							
95	BA	0.0433							
96	LS	0.1	98.000			65.000	0.1		
97	UK	40	.02	.1	28.68				
98	UK	150	.05	.4	71.32				
99	RD	1200	.07	.04		TRAP	0	2	
100	KO					22			

101	KK	SB85								
102	BA	0.0185								
103	LS	0.1	98.000			75.000		0.1		
104	UK	20	.02	.1	11.00					
105	UK	200	.05	.4	89.00					
106	RD	1500	.05	.04		TRAP		0		2
107	KO					22				
108	KK	HC212								
109	HC	3								
110	KO					22				
111	KK	CV538East Jordan Canal								
112	RD	893.659	0.00050	0.030		TRAP		20.00		1.50
113	KO					22				
114	KK	CV537East Jordan Canal								
115	RD	588.552	0.00050	0.030		TRAP		20.00		1.50
116	KO					22				
117	KK	SB483								
118	BA	0.0230								
119	LS	0.1	98.000			64.205		0.1		
120	UK	25	.1	.1	15.37					
121	UK	80	.1	.4	84.63					
122	RD	1500	.1	.02		TRAP		2		25
123	KO					22				
124	KK	DB29								
125	RS		FLOW							
126	SV	0	0.013	0.0267	0.0417	0.058	0.075	0.0938	0.147	0.1617
127	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
128	SQ	0	1.2	1.6974	2.0789	2.4	2.684	2.94	3.0135	7.35
129	KO					22				
130	KK	CV686to East Jordan Canal								
131	RD	150.002	0.10600	0.015		CIRC		2.00		0.00
132	KO					22				
133	KK	SB484								
134	BA	0.0496								
135	LS	0.1	98.000			64.451		0.1		
136	UK	20	.05	.1	22.07					
137	UK	75	.05	.4	77.93					
138	RD	2000	.05	.02		TRAP		2		25
139	KO					22				
140	KK	HC310								
141	HC	2								
142	KO					22				
143	KK	CV687to East Jordan Canal								
144	RD	222.905	0.05030	0.015		CIRC		1.25		0.00
145	KO					22				
146	KK	CV688to East Jordan Canal								
147	RD	234.031	0.03430	0.015		CIRC		1.25		0.00
148	KO					22				
149	KK	CV689to East Jordan Canal								
150	RD	364.368	0.06520	0.015		CIRC		1.25		0.00
151	KO					22				
152	KK	CV690to East Jordan Canal								
153	RD	452.124	0.10690	0.015		CIRC		1.50		0.00
154	KO					22				
155	KK	DB78								
156	RS		FLOW							
157	SV	0	0.04	0.0842	0.1317	0.183	0.238	0.2961	0.464	0.5104
158	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
159	SQ	0	1.895	2.679	3.281	3.789	4.236	4.64	4.756	11.6
160	KO					22				
161	KK	SB479								
162	BA	0.0179								
163	LS	0.1	98.000			65.000		0.1		

164	UK	15	.02	.1	23.38					
165	UK	100	.1	.4	76.62					
166	RD	900	.01	.015		CIRC	1.25			
167	KO					22				
168	KK	HC31								
169	HC	2								
170	KO					22				
171	KK	CV715to East Jordan Canal								
172	RD	512.804	0.03000	0.015		CIRC	1.25	0.00		
173	KO					22				
174	KK	SB475								
175	BA	0.0333								
176	LS	0.1	98.000			69.369	0.1			
177	UK	30	.02	.1	65.00					
178	UK	300	.1	.4	35.00					
179	RD	1300	.1	.04		TRAP	0	2		
180	KO					22				
181	KK	HC312								
182	HC	2								
183	KO					22				
184	KK	CV717to East Jordan Canal								
185	RD	1130.45	0.10000	0.030		TRAP	0.00	2.00		
186	KO					22				
187	KK	SB477								
188	BA	0.0344								
189	LS	0.1	98.000			60.000	0.1			
190	UK	25	.05	.1	22.39					
191	UK	70	.1	.4	77.6					
192	RD	500	.1	.02	0.003	TRAP	2	25		
193	RD	1300	.1	.02		TRAP	2	25		
194	KO					22				
195	KK	DB80								
196	RS		FLOW							
197	SV	0	0.019	0.0399	0.0625	0.087	0.113	0.1404	0.22	0.242
198	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
199	SQ	0	1.796	2.5403	3.1113	3.593	4.017	4.4	4.51	11
200	KO					22				
201	KK	SB478								
202	BA	0.037								
203	LS	0.1	98.000			60.804	0.1			
204	UK	20	.05	.1	16.16					
205	UK	200	.1	.4	83.84					
206	RD	1200	.1	.02		TRAP	2	25		
207	KO					22				
208	KK	HC314								
209	HC	2								
210	KO					22				
211	KK	CV713to East Jordan Canal								
212	RD	918.836	0.14000	0.030		TRAP	0.00	2.00		
213	KO					22				
214	KK	DB79								
215	RS		FLOW							
216	SV	0	0.04	0.0831	0.13	0.181	0.235	0.2922	0.458	0.5038
217	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
218	SQ	0	1.87	2.644	3.239	3.74	4.181	4.58	4.695	11.45
219	KO					22				
220	KK	SB837								
221	BA	0.0369								
222	LS	0.1	98.000			81.000	0.1			
223	UK	30	.02	.1	35.00					
224	UK	300	.1	.4	65.00					
225	RD	1800	.1	.04		TRAP	0	2		
226	KO					22				
227	KK	HC313								

228	HC	2						
229	KO					22		
230	KK	CV716to East Jordan Canal						
231	RD	1984.04	0.10000	0.040		TRAP	0.00	2.00
232	KO					22		
233	KK	HC17						
234	HC	2						
235	KO					22		
236	KK	CV726to East Jordan Canal						
237	RD	87.941	0.01000	0.040		TRAP	0.00	2.00
238	KO					22		
239	KK	SB474						
240	BA	0.0665						
241	LS	0.1	98.000			65.000	0.1	
242	UK	20	.05	.1	22.17			
243	UK	70	.05	.4	77.83			
244	RD	500	.05	.02	.007	TRAP	2	25
245	RD	2000	.05	.015		CIRC	24	
246	KO					22		
247	KK	CV674to East Jordan Canal						
248	RD	314.328	0.03300	0.015		CIRC	2.50	0.00
249	KO					22		
250	KK	CV675to East Jordan Canal						
251	RD	189.127	0.09000	0.015		CIRC	3.00	0.00
252	KO					22		
253	KK	CV673to East Jordan Canal						
254	RD	687.840	0.00800	0.015		CIRC	2.50	0.00
255	KO					22		
256	KK	CV685to East Jordan Canal						
257	RD	146.902	0.00950	0.015		CIRC	2.50	0.00
258	KO					22		
259	KK	CV684to East Jordan Canal						
260	RD	238.739	0.02260	0.015		CIRC	2.50	0.00
261	KO					22		
262	KK	CV676to East Jordan Canal						
263	RD	487.683	0.07130	0.015		CIRC	2.50	0.00
264	KO					22		
265	KK	SB649						
266	BA	0.0502						
267	LS	0.1	98.000			60.000	0.1	
268	UK	15	.05	.1	24.47			
269	UK	60	.05	.4	75.53			
270	RD	550	.1	.02	.0025	TRAP	2	25
271	RD	1500	.05	.015		CIRC	2	
272	KO					22		
273	KK	CV677to East Jordan Canal						
274	RD	188.712	0.08450	0.015		CIRC	2.00	0.00
275	KO					22		
276	KK	HC216						
277	HC	2						
278	KO					22		
279	KK	CV678to East Jordan Canal						
280	RD	423.497	0.05560	0.015		CIRC	2.50	0.00
281	KO					22		
282	KK	CV679to East Jordan Canal						
283	RD	132.897	0.04170	0.015		CIRC	3.00	0.00
284	KO					22		
285	KK	SB648						
286	BA	0.0203						
287	LS	0.1	98.000			65.000	0.1	
288	UK	20	.05	.1	24.08			

289	UK	50	.05	.4	75.92						
290	RD	400	.005	.02	.003	TRAP	2	25			
291	RD	1100	.07	.015		CIRC	1.25				
292	KO					22					
293	KK CV680to East Jordan Canal										
294	RD	138.908	0.00500	0.015		CIRC	1.00	0.00			
295	KO					22					
296	KK HC215										
297	HC	2									
298	KO					22					
299	KK CV681to East Jordan Canal										
300	RD	211.329	0.00900	0.015		CIRC	3.50	0.00			
301	KO					22					
302	KK CV682to East Jordan Canal										
303	RD	270.703	0.01790	0.015		CIRC	3.50	0.00			
304	KO					22					
305	KK SB788										
306	BA	0.0396									
307	LS	0.1	98.000			75.000	0.1				
308	UK	30	.02	.1	11.00						
309	UK	250	.05	.4	89.00						
310	RD	1300	.08	.04		TRAP	0	2			
311	KO					22					
312	KK HC214										
313	HC	3									
314	KO					22					
315	KK DB8										
316	RS	FLOW									
317	SV	0	0.345	0.7203	1.1269	1.564	2.033	2.5326	3.969	4.3659	
318	SE	0	0.5	1	1.5	2	2.5	3	4	4.5	
319	SQ	0	32.41	45.83	56.13	64.81	72.46	79.38	81.365	198.45	
320	KO					22					
321	KK CV683to East Jordan Canal										
322	RD	125.889	0.01500	0.015		CIRC	1.67	0.00			
323	KO					22					
324	KK HC213										
325	HC	2									
326	KO					22					
327	KK CV385East Jordan Canal										
328	RD	3075.40	0.00050	0.030		TRAP	20.00	1.50			
329	KO					22					
330	KK SB789										
331	BA	0.0895									
332	LS	0.1	98.000			68.097	0.1				
333	UK	30	.02	.1	1.00						
334	UK	250	.15	.4	99.00						
335	RD	1000	.15	.04	.02	TRAP	0	2			
336	RD	2500	.005	.04		TRAP	12	2			
337	KO					22					
338	KK SB787										
339	BA	0.0443									
340	LS	0.1	98.000			65.000	0.1				
341	UK	20	.02	.1	19.25						
342	UK	75	.02	.4	80.75						
343	RD	700	.05	.02	.005	TRAP	2	25			
344	RD	2170	.02	.02		TRAP	2	25			
345	KO					22					
346	KK SB786										
347	BA	0.0665									
348	LS	0.1	98.000			63.562	0.1				
349	UK	20	.03	.1	10.38						
350	UK	100	.03	.4	89.62						
351	RD	2250	.03	.02		TRAP	2	25			
352	KO					22					

353	KK	SB785						
354	BA	0.054						
355	LS	0.1	98.000		75.000	0.1		
356	UK	25	.025	.1	11.00			
357	UK	250	.02	.4	89.00			
358	RD	600	.02	.02	0.025	TRAP	2	10
359	RD	2200	.005	.04		TRAP	10	2
360	KO					22		
361	KK	HC217						
362	HC	5						
363	KO					22		
364	KK	CV348East Jordan Canal						
365	RD	2251.96	0.00050	0.030		TRAP	20.00	1.50
366	KO					22		
367	KK	SB874						
368	BA	0.0615						
369	LS	0.1	98.000		78.000	0.1		
370	UK	20	.02	.1	18.00			
371	UK	200	.02	.4	82.00			
372	RD	2600	.025	.02		TRAP	2	25
373	KO					22		
374	KK	HC						
375	HC	2						
376	KO					22		
377	KK	CV347East Jordan Canal						
378	RD	275.580	0.00050	0.030		TRAP	20.00	1.50
379	KO					22		
380	KK	SB896						
381	BA	0.0577						
382	LS	0.1	98.000		78.000	0.1		
383	UK	20	.02	.1	18.00			
384	UK	150	0.01	.4	82.00			
385	RD	2500	.02	.02		TRAP	2	25
386	KO					22		
		PAGE 10						
387	KK	CV346to East Jordan Canal						
388	RD	107.676	0.00300	0.015		CIRC	1.50	0.00
389	KO					22		
390	KK	HC2						
391	HC	2						
392	KO					22		
393	KK	CV345East Jordan Canal						
394	RD	594.174	0.00050	0.030		TRAP	20.00	1.50
395	KO					22		
396	KK	DH36						
397	DT	DR36						
398	DI	0	25	50	100	200	400	800
399	DQ	0	25	50	100	200	400	800
400	KO					22		
401	KK	SB169						
402	BA	0.0504						
403	LS	0.1	98.000		75.000	0.1		
404	UK	20	.02	.1	11.00			
405	UK	80	.01	.4	89.00			
406	RD	1000	.01	.02		TRAP	2	25
407	KO					22		
408	KK	HC218						
409	HC	2						
410	KO					22		
411	KK	CV276East Jordan Canal						
412	RD	2551.74	0.00050	0.030		TRAP	20.00	1.50
413	KO					22		
414	KK	SB804						

415	BA	0.0273								
416	LS	0.1	98.000		70.493		0.1			
417	UK	20	.02	.1	11.33					
418	UK	75	.02	.4	88.67					
419	RD	2000	.01	.02		TRAP	2	25		
420	KO					22				
421	KK	CV272to East Jordan Canal								
422	RD	627.594	0.00400	0.015		CIRC	2.00	0.00		
423	KO					22				
424	KK	HC3								
425	HC	2								
426	KO					22				
427	KK	CV273East Jordan Canal								
428	RD	1050.47	0.00050	0.030		TRAP	20.00	1.50		
429	KO					22				
430	KK	SB805								
431	BA	0.0450								
432	LS	0.1	98.000		75.000		0.1			
433	UK	20	.02	.1	11.00					
434	UK	90	.02	.4	89.00					
435	RD	1800	.005	.02		TRAP	2	25		
436	KO					22				
437	KK	CV206to East Jordan Canal								
438	RD	1094.49	0.00100	0.015		CIRC	1.25	0.00		
439	KO					22				
440	KK	DB35								
441	RS		FLOW							
442	SV	0	0.027	0.057	0.0892	0.124	0.161	0.2004	0.314	0.3454
443	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
444	SQ	0	2.564	3.6258	4.4406	5.128	5.733	6.28	6.437	15.7
445	KO					22				
446	KK	CV207to East Jordan Canal								
447	RD	532.660	0.00300	0.015		CIRC	1.25	0.00		
448	KO					22				
449	KK	SB806								
450	BA	0.0528								
451	LS	0.1	98.000		75.000		0.1			
452	UK	30	.02	.1	11.00					
453	UK	200	.005	.4	89.00					
454	RD	400	.005	.04	0.012	TRAP	2	5		
455	RD	900	.005	.04		TRAP	12	2		
456	KO					22				
457	KK	HC219								
458	HC	2								
459	KO					22				
460	KK	CV696to East Jordan Canal								
461	RD	303.503	0.00500	0.015		CIRC	2.00	0.00		
462	KO					22				
463	KK	CV342to East Jordan Canal								
464	RD	342.935	0.01000	0.030		TRAP	1.50	0.50		
465	KO					22				
466	KK	CV695to East Jordan Canal								
467	RD	464.597	0.01900	0.015		CIRC	2.00	0.00		
468	KO					22				
469	KK	HC4								
470	HC	2								
471	KO					22				
472	KK	CV340East Jordan Canal								
473	RD	1306.12	0.00050	0.030		TRAP	20.00	1.50		
474	KO					22				
475	KK	SB776								
476	BA	0.0415								

477	LS	0.1	98.000			75.828	0.1	
478	UK	30	.02	.1	11.00			
479	UK	200	.01	.4	89.00			
480	RD	1000	.01	.02		TRAP	1	10
481	KO					22		
482	KK	CV339to East Jordan Canal						
483	RD	548.907	0.01500	0.015		CIRC	2.00	0.00
484	KO					22		
485	KK	SB875						
486	BA	0.044						
487	LS	0.1	98.000			65.000	0.1	
488	UK	25	.02	.1	11.96			
489	UK	200	.01	.4	88.04			
490	RD	1800	.01	.03		TRAP	2	10
491	KO					22		
492	KK	HC220						
493	HC	2						
494	KO					22		
495	KK	CV338to East Jordan Canal						
496	RD	727.333	0.01000	0.030		TRAP	1.50	0.50
497	KO					22		
498	KK	CV337to East Jordan Canal						
499	RD	302.264	0.01000	0.030		TRAP	1.50	0.50
500	KO					22		
501	KK	SB775						
502	BA	0.0504						
503	LS	0.1	98.000			78.000	0.1	
504	UK	20	.02	.1	18.00			
505	UK	120	.02	.4	82.00			
506	RD	400	.01	.03	0.01	TRAP	2	25
507	RD	1700	.01	.02		TRAP	2	25
508	KO					22		
509	KK	HC22						
510	HC	2						
511	KO					22		
512	KK	CV336to East Jordan Canal						
513	RD	623.538	0.01000	0.030		TRAP	1.50	0.50
514	KO					22		
515	KK	SB773						
516	BA	0.043						
517	LS	0.1	98.000			78.000	0.1	
518	UK	30	.02	.1	18.00			
519	UK	200	.005	.4	82.00			
520	RD	1000	.005	.04		TRAP	0	10
521	KO					22		
522	KK	HC222						
523	HC	3						
524	KO					22		
525	KK	CV335East Jordan Canal						
526	RD	1700.71	0.00050	0.030		TRAP	20.00	1.50
527	KO					22		
528	KK	SB772						
529	BA	0.0452						
530	LS	0.1	98.000			78.000	0.1	
531	UK	20	.02	.1	18.00			
532	UK	80	.02	.4	82.00			
533	RD	500	.05	.02	.01	TRAP	2	25
534	RD	1000	.005	.02		TRAP	2	25
535	KO					22		
536	KK	CV239to East Jordan Canal						
537	RD	836.168	0.00700	0.015		CIRC	1.00	0.00

538	KO					22		
539	KK	SB763						
540	BA	0.0334						
541	LS	0.1	98.000			81.000	0.1	
542	UK	15	.02	.1	35.00			
543	UK	65	.02	.4	65.00			
544	RD	1200	.02	.02		TRAP	2	25
545	KO					22		
546	KK	SB77						
547	BA	0.0345						
548	LS	0.1	98.000			65.000	0.1	
549	UK	40	.02	.1	77.24			
550	UK	75	.05	.4	22.76			
551	RD	2200	.08	.02		TRAP	2	25
552	KO					22		
553	KK	HC325						
554	HC	2						
555	KO					22		
556	KK	CV246to East Jordan Canal						
557	RD	82.167	0.01200	0.015		CIRC	1.00	0.00
558	KO					22		
559	KK	CV237to East Jordan Canal						
560	RD	675.848	0.00500	0.015		CIRC	1.50	0.00
561	KO					22		
562	KK	CV238to East Jordan Canal						
563	RD	681.066	0.00500	0.015		CIRC	1.00	0.00
564	KO					22		
565	KK	HC224						
566	HC	2						
567	KO					22		
568	KK	CV240to East Jordan Canal						
569	RD	357.124	0.00700	0.015		CIRC	1.00	0.00
570	KO					22		
571	KK	SB770						
572	BA	0.0094						
573	LS	0.1	98.000			80.000	0.1	
574	UK	15	.02	.1	22.00			
575	UK	300	.005	.4	78.00			
576	RD	650	.005	.04		TRAP	10	2
577	KO					22		
578	KK	HC223						
579	HC	3						
580	KO					22		
581	KK	CV234East Jordan Canal						
582	RD	708.474	0.00050	0.030		TRAP	20.00	1.50
583	KO					22		
584	KK	SB769						
585	BA	0.0165						
586	LS	0.1	98.000			65.000	0.1	
587	UK	20	.02	.1	24.6			
588	UK	75	.02	.4	75.39			
589	RD	400	.005	.02	.003	TRAP	2	25
590	RD	1700	.01	.02		TRAP	2	25
591	KO					22		
592	KK	HC226						
593	HC	2						
594	KO					22		
595	KK	CV228East Jordan Canal						
596	RD	760.905	0.00050	0.030		TRAP	20.00	1.50
597	KO					22		

598	KK	SB60						
599	BA	0.0274						
600	LS	0.1	98.000		86.000	0.1		
601	UK	40	.02	.1	50.00			
602	UK	100	.1	.4	50.00			
603	RD	900	.015	.02		TRAP	2	25
604	KO					22		
605	KK	CV135to East Jordan Canal						
606	RD	518.341	0.01300	0.015		CIRC	1.50	0.00
607	KO					22		
608	KK	CV136to East Jordan Canal						
609	RD	170.923	0.03500	0.015		CIRC	1.50	0.00
610	KO					22		
611	KK	SB762						
612	BA	0.0295						
613	LS	0.1	98.000		60.000	0.1		
614	UK	30	.02	.1	47.90			
615	UK	25	.02	.4	52.10			
616	RD	900	.02	.015		CIRC	1.25	
617	KO					22		
618	KK	HC227						
619	HC	2						
620	KO					22		
621	KK	CV138to East Jordan Canal						
622	RD	147.175	0.03400	0.015		CIRC	1.50	0.00
623	KO					22		
624	KK	CV139to East Jordan Canal						
625	RD	219.902	0.01500	0.015		CIRC	1.75	0.00
626	KO					22		
627	KK	SB764						
628	BA	0.0334						
629	LS	0.1	98.000		60.000	0.1		
630	UK	50	.01	.1	63.24			
631	UK	10	.02	.4	36.76			
632	RD	780	.01	.015		CIRC	2	
633	KO					22		
634	KK	HC228						
635	HC	2						
636	KO					22		
637	KK	CV140to East Jordan Canal						
638	RD	401.330	0.01000	0.015		CIRC	2.25	0.00
639	KO					22		
640	KK	CV141to East Jordan Canal						
641	RD	419.731	0.01900	0.015		CIRC	2.25	0.00
642	KO					22		
643	KK	SB766						
644	BA	0.0375						
645	LS	0.1	98.000		60.000	0.1		
646	UK	50	.01	.1	65.78			
647	UK	20	.02	.4	34.22			
648	RD	1000	.01	.015		CIRC	1.25	
649	KO					22		
650	KK	HC229						
651	HC	2						
652	KO					22		
653	KK	CV129to East Jordan Canal						
654	RD	731.201	0.05200	0.015		CIRC	2.25	0.00
655	KO					22		
656	KK	CV130to East Jordan Canal						

657	RD	417.372	0.03800	0.015		CIRC	2.25	0.00		
658	KO					22				
659	KK	DB10								
660	RS		FLOW							
661	SV	0	0.237	0.495	0.774	1.075	1.397	1.74	2.727	3
662	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
663	SQ	0	7.626	10.785	13.209	15.25	17.05	18.68	19.147	46.7
664	KO					22				
665	KK	CV131to East Jordan Canal								
666	RD	172.257	0.13000	0.015		CIRC	2.00	0.00		
667	KO					22				
668	KK	SB767								
669	BA	0.0178								
670	LS	0.1	98.000			61.864	0.1			
671	UK	40	.1	.1	40.85					
672	UK	200	.1	.4	59.15					
673	RD	600	.2	.02	.001	TRAP	2	25		
674	RD	1300	.005	.04		TRAP	10	2		
675	KO					22				
676	KK	SB768								
677	BA	0.0267								
678	LS	0.1	98.000			65.000	0.1			
679	UK	40	.075	.1	42.72					
680	UK	70	.07	.4	57.28					
681	RD	1950	.1	.02		TRAP	2	25		
682	KO					22				
683	KK	HC230								
684	HC	3								
685	KO					22				
686	KK	CV132to East Jordan Canal								
687	RD	1033.58	0.02000	0.015		CIRC	2.00	0.00		
688	KO					22				
689	KK	CV133to East Jordan Canal								
690	RD	294.204	0.01650	0.015		CIRC	2.00	0.00		
691	KO					22				
692	KK	CV134to East Jordan Canal								
693	RD	625.603	0.01780	0.015		CIRC	2.00	0.00		
694	KO					22				
695	KK	SB756								
696	BA	0.0285								
697	LS	0.1	98.000			78.000	0.1			
698	UK	20	.03	.1	19.00					
699	UK	80	.03	.4	81.00					
700	RD	1200	.03	.02		TRAP	2	25		
701	KO					22				
702	KK	HC225								
703	HC	3								
704	KO					22				
705	KK	CV227East Jordan Canal								
706	RD	653.505	0.00050	0.030		TRAP	20.00	1.50		
707	KO					22				
708	KK	SB754								
709	BA	0.0353								
710	LS	0.1	98.000			78.000	0.1			
711	UK	20	.02	.1	18.00					
712	UK	70	.02	.4	82.00					
713	RD	1500	.01	.04		TRAP	2	25		
714	KO					22				
715	KK	HC23								
716	HC	2								
717	KO					22				

718	KK	CV232	East Jordan Canal						
719	RD	674.941	0.00050	0.030			TRAP	20.00	1.50
720	KO						22		
721	KK	SB548							
722	BA	0.0472							
723	LS	0.1	98.000				65.000	0.1	
724	UK	20	.02	.1	12.23				
725	UK	70	.02	.4	87.77				
726	RD	600	.01	.02	0.01		TRAP	2	25
727	RD	1300	0.006	.02			TRAP	2	25
728	KO						22		
729	KK	HC232							
730	HC	2							
731	KO						22		
732	KK	CV384	East Jordan Canal						
733	RD	1487.47	0.00050	0.030			TRAP	20.00	1.50
734	KO						22		
735	KK	TP010							
736	BA	0.077							
737	LS	0.1	98				73	0.1	
738	UK	50	0.02	0.1	24				
739	UK	80	0.013	0.5	76				
740	RD	2260	0.014	0.04			TRAP	2	25
741	KK	RTP01							
742	RD	2150	0.0437	0.015			CIRC	2.5	
743	KK	TP020							
744	BA	0.038							
745	LS	0.1	98				73	0.1	
746	UK	50	0.02	0.1	30				
747	UK	100	0.01	0.5	70				
748	RD	2650	0.044	0.04			TRAP	2	25
749	KK	RTP02							
750	RD	550	0.0036	0.015			CIRC	3	
751	KK	TP030							
752	BA	0.05							
753	LS	0.1	98				73	0.1	
754	UK	50	0.02	0.1	30				
755	UK	120	0.042	0.5	70				
756	RD	2010	0.06	0.04			TRAP	2	25
757	KK	CTP03							
758	HC	3							
759	KK	RTP033							
760	RD	1800	0.0356	0.015			CIRC	2.5	
761	KK	TP100							
762	BA	0.063							
763	LS	0.1	98				73	0.1	
764	UK	50	0.02	0.1	26				
765	UK	100	0.04	0.5	74				
766	RD	2000	0.03	0.04			TRAP	2	25
767	KK	RTP10							
768	RD	1150	0.033	0.015			CIRC	2	
769	KK	TP110							
770	BA	0.06297							
771	LS	0.1	98				73	0.1	
772	UK	50	0.02	0.1	27				
773	UK	240	0.113	0.5	73				
774	RD	1970	0.035	0.04			TRAP	2	25
775	KK	TP120							
776	BA	0.038							
777	LS	0.1	98				73	0.1	

778	UK	50	0.02	0.1	30						
779	UK	60	0.05	0.5	70						
780	RD	3700	0.029	0.04		TRAP	2	25			
781	KK	TP12									
782	HC	3									
783	KK	ALTHDB									
784	RS	1	FLOW	-							
785	SV	0	0.04	0.41	1.61	3.12	3.5	3.75	0	0	0
786	SQ	0	4.5	8.4	12.9	16.2	54	122	0	0	0
787	SE	96	97	98	100	102	102.5	103			
788	KK	RTP123									
789	RD	2220	0.0072	0.015		CIRC	2.5				
790	KK	TP130									
791	BA	0.06									
792	LS	0.1	98			73	0.1				
793	UK	50	0.02	0.1	22						
794	UK	600	0.052	0.5	78						
795	RD	1590	0.013	0.04		TRAP	2	25			
796	KK	CTP13									
797	HC	3									
798	KK	RTP133									
799	RD	450	0.0267	0.015		CIRC	3				
800	KK	TP200									
801	BA	0.0866									
802	LS	0.1	98			73	0.1				
803	UK	50	0.02	0.1	2						
804	UK	100	0.01	0.5	79						
805	RD	2800	0.122	0.015		CIRC	1.5				
806	KK	RTP20									
807	RD	1500	0.0667	0.015		CIRC	1.5				
808	KK	TP210									
809	BA	0.03859									
810	LS	0.1	98			74	0.1				
811	UK	50	0.02	0.1	33						
812	UK	100	0.09	0.5	67						
813	RD	2370	0.025	0.015		CIRC	2				
814	KK	CTP21									
815	HC	3									
816	KK	TP300									
817	BA	0.05109									
818	LS	0.1	98			74	0.1				
819	UK	50	0.02	0.1	36						
820	UK	100	0.05	0.5	64						
821	RD	1830	0.065	0.04		TRAP	2	25			
822	KK	RTP30									
823	RD	570	0.1193	0.015		CIRC	1.8				
824	KK	TP310									
825	BA	0.02984									
826	LS	0.1	98			71	0.1				
827	UK	50	0.02	0.1	4						
828	UK	200	0.07	0.5	96						
829	RD	850	0.073	0.05		TRAP	2	25			
830	KK	CTP31									
831	HC	2									
832	KK	RTP312									
833	RD	720	0.0653	0.015		CIRC	1.8				
834	KK	TP330									
835	BA	0.06297									

836	LS	0.1	98			73	0.1												
837	UK	50	0.02	0.1	30														
838	UK	110	0.136	0.5	70														
839	RD	2430	0.084	0.04		TRAP	2	25											
840	KK	RTP33																	
841	RD	320	0.0031	0.015		CIRC	2												
842	KK	TP320																	
843	BA	0.03516																	
844	LS	0.1	98			71	0.1												
845	UK	50	0.02	0.1	5														
846	UK	600	0.06	0.5	95														
847	RD	2350	0.003	0.015		CIRC	3												
848	KK	CTP32																	
849	HC	3																	
850	KK	RTP323																	
851	RD	900	0.0022	0.015		CIRC	3												
852	KK	RTP324																	
853	RD	480	0.0083	0.015		CIRC	4												
854	KK	CTP325																	
855	HC	2																	
856	KK	STMDET																	
857	RS	1	STOR	0															
858	SV	0	0.001	0.11	1.49	6.6	14.8	15.5	16	17	18								
859	SQ	0	21.1	23	26.5	29.5	32.3	61	112	255	441								
860	SE	52	58	59	61	63	65	65.5	66	67	68								
861	KK	TP450																	
862	BA	0.0425																	
863	LS	0.1	98			72	0.1												
864	UK	50	0.02	0.1	24														
865	UK	220	0.032	0.5	76														
866	RD	2530	0.018	0.04		TRAP	2	25											
867	KK	CTP45																	
868	HC	2																	
869	KK	RTP452																	
870	RD	700	0.0429	0.015		CIRC	4												
871	KK	TP400																	
872	BA	0.125																	
873	LS	0.1	98			72	0.1												
874	UK	50	0.02	0.1	24														
875	UK	200	0.03	0.4	76														
876	RD	3530	0.026	0.02		TRAP	2	25											
877	KK	RTP40																	
878	RD	1200	0.025	0.015		CIRC	2.3												
879	KK	TP410																	
880	BA	0.044																	
881	LS	0.1	98			71	0.1												
882	UK	50	0.02	0.1	15														
883	UK	600	0.032	0.5	85														
884	RD	1260	0.008	0.08		TRAP	2	25											
885	KK	CTP41																	
886	HC	2																	
887	KK	RTP412																	
888	RD	1920	0.0089	0.015		CIRC	3												
889	KK	TP430																	
890	BA	0.086																	
891	LS	0.1	98			73	0.1												
892	UK	50	0.02	0.1	30														
893	UK	320	0.044	0.5	70														

894	RD	2150	0.022	0.04		TRAP	2	25					
895	KK	TP420											
896	BA	0.065											
897	LS	0.1	98			73	0.1						
898	UK	50	0.02	0.1	30								
899	UK	350	0.034	0.5	70								
900	RD	2480	0.013	0.015		CIRC	2.5						
901	KK	CTP42											
902	HC	3											
903	KK	RTP423											
904	RD	400	0.01	0.015		CIRC	3						
905	KK	AELDET											
906	RS	1	FLOW	-									
907	SV	0	0.001	1.48	3.49	6.36	10	10.5	11	0	0		
908	SQ	0	25	30.6	35.3	39.4	43.2	82	151	0	0		
909	SE	44	47	48	49	50	51	51.5	52				
910	KK	RTP425											
911	RD	1380	0.013	0.015		CIRC	2						
912	KK	TP440											
913	BA	0.089											
914	LS	0.1	98			73	0.1						
915	UK	50	0.02	0.1	32								
916	UK	450	0.013	0.3	68								
917	RD	1680	0.006	0.015		CIRC	2						
918	KK	CTP44											
919	HC	2											
920	KK	RTP442											
921	RD	1050	0.0057	0.015		CIRC	3						
922	KK	TP460											
923	BA	0.053											
924	LS	0.1	98			72	0.1						
925	UK	50	0.02	0.1	22								
926	UK	50	0.02	0.5	78								
927	RD	1840	0.006	0.04		TRAP	2	25					
928	KK	CTP46											
929	HC	3											
930	KK	RTP463											
931	RD	1400	0.0257	0.015		CIRC	4						
932	KK	TP470											
933	BA	0.09											
934	LS	0.1	98			73	0.1						
935	UK	50	0.02	0.1	27								
936	UK	100	0.02	0.5	73								
937	RD	2480	0.032	0.04		TRAP	2	25					
938	KK	CTP47											
939	HC	2											
940	KK	TPDET											
941	RS	1	FLOW	-									
942	SV	0	0.001	0.43	1.79	3.98	6.48	9.16	10	10.5			
943	SQ	0	13.8	15.4	16.9	18.2	19.5	20.7	58	126			
944	SE	90.4	95	96	97	98	99	100	100.5	101			
945	KO					22							
946	KK	CV150to East Jordan Canal											
947	RD	854.077	0.01500	0.015		CIRC	1.00	0.00					
948	KO					22							
949	KK	SB744											
950	BA	0.0176											
951	LS	0.1	98.000			81.000	0.1						

952	UK	30	.02	.1	35.00			
953	UK	100	.005	.4	65.00			
954	RD	300	.005	.04		TRAP	2	10
955	KO					22		
956	KK	SB745						
957	BA	0.0225						
958	LS	0.1	98.000			65.000	0.1	
959	UK	16	.02	.1	15.3			
960	UK	50	.02	.4	84.69			
961	RD	1000	.01	.015		CIRC	2.25	
962	KO					22		
963	KK	HC235						
964	HC	3						
965	KO					22		
966	KK	CV151to East Jordan Canal						
967	RD	533.677	0.01500	0.015		CIRC	2.25	0.00
968	KO					22		
969	KK	CV152to East Jordan Canal						
970	RD	521.197	0.00410	0.015		CIRC	2.50	0.00
971	KO					22		
972	KK	SB749						
973	BA	0.0308						
974	LS	0.1	98.000			70.000	0.1	
975	UK	23	.02	.1	27.85			
976	UK	70	.02	.4	72.15			
977	RD	500	.02	.02	0.0035	TRAP	2	25
978	RD	1000	.02	.015		CIRC	1.25	
979	KO					22		
980	KK	CV264to East Jordan Canal						
981	RD	678.145	0.02280	0.015		CIRC	1.25	0.00
982	KO					22		
983	KK	CV156to East Jordan Canal						
984	RD	267.013	0.02000	0.015		CIRC	1.25	0.00
985	KO					22		
986	KK	SB750						
987	BA	0.0196						
988	LS	0.1	98.000			70.000	0.1	
989	UK	23	.02	.1	26.27			
990	UK	70	.02	.4	73.73			
991	RD	300	.03	.02	.003	TRAP	2	25
992	RD	300	.03	.015		CIRC	1.25	
993	KO					22		
994	KK	HC240						
995	HC	2						
996	KO					22		
997	KK	CV158to East Jordan Canal						
998	RD	713.006	0.02000	0.015		CIRC	1.25	0.00
999	KO					22		
1000	KK	CV160to East Jordan Canal						
1001	RD	214.453	0.03700	0.015		CIRC	1.25	0.00
1002	KO					22		
1003	KK	SB748						
1004	BA	0.0358						
1005	LS	0.1	98.000			60.000	0.1	
1006	UK	25	.02	.1	20.43			
1007	UK	70	.02	.4	79.57			
1008	RD	300	.03	.02	.004	TRAP	2	25
1009	RD	1000	.03	.015		CIRC	1.25	
1010	KO					22		
1011	KK	SB747						
1012	BA	0.0557						

1013	LS	0.1	98.000			60.000	0.1			
1014	UK	25	.02	.1	27.85					
1015	UK	70	.02	.4	72.15					
1016	RD	300	.03	.02	.004	TRAP	2	25		
1017	RD	1200	.03	.02		TRAP	2	25		
1018	KO					22				
1019	KK	HC237								
1020	HC	3								
1021	KO					22				
1022	KK	DB3								
1023	RS		FLOW							
1024	SV	0	0.079	0.165	0.2581	0.358	0.466	0.58	0.909	0.9999
1025	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
1026	SQ	0	7.422	10.496	12.855	14.84	16.6	18.18	18.635	45.45
1027	KO					22				
1028	KK	CV162to East Jordan Canal								
1029	RD	512.875	0.00300	0.015		CIRC	1.50	0.00		
1030	KO					22				
1031	KK	SB752								
1032	BA	0.0278								
1033	LS	0.1	98.000			81.000	0.1			
1034	UK	30	.02	.1	35.00					
1035	UK	200	.01	.4	65.00					
1036	RD	500	.01	.02		TRAP	1	10		
1037	KO					22				
1038	KK	HC238								
1039	HC	2								
1040	KO					22				
1041	KK	DB4								
1042	RS		FLOW							
1043	SV	0	0.015	0.0323	0.0505	0.07	0.091	0.1136	0.178	0.1958
1044	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
1045	SQ	0	1.453	2.0554	2.5173	2.907	3.25	3.56	3.649	8.9
1046	KO					22				
1047	KK	CV154to East Jordan Canal								
1048	RD	117.467	0.00300	0.015		CIRC	1.25	0.00		
1049	KO					22				
1050	KK	SB746								
1051	BA	0.0317								
1052	LS	0.1	98.000			81.000	0.1			
1053	UK	15	.02	.1	35.00					
1054	UK	75	.01	.4	65.00					
1055	RD	500	.01	.04		TRAP	2	25		
1056	KO					22				
1057	KK	HC236								
1058	HC	3								
1059	KO					22				
1060	KK	DB5								
1061	RS		FLOW							
1062	SV	0	0.04	0.0833	0.1303	0.181	0.235	0.2929	0.459	0.5049
1063	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
1064	SQ	0	3.748	5.3001	6.4912	7.495	8.38	9.18	9.4095	22.95
1065	KO					22				
1066	KK	SB753								
1067	BA	0.0360								
1068	LS	0.1	98.000			75.000	0.1			
1069	UK	30	.02	.1	11.00					
1070	UK	150	.01	.4	89.00					
1071	RD	2000	.01	.02		TRAP	2	10		
1072	KO					22				
1073	KK	HC239								
1074	HC	2								

1075	KO					22		
1076	KK	CV155to East Jordan Canal						
1077	RD	1446.35	0.00500	0.015		CIRC	2.00	0.00
1078	KO					22		
1079	KK	SB549						
1080	BA	0.0297						
1081	LS	0.1	98.000			75.000	0.1	
1082	UK	30	.02	.1	11.00			
1083	UK	250	.01	.4	89.00			
1084	RD	1350	.005	.04		TRAP	10	2.
1085	KO					22		
1086	KK	HC233						
1087	HC	3						
1088	KO					22		
1089	KK	CV383East Jordan Canal						
1090	RD	1367.63	0.00050	0.030		TRAP	20.00	1.50
1091	KO					22		
1092	KK	SB74						
1093	BA	0.0357						
1094	LS	0.1	98.000			75.000	0.1	
1095	UK	15	.02	.1	11.00			
1096	UK	20	.05	.4	89.00			
1097	RD	2500	.005	.04		TRAP	2	5
1098	KO					22		
1099	KK	HC243						
1100	HC	2						
1101	KO					22		
1102	KK	CV723East Jordan Canal						
1103	RD	1096.28	0.00050	0.030		TRAP	20.00	1.50
1104	KO					22		
1105	KK	SB735						
1106	BA	0.0299						
1107	LS	0.1	98.000			65.000	0.1	
1108	UK	25	.02	.1	26.30			
1109	UK	90	.02	.4	73.70			
1110	RD	500	.005	.02	.005	TRAP	2	25
1111	RD	1000	.005	.015		CIRC	.83	
1112	KO					22		
1113	KK	CV382to East Jordan Canal						
1114	RD	604.441	0.02500	0.015		CIRC	1.25	0.00
1115	KO					22		
1116	KK	SB739						
1117	BA	0.0382						
1118	LS	0.1	98.000			61.380	0.1	
1119	UK	20	.02	.1	35.86			
1120	UK	70	.02	.4	64.14			
1121	RD	2000	.005	.02		TRAP	2	25
1122	KO					22		
1123	KK	HC244						
1124	HC	2						
1125	KO					22		
1126	KK	CV178to East Jordan Canal						
1127	RD	1282.61	0.00700	0.015		CIRC	1.25	0.00
1128	KO					22		
1129	KK	SB738						
1130	BA	0.0468						
1131	LS	0.1	98.000			63.783	0.1	
1132	UK	20	.02	.1	23.67			
1133	UK	70	.02	.4	76.33			
1134	RD	700	.005	.02	0.005	TRAP	2	25
1135	RD	700	.005	.015		CIRC	1.25	

1136	KO												22
1137	KK	CV179to East Jordan Canal											
1138	RD	267.740	0.01720	0.015									CIRC 1.50 0.00
1139	KO												22
1140	KK	HC245											
1141	HC	2											
1142	KO												22
1143	KK	CV176to East Jordan Canal											
1144	RD	449.165	0.00700	0.015									CIRC 2.00 0.00
1145	KO												22
1146	KK	SB740											
1147	BA	0.0294											
1148	LS	0.1	98.000					65.000		0.1			
1149	UK	20	.02	.1	27.69								
1150	UK	70	.02	.4	72.3								
1151	RD	400	.008	.02	.005								TRAP 2 25
1152	RD	1400	.005	.02									TRAP 2 25
1153	KO												22
1154	KK	CV164to East Jordan Canal											
1155	RD	258.216	0.00500	0.015									CIRC 1.50 0.00
1156	KO												22
1157	KK	CV165to East Jordan Canal											
1158	RD	758.271	0.00500	0.015									CIRC 1.50 0.00
1159	KO												22
1160	KK	CV166to East Jordan Canal											
1161	RD	163.804	0.01200	0.015									CIRC 1.50 0.00
1162	KO												22
1163	KK	CV167to East Jordan Canal											
1164	RD	167.318	0.00500	0.015									CIRC 1.50 0.00
1165	KO												22
1166	KK	HC246											
1167	HC	2											
1168	KO												22
1169	KK	DB6											
1170	RS		FLOW										
1171	SV	0	0.085	0.1775	0.2777	0.385	0.501	0.624	0.978	1.0758			
1172	SE	0	0.5	1	1.5	2	2.5	3	4	4.5			
1173	SQ	0	7.985	11.293	13.831	15.97	17.86	19.56	20.049	48.9			
1174	KO												22
1175	KK	CV168to East Jordan Canal											
1176	RD	120.059	0.00100	0.015									CIRC 1.00 0.00
1177	KO												22
1178	KK	HC242											
1179	HC	2											
1180	KO												22
1181	KK	CV724East Jordan Canal											
1182	RD	416.974	0.00050	0.030									TRAP 20.00 1.50
1183	KO												22
1184	KK	SB657											
1185	BA	0.0173											
1186	LS	0.1	98.000					75.000		0.1			
1187	UK	30	.02	.1	35.00								
1188	UK	200	.02	.4	65.00								
1189	RD	2000	.005	.04									TRAP 10 2
1190	KO												22
1191	KK	HC24											
1192	HC	2											
1193	KO												22

1194	KK	CV378East Jordan Canal												
1195	RD	1901.09	0.00050	0.030		TRAP	20.00	1.50						
1196	KO					22								
1197	KK	SB736												
1198	BA	0.0263												
1199	LS	0.1	98.000			64.143	0.1							
1200	UK	20	.02	.1	28.96									
1201	UK	70	.02	.4	71.04									
1202	RD	425	.005	.02	0.006	TRAP	2	25						
1203	RD	775	.005	.015		CIRC	1.25							
1204	KO					22								
1205	KK	CV182to East Jordan Canal												
1206	RD	439.106	0.00200	0.015		CIRC	1.25	0.00						
1207	KO					22								
1208	KK	SB737												
1209	BA	0.0359												
1210	LS	0.1	98.000			63.265	0.1							
1211	UK	20	.02	.1	24.37									
1212	UK	70	.02	.4	75.63									
1213	RD	300	.005	.02	0.005	TRAP	2	25						
1214	RD	500	.005	.015		CIRC	1.25							
1215	KO					22								
1216	KK	HC249												
1217	HC	2												
1218	KO					22								
1219	KK	CV184to East Jordan Canal												
1220	RD	809.881	0.00400	0.015		CIRC	2.00	0.00						
1221	KO					22								
1222	KK	CV186to East Jordan Canal												
1223	RD	199.074	0.00500	0.015		CIRC	2.00	0.00						
1224	KO					22								
1225	KK	SB655												
1226	BA	0.0423												
1227	LS	0.1	98.000			78.947	0.1							
1228	UK	20	.02	.1	27.62									
1229	UK	70	.02	.4	72.38									
1230	RD	500	.005	.02	0.01	TRAP	2	25						
1231	RD	1000	.005	.015		CIRC	1.5							
1232	KO					22								
1233	KK	CV189to East Jordan Canal												
1234	RD	807.576	0.00100	0.015		CIRC	2.00	0.00						
1235	KO					22								
1236	KK	HC248												
1237	HC	2												
1238	KO					22								
1239	KK	DB7												
1240	RS		FLOW											
1241	SV	0	0.063	0.1316	0.2058	0.286	0.371	0.4626	0.725	0.7975				
1242	SE	0	0.5	1	1.5	2	2.5	3	4	4.5				
1243	SQ	0	5.92	8.3716	10.253	11.84	13.24	14.5	14.863	36.25				
1244	KO					22								
1245	KK	CV380to East Jordan Canal												
1246	RD	122.827	0.00100	0.015		CIRC	1.00	0.00						
1247	KO					22								
1248	KK	HC247												
1249	HC	2												
1250	KO					22								
1251	KK	CV379East Jordan Canal												
1252	RD	607.516	0.00050	0.030		TRAP	20.00	1.50						
1253	KO					22								

1254	KK	SB255									
1255	BA	0.0099									
1256	LS	0.1	98.000			86.000	0.1				
1257	UK	30	.02	.1	60.00						
1258	UK	350	.01	.4	40.00						
1259	RD	550	.005	.04		TRAP	12		2		
1260	KO					22					
1261	KK	HC250									
1262	HC	2									
1263	KO					22					
1264	KK	CV499East Jordan Canal									
1265	RD	1797.69	0.00050	0.030		TRAP	20.00		1.50		
1266	KO					22					
1267	KK	DH37									
1268	DT	DR37									
1269	DI	0	25	50	100	200	400		800		
1270	DQ	0	25	50	100	200	400		800		
1271	KO					22					
1272	KK	EJS010									
1273	PB	1.06									
1274	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1275	PI	.015	.015	.134	.160	.095	.051	.051	.051	.035	.035
1276	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1277	PI	.010	.010	.010	.010	.010	.010				
1278	BA	.109									
1279	LU	.46	.73	36.10							
1280	IN	5									
1281	UI	1.	1.	1.	1.	1.	1.	1.	2.	2.	4.
1282	UI	4.	7.	7.	16.	16.	27.	27.	36.	36.	46.
1283	UI	46.	56.	56.	66.	66.	66.	77.	77.	88.	88.
1284	UI	98.	98.	109.	109.	120.	120.	112.	112.	104.	104.
1285	UI	96.	96.	88.	88.	80.	80.	71.	71.	63.	63.
1286	UI	63.	54.	54.	47.	47.	40.	40.	35.	35.	32.
1287	UI	32.	28.	28.	25.	25.	23.	23.	21.	21.	19.
1288	UI	19.	18.	18.	18.	16.	16.	15.	15.	14.	14.
1289	UI	13.	13.	12.	12.	12.	12.	11.	11.	10.	10.
1290	UI	10.	10.	9.	9.	9.	9.	8.	8.	8.	8.
1291	UI	8.	7.	7.	7.	7.	7.	7.	7.	7.	6.
1292	UI	6.	6.	6.	6.	6.	5.	5.	5.	5.	5.
1293	UI	5.	5.	5.	5.	5.	5.	4.	4.	4.	4.
1294	UI	4.	4.	4.	4.	4.	4.	4.	4.	3.	3.
1295	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1296	UI	3.	3.	3.	2.	2.	2.	2.	2.	2.	2.
1297	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1298	UI	2.	2.	2.	2.	2.	2.	2.	2.	1.	1.
1299	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1300	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1301	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1302	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1303	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1304	UI	1.	1.	1.	1.	1.	1.	0.	0.	0.	0.
1305	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1306	UI	0.									
1307	KK	RJS01									
	*										
	*	NOTE: FULL PIPE CAPACITY IS 25.9-CFS.									
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1308	RD	920.	.0174	.0150		CIRC	2.00		.00		
1309	KK	EJS020									
1310	PB	1.05									
1311	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1312	PI	.015	.015	.133	.159	.094	.051	.051	.051	.035	.035
1313	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1314	PI	.010	.010	.010	.010	.010	.010				
1315	BA	.045									
1316	LU	.61	.67	24.45							

1317	IN	5									
1318	UI	0.	1.	1.	1.	1.	2.	2.	5.	11.	18.
1319	UI	25.	25.	31.	38.	45.	52.	52.	59.	66.	74.
1320	UI	74.	81.	76.	70.	65.	65.	59.	54.	48.	43.
1321	UI	43.	37.	31.	27.	27.	24.	21.	19.	17.	17.
1322	UI	16.	14.	13.	12.	12.	11.	10.	10.	10.	9.
1323	UI	8.	8.	7.	7.	7.	7.	6.	6.	6.	6.
1324	UI	5.	5.	5.	5.	5.	4.	4.	4.	4.	4.
1325	UI	4.	4.	4.	3.	3.	3.	3.	3.	3.	3.
1326	UI	3.	3.	3.	2.	2.	2.	2.	2.	2.	2.
1327	UI	2.	2.	2.	2.	2.	2.	2.	1.	1.	1.
1328	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1329	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1330	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	0.
1331	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1332	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1333	UI	0.	0.	0.							
1334	KK	CJS02									
1335	HC	2									
1336	KK	RJS022									
	*										
	*	* NOTE: FULL PIPE CAPACITY IS 21.5-CFS.									
	*	* HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	* OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1337	RD	1830.	.0120	.0150		CIRC	2.00	.00			
1338	KK	RJS023									
1339	RD	1450.	.0014	.0450		TRAP	6.00	1.00			
1340	KK	EJS030									
1341	BA	.035									
1342	LU	.71	.61	30.60							
1343	IN	5									
1344	UI	0.	1.	1.	1.	1.	2.	4.	9.	16.	21.
1345	UI	27.	27.	33.	39.	45.	51.	58.	64.	70.	70.
1346	UI	66.	61.	56.	51.	47.	42.	37.	37.	32.	27.
1347	UI	24.	21.	19.	17.	15.	15.	14.	12.	11.	10.
1348	UI	10.	9.	8.	8.	8.	7.	7.	6.	6.	6.
1349	UI	5.	5.	5.	5.	5.	4.	4.	4.	4.	4.
1350	UI	4.	4.	3.	3.	3.	3.	3.	3.	3.	3.
1351	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1352	UI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
1353	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1354	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1355	UI	1.	1.	0.	0.	0.	0.	0.	0.	0.	0.
1356	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1357	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1358	KK	EJS040									
1359	BA	.052									
1360	LU	.20	.64	75.00							
1361	IN	5									
1362	UI	0.	0.	0.	0.	0.	1.	1.	1.	1.	2.
1363	UI	2.	3.	3.	8.	8.	13.	13.	18.	18.	22.
1364	UI	22.	27.	27.	32.	32.	37.	37.	43.	43.	48.
1365	UI	48.	53.	53.	58.	58.	54.	54.	51.	51.	46.
1366	UI	46.	43.	43.	39.	39.	34.	34.	31.	31.	26.
1367	UI	26.	23.	23.	20.	20.	17.	17.	15.	15.	14.
1368	UI	14.	12.	12.	11.	11.	10.	10.	9.	9.	9.
1369	UI	9.	9.	8.	8.	7.	7.	7.	7.	6.	6.
1370	UI	6.	6.	6.	6.	5.	5.	5.	5.	5.	5.
1371	UI	4.	4.	4.	4.	4.	4.	4.	4.	4.	4.
1372	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1373	UI	3.	3.	3.	3.	3.	3.	2.	2.	2.	2.
1374	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1375	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1376	UI	2.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1377	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1378	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1379	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1380	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.

1381	UI	1.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1382	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1383	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1384	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1385	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1386	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1387	KK	CJS1025									
1388	HC	3									
1389	KO	22									
1390	KK	HC338									
1391	HC	2									
1392	KO	22									
1393	KK	CV863East Jordan Canal									
1394	RD	2829.16	0.00050	0.030		TRAP	20.00	1.50			
1395	KO	22									
1396	KK	CP010									
1397	PB	1.06									
1398	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1399	PI	.015	.015	.134	.161	.095	.051	.051	.051	.035	.035
1400	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1401	PI	.010	.010	.010	.010	.010	.010				
1402	BA	.073									
1403	LU	.54	.68	36.26							
1404	IN	5									
1405	UI	0.	0.	1.	1.	1.	1.	2.	2.	3.	3.
1406	UI	5.	5.	12.	12.	20.	27.	27.	34.	34.	42.
1407	UI	42.	49.	49.	57.	57.	65.	65.	73.	73.	81.
1408	UI	89.	89.	83.	83.	78.	78.	71.	71.	65.	65.
1409	UI	59.	59.	53.	47.	47.	40.	40.	35.	35.	30.
1410	UI	30.	26.	26.	24.	24.	21.	21.	19.	17.	17.
1411	UI	16.	16.	14.	14.	13.	13.	12.	12.	11.	11.
1412	UI	11.	11.	10.	9.	9.	9.	9.	8.	8.	8.
1413	UI	8.	7.	7.	7.	7.	6.	6.	6.	6.	6.
1414	UI	6.	6.	5.	5.	5.	5.	5.	5.	5.	5.
1415	UI	4.	4.	4.	4.	4.	4.	4.	4.	4.	4.
1416	UI	4.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1417	UI	3.	3.	3.	3.	3.	3.	2.	2.	2.	2.
1418	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1419	UI	2.	2.	2.	2.	2.	2.	2.	2.	1.	1.
1420	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1421	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1422	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1423	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1424	UI	1.	1.	1.	1.	1.	1.	1.	0.	0.	0.
1425	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1426	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1427	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1428	KK	RCP01									
	*										
	*	NOTE: FULL PIPE CAPACITY IS 34.6-CFS.									
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1429	RD	970.	.0309	.0150		CIRC	2.00	.00			
1430	KK	CP020									
1431	PB	1.05									
1432	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1433	PI	.015	.015	.133	.159	.094	.051	.051	.051	.035	.035
1434	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1435	PI	.010	.010	.010	.010	.010	.010				
1436	BA	.042									
1437	LU	.71	.70	9.55							
1438	IN	5									
1439	UI	0.	1.	1.	1.	1.	2.	5.	11.	18.	18.
1440	UI	24.	31.	38.	45.	52.	59.	59.	66.	73.	81.
1441	UI	75.	70.	64.	64.	59.	54.	48.	43.	37.	37.
1442	UI	31.	27.	24.	21.	19.	17.	17.	16.	14.	13.
1443	UI	12.	11.	11.	10.	9.	9.	8.	8.	7.	7.

1444	UI	7.	7.	6.	6.	6.	6.	5.	5.	5.	5.
1445	UI	4.	4.	4.	4.	4.	4.	4.	3.	3.	3.
1446	UI	3.	3.	3.	3.	3.	3.	3.	2.	2.	2.
1447	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1448	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1449	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1450	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	0.
1451	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1452	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1453	UI	0.	0.								
1454	KK	CP030									
1455	BA	.104									
1456	LU	.48	.63	7.30							
1457	IN	5									
1458	UI	0.	0.	0.	0.	1.	1.	1.	1.	1.	1.
1459	UI	1.	1.	1.	1.	2.	2.	2.	2.	4.	4.
1460	UI	4.	10.	10.	10.	16.	16.	16.	22.	22.	22.
1461	UI	22.	28.	28.	28.	34.	34.	34.	40.	40.	40.
1462	UI	47.	47.	47.	47.	53.	53.	53.	60.	60.	60.
1463	UI	66.	66.	66.	66.	73.	73.	73.	68.	68.	68.
1464	UI	63.	63.	63.	58.	58.	58.	58.	53.	53.	53.
1465	UI	48.	48.	48.	43.	43.	43.	43.	38.	38.	38.
1466	UI	33.	33.	33.	28.	28.	28.	24.	24.	24.	24.
1467	UI	22.	22.	22.	19.	19.	19.	17.	17.	17.	15.
1468	UI	15.	15.	15.	14.	14.	14.	13.	13.	13.	12.
1469	UI	12.	12.	12.	11.	11.	11.	10.	10.	10.	9.
1470	UI	9.	9.	9.	9.	9.	9.	8.	8.	8.	8.
1471	UI	8.	8.	7.	7.	7.	7.	7.	7.	7.	6.
1472	UI	6.	6.	6.	6.	6.	6.	6.	6.	6.	5.
1473	UI	5.	5.	5.	5.	5.	5.	5.	5.	5.	5.
1474	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	4.
1475	UI	4.	4.	4.	4.	4.	4.	4.	4.	3.	3.
1476	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1477	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1478	UI	3.	3.	3.	3.	2.	2.	2.	2.	2.	2.
1479	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1480	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1481	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1482	UI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
1483	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1484	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1485	UI	1.	1.	1.							
1486	KK	CCP03									
1487	HC	3									
1488	KK	RCP032									
	*										
	*	NOTE: FULL PIPE CAPACITY IS	51.9-CFS.								
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1489	RD	850.	.0212	.0150		CIRC	2.50	.00			
1490	KK	CRSTD									
1491	KM	CRESCENT POND									
1492	RS	1	FLOW	-							
1493	SV	.0	.0	.4	1.1	2.3	3.9	4.2	4.5		
1494	SQ	.0	21.6	27.9	33.0	37.4	41.4	80.0	150.0		
1495	SE	74.5	77.0	78.0	79.0	80.0	81.0	81.5	82.0		
1496	KK	RCP034									
	*										
	*	NOTE: FULL PIPE CAPACITY IS	24.9-CFS.								
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1497	RD	750.	.0160	.0150		CIRC	2.00	.00			
1498	KK	EJS050									
1499	PB	1.05									
1500	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1501	PI	.015	.015	.133	.159	.094	.051	.051	.051	.035	.035

1502	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1503	PI	.010	.010	.010	.010	.010	.010				
1504	BA	.094									
1505	LU	.32	.65	17.03							
1506	IN	5									
1507	UI	1.	1.	1.	1.	1.	1.	2.	2.	3.	3.
1508	UI	6.	6.	15.	15.	25.	25.	34.	34.	43.	43.
1509	UI	52.	52.	62.	62.	72.	72.	82.	92.	92.	102.
1510	UI	102.	112.	112.	104.	104.	97.	97.	89.	89.	82.
1511	UI	82.	74.	74.	66.	66.	59.	59.	51.	51.	43.
1512	UI	43.	37.	37.	33.	29.	29.	26.	26.	24.	24.
1513	UI	22.	22.	20.	20.	18.	18.	17.	17.	15.	15.
1514	UI	14.	14.	13.	13.	12.	12.	12.	12.	11.	11.
1515	UI	10.	10.	10.	9.	9.	9.	9.	8.	8.	8.
1516	UI	8.	7.	7.	7.	7.	7.	7.	6.	6.	6.
1517	UI	6.	6.	6.	6.	6.	5.	5.	5.	5.	5.
1518	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	4.
1519	UI	4.	4.	4.	4.	3.	3.	3.	3.	3.	3.
1520	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
1521	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1522	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1523	UI	2.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1524	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1525	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1526	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1527	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1528	UI	1.	1.	1.	1.	1.	0.	0.	0.	0.	0.
1529	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1530	UI	0.	0.								
1531	KK	CCP1028									
1532	HC	2									
1533	KO					22					
1534	KK	HC339									
1535	HC	2									
1536	KO					22					
1537	KK	CV738East Jordan Canal									
1538	RD	796.779 0.00050 0.030				TRAP	20.00	1.50			
1539	KO					22					
1540	KK	EJS060									
1541	PB	1.05									
1542	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1543	PI	.015	.015	.133	.159	.094	.051	.051	.051	.035	.035
1544	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1545	PI	.010	.010	.010	.010	.010	.010				
1546	BA	.072									
1547	LU	.54	.68	15.75							
1548	IN	5									
1549	UI	1.	1.	2.	3.	5.	10.	23.	53.	68.	82.
1550	UI	97.	113.	128.	144.	160.	175.	164.	153.	140.	128.
1551	UI	116.	104.	79.	68.	59.	52.	46.	41.	37.	34.
1552	UI	31.	28.	26.	24.	22.	21.	19.	17.	16.	15.
1553	UI	14.	13.	13.	12.	11.	11.	10.	10.	10.	9.
1554	UI	9.	8.	8.	7.	7.	7.	6.	6.	6.	6.
1555	UI	5.	5.	5.	5.	5.	4.	4.	4.	4.	4.
1556	UI	3.	3.	3.	3.	3.	3.	3.	2.	2.	2.
1557	UI	2.	2.	2.	2.	2.	2.	2.	2.	1.	1.
1558	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1559	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1560	UI	1.	1.	0.							
1561	KO					22					
1562	KK	HC340									
1563	HC	2									
1564	KO					22					
1565	KK	CV739East Jordan Canal									
1566	RD	2205.41 0.00050 0.030				TRAP	20.00	1.50			
1567	KO					22					
1568	KK	HC34									

1569	HC	2									
1570	KO					22					
1571	KK	EJS090									
1572	PB	1.05									
1573	PI	.010	.010	.010	.010	.010	.015	.015	.015	.015	
1574	PI	.015	.015	.133	.159	.094	.051	.051	.035	.035	
1575	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	
1576	PI	.010	.010	.010	.010	.010	.010				
1577	BA	.043									
1578	LU	.80	.60	1.00							
1579	IN	5									
1580	UI	0.	0.	0.	0.	0.	0.	0.	0.	1.	
1581	UI	1.	1.	1.	1.	1.	2.	2.	5.	5.	
1582	UI	8.	8.	8.	11.	11.	11.	14.	14.	17.	
1583	UI	17.	20.	20.	20.	23.	23.	26.	26.	29.	
1584	UI	29.	29.	33.	33.	33.	36.	36.	33.	33.	
1585	UI	31.	31.	31.	29.	29.	29.	26.	26.	24.	
1586	UI	24.	21.	21.	21.	19.	19.	19.	16.	16.	
1587	UI	14.	14.	12.	12.	12.	11.	11.	11.	9.	
1588	UI	8.	8.	8.	8.	8.	8.	7.	7.	6.	
1589	UI	6.	6.	6.	6.	5.	5.	5.	5.	5.	
1590	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	
1591	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	
1592	UI	3.	3.	3.	3.	3.	3.	3.	2.	2.	
1593	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	
1594	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	
1595	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	
1596	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1597	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1598	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1599	KO					22					
1600	KK	CV815East Jordan Canal									
1601	RD	3022.50	0.00050	0.030		TRAP	20.00	1.50			
1602	KO					22					
1603	KK	DH4									
1604	DT	DR4									
1605	DI	0	25	50	100	200	400	800			
1606	DQ	0	25	50	100	200	400	800			
1607	KO					22					
1608	KK	CV816East Jordan Canal									
1609	RD	1958.06	0.00050	0.030		TRAP	20.00	1.50			
1610	KO					22					
1611	KK	EJC003									
1612	PB	1.08									
1613	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	
1614	PI	.015	.015	.138	.185	.095	.051	.051	.035	.035	
1615	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	
1616	PI	.010	.010	.010	.010	.010	.010				
1617	BA	.088									
1618	LU	.40	.75	40.00							
1619	IN	5									
1620	UI	1.	1.	1.	1.	1.	2.	2.	4.	7.	
1621	UI	17.	29.	29.	39.	39.	49.	59.	59.	70.	
1622	UI	82.	93.	93.	104.	116.	116.	127.	119.	119.	
1623	UI	101.	101.	93.	93.	84.	75.	75.	67.	58.	
1624	UI	49.	49.	43.	38.	38.	34.	30.	30.	27.	
1625	UI	24.	22.	22.	20.	19.	19.	17.	16.	16.	
1626	UI	15.	14.	13.	13.	12.	12.	12.	11.	11.	
1627	UI	10.	10.	9.	9.	9.	8.	8.	8.	8.	
1628	UI	7.	7.	7.	7.	6.	6.	6.	6.	6.	
1629	UI	6.	5.	5.	5.	5.	5.	5.	4.	4.	
1630	UI	4.	4.	4.	4.	4.	4.	4.	3.	3.	
1631	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	
1632	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	
1633	UI	2.	2.	2.	2.	2.	2.	2.	1.	1.	
1634	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1635	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1636	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1637	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	

1638	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1639	UI	0.									
1640	KK	RJC004									
1641	RD	1600.	.0025	.0500		TRAP	2.00	1.00			
1642	KK	EJC005									
1643	BA	.049									
1644	LU	.40	.75	40.00							
1645	IN	5									
1646	UI	0.	0.	1.	1.	1.	1.	1.	2.	4.	4.
1647	UI	9.	16.	16.	22.	27.	27.	33.	33.	39.	46.
1648	UI	46.	52.	58.	58.	65.	71.	71.	67.	67.	62.
1649	UI	57.	57.	52.	47.	47.	42.	42.	38.	32.	32.
1650	UI	28.	24.	24.	21.	19.	19.	17.	17.	15.	14.
1651	UI	14.	12.	11.	11.	11.	10.	10.	9.	9.	8.
1652	UI	8.	8.	7.	7.	7.	7.	6.	6.	6.	6.
1653	UI	5.	5.	5.	5.	5.	5.	4.	4.	4.	4.
1654	UI	4.	4.	4.	4.	4.	3.	3.	3.	3.	3.
1655	UI	3.	3.	3.	3.	3.	3.	3.	3.	2.	2.
1656	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1657	UI	2.	2.	2.	2.	2.	1.	1.	1.	1.	1.
1658	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1659	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1660	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1661	UI	1.	1.	1.	1.	1.	1.	0.	0.	0.	0.
1662	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1663	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1664	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1665	KK	CJC006									
1666	HC	2									
1667	KO					22					
1668	KK	HC343									
1669	HC	2									
1670	KO					22					
1671	KK	CV742East Jordan Canal									
1672	RD	1702.14	0.00050	0.030		TRAP	20.00	1.50			
1673	KO					22					
1674	KK	EJC010									
1675	PB	1.08									
1676	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1677	PI	.015	.015	.138	.185	.095	.051	.051	.051	.035	.035
1678	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1679	PI	.010	.010	.010	.010	.010	.010				
1680	BA	.020									
1681	LU	.74	.60	66.80							
1682	IN	5									
1683	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1684	UI	0.	1.	1.	1.	1.	2.	2.	2.	4.	4.
1685	UI	6.	6.	6.	7.	7.	9.	9.	9.	10.	10.
1686	UI	12.	12.	12.	14.	14.	16.	16.	16.	17.	17.
1687	UI	19.	19.	19.	18.	18.	16.	16.	15.	15.	15.
1688	UI	14.	14.	13.	13.	13.	11.	11.	10.	10.	10.
1689	UI	9.	9.	7.	7.	7.	6.	6.	6.	6.	6.
1690	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	3.
1691	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
1692	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1693	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	1.
1694	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1695	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1696	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1697	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1698	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1699	UI	1.	1.	1.	1.	1.	1.	1.	1.	0.	0.
1700	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1701	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1702	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1703	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1704	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1705	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

1706	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1707	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1708	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1709	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1710	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1711	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1712	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1713	KK	REJ01									
	*										
	*	NOTE: FULL PIPE CAPACITY IS	6.2-CFS.								
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
1714	RD	870.	.0046	.0150		CIRC	1.50	.00			
1715	KK	REJ012									
1716	RD	2350.	.0009	.0450		TRAP	6.00	1.00			
1717	KK	EJC020									
1718	BA	.058									
1719	LU	.77	.60	48.00							
1720	IN	5									
1721	UI	0.	0.	0.	0.	0.	1.	1.	1.	1.	1.
1722	UI	1.	2.	2.	2.	3.	3.	7.	7.	7.	11.
1723	UI	11.	11.	15.	15.	20.	20.	20.	24.	24.	28.
1724	UI	28.	28.	33.	33.	37.	37.	37.	42.	42.	42.
1725	UI	46.	46.	51.	51.	51.	48.	48.	44.	44.	44.
1726	UI	41.	41.	41.	37.	37.	34.	34.	34.	30.	30.
1727	UI	27.	27.	27.	23.	23.	23.	20.	20.	17.	17.
1728	UI	17.	15.	15.	13.	13.	13.	12.	12.	12.	11.
1729	UI	11.	10.	10.	10.	9.	9.	8.	8.	8.	8.
1730	UI	8.	7.	7.	7.	6.	6.	6.	6.	6.	6.
1731	UI	6.	6.	5.	5.	5.	5.	5.	5.	5.	5.
1732	UI	4.	4.	4.	4.	4.	4.	4.	4.	4.	4.
1733	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1734	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1735	UI	3.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1736	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1737	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1738	UI	2.	2.	1.	1.	1.	1.	1.	1.	1.	1.
1739	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1740	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1741	KK	CEJ02									
	*	COMBINES EJC010 WITH EJC020									
1742	HC	2									
1743	KO					22					
1744	KK	HC344									
1745	HC	2									
1746	KO					22					
1747	KK	CV743East Jordan Canal									
1748	RD	3645.25	0.00050	0.030		TRAP	20.00	1.50			
1749	KO					22					
1750	KK	EJC040									
1751	PB	1.07									
1752	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1753	PI	.015	.015	.137	.183	.094	.050	.050	.050	.035	.035
1754	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1755	PI	.010	.010	.010	.010	.010	.010				
1756	BA	.052									
1757	LU	.20	.56	95.00							
1758	IN	5									
1759	UI	0.	0.	1.	1.	1.	1.	2.	2.	4.	10.
1760	UI	10.	17.	23.	23.	30.	30.	36.	43.	43.	50.
1761	UI	57.	57.	64.	70.	70.	77.	72.	72.	67.	62.
1762	UI	62.	57.	51.	51.	46.	41.	41.	35.	30.	30.
1763	UI	26.	23.	23.	20.	20.	18.	16.	16.	15.	14.
1764	UI	14.	12.	12.	12.	11.	10.	10.	9.	9.	9.
1765	UI	8.	8.	8.	7.	7.	7.	6.	6.	6.	6.
1766	UI	6.	5.	5.	5.	5.	5.	5.	4.	4.	4.

1767	VI	4.	4.	4.	4.	4.	4.	3.	3.	3.	3.	
1768	VI	3.	3.	3.	3.	3.	3.	3.	3.	2.	2.	
1769	VI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.	
1770	VI	2.	2.	2.	2.	2.	2.	1.	1.	1.	1.	
1771	VI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1772	VI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1773	VI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1774	VI	1.	1.	1.	0.	0.	0.	0.	0.	0.	0.	
1775	VI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	
1776	VI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	
1777	VI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	
1778	KO											
						22						
1779	KK	CV745East Jordan Canal										
1780	RD	1181.91	0.00050	0.030		TRAP	20.00	1.50				
1781	KO					22						
1782	KK	EJC050										
1783	PB	1.07										
1784	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015	
1785	PI	.015	.015	.137	.183	.094	.050	.050	.050	.035	.035	
1786	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015	
1787	PI	.010	.010	.010	.010	.010	.010					
1788	BA	.049										
1789	LU	.36	.73	48.97								
1790	IN	5										
1791	VI	1.	1.	1.	2.	3.	6.	15.	25.	34.	43.	
1792	VI	53.	62.	72.	82.	92.	103.	113.	105.	98.	90.	
1793	VI	82.	75.	67.	59.	51.	44.	38.	33.	30.	27.	
1794	VI	24.	22.	20.	18.	17.	15.	14.	13.	12.	12.	
1795	VI	11.	10.	10.	9.	9.	8.	8.	7.	7.	7.	
1796	VI	6.	6.	6.	6.	5.	5.	5.	5.	5.	4.	
1797	VI	4.	4.	4.	4.	3.	3.	3.	3.	3.	3.	
1798	VI	3.	3.	2.	2.	2.	2.	2.	2.	2.	2.	
1799	VI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.	
1800	VI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1801	VI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.	
1802	VI	1.	0.	0.	0.	0.	0.	0.	0.	0.	0.	
1803	KO											
						22						
1804	KK	HC347										
1805	HC	2										
1806	KO											
						22						
1807	KK	CV820East Jordan Canal										
1808	RD	1305.97	0.00050	0.030		TRAP	20.00	1.50				
1809	KO					22						
1810	KK	DH6										
1811	DT	DR6										
1812	DI	0	25	50	100	200	400	800				
1813	DQ	0	25	50	100	200	400	800				
1814	KO											
						22						
1815	KK	CV821East Jordan Canal										
1816	RD	1646.58	0.00050	0.030		TRAP	20.00	1.50				
1817	KO					22						
1818	KK	EJC055										
1819	PB	1.06										
1820	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015	
1821	PI	.015	.015	.136	.176	.094	.050	.050	.050	.035	.035	
1822	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015	
1823	PI	.010	.010	.010	.010	.010	.010					
1824	BA	.114										
1825	LU	.40	.75	45.49								
1826	IN	5										
1827	VI	1.	1.	1.	1.	2.	2.	2.	2.	4.	4.	
1828	VI	8.	8.	18.	18.	30.	30.	41.	41.	52.	52.	
1829	VI	63.	63.	74.	74.	86.	86.	98.	98.	111.	111.	
1830	VI	123.	135.	135.	126.	126.	117.	117.	108.	108.	98.	
1831	VI	98.	89.	89.	80.	80.	71.	71.	61.	61.	52.	
1832	VI	52.	45.	45.	40.	40.	36.	36.	32.	32.	28.	
1833	VI	28.	26.	26.	24.	22.	22.	20.	20.	18.	18.	

1834	UI	17.	17.	16.	16.	15.	15.	14.	14.	13.	13.
1835	UI	12.	12.	12.	12.	11.	11.	10.	10.	10.	10.
1836	UI	9.	9.	9.	9.	8.	8.	8.	8.	8.	7.
1837	UI	7.	7.	7.	7.	7.	6.	6.	6.	6.	6.
1838	UI	6.	6.	6.	5.	5.	5.	5.	5.	5.	5.
1839	UI	5.	5.	5.	4.	4.	4.	4.	4.	4.	4.
1840	UI	4.	4.	4.	4.	3.	3.	3.	3.	3.	3.
1841	UI	3.	3.	3.	3.	3.	3.	3.	3.	2.	2.
1842	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1843	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1844	UI	2.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1845	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1846	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1847	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1848	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1849	UI	1.	1.	1.	0.	0.	0.	0.	0.	0.	0.
1850	UI	0.	0.	0.							
1851	KK	RJC056									
1852	KM	TO EJC 065									
1853	RD	700.	.0014	.0450			TRAP 6.00	1.00			
1854	KO						22				
1855	KK	HC348									
1856	HC	2									
1857	KO						22				
1858	KK	CV747East Jordan Canal									
1859	RD	760.388	0.00050	0.030			TRAP 20.00	1.50			
1860	KO						22				
1861	KK	HC349									
1862	HC	2									
1863	KO						22				
1864	KK	EJC075									
1865	PB	1.06									
1866	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1867	PI	.015	.015	.136	.176	.094	.050	.050	.050	.035	.035
1868	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1869	PI	.010	.010	.010	.010	.010	.010				
1870	BA	.032									
1871	LU	.40	.75	40.00							
1872	IN	5									
1873	UI	0.	0.	0.	1.	1.	1.	1.	3.	3.	7.
1874	UI	11.	11.	15.	19.	19.	23.	27.	27.	32.	36.
1875	UI	36.	41.	45.	50.	50.	46.	43.	43.	40.	36.
1876	UI	36.	33.	29.	29.	26.	23.	23.	19.	17.	15.
1877	UI	15.	13.	12.	12.	10.	10.	10.	9.	8.	8.
1878	UI	7.	7.	7.	6.	6.	5.	5.	5.	5.	5.
1879	UI	5.	4.	4.	4.	4.	4.	4.	3.	3.	3.
1880	UI	3.	3.	3.	3.	3.	3.	3.	2.	2.	2.
1881	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1882	UI	2.	2.	2.	2.	2.	1.	1.	1.	1.	1.
1883	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1884	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1885	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1886	UI	1.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1887	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1888	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1889	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1890	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1891	KO						22				
1892	KK	CV750East Jordan Canal									
1893	RD	467.771	0.00050	0.030			TRAP 20.00	1.50			
1894	KO						22				
1895	KK	EJC080									
1896	PB	1.06									
1897	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1898	PI	.015	.015	.136	.176	.094	.050	.050	.050	.035	.035
1899	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1900	PI	.010	.010	.010	.010	.010	.010				

1901	BA	.088									
1902	LU	.40	.75	40.50							
1903	IN	5									
1904	UI	1.	1.	1.	1.	1.	2.	2.	4.	7.	7.
1905	UI	17.	29.	29.	39.	49.	49.	60.	60.	71.	82.
1906	UI	82.	93.	105.	105.	116.	128.	128.	119.	119.	111.
1907	UI	102.	102.	93.	85.	85.	76.	67.	67.	58.	58.
1908	UI	50.	43.	43.	38.	34.	34.	30.	27.	27.	25.
1909	UI	25.	22.	20.	20.	19.	18.	18.	16.	15.	15.
1910	UI	14.	14.	13.	12.	12.	12.	11.	11.	10.	10.
1911	UI	10.	9.	9.	9.	8.	8.	8.	8.	8.	7.
1912	UI	7.	7.	7.	7.	6.	6.	6.	6.	6.	6.
1913	UI	5.	5.	5.	5.	5.	5.	5.	5.	4.	4.
1914	UI	4.	4.	4.	4.	4.	4.	3.	3.	3.	3.
1915	UI	3.	3.	3.	3.	3.	3.	3.	3.	2.	2.
1916	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1917	UI	2.	2.	2.	2.	2.	1.	1.	1.	1.	1.
1918	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1919	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1920	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1921	UI	1.	1.	1.	1.	1.	1.	1.	1.	0.	0.
1922	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1923	KO					22					
1924	KK	HC352									
1925	HC	2									
1926	KO					22					
1927	KK	CV751East Jordan Canal									
1928	RD	2312.55	0.00050	0.030		TRAP	20.00	1.50			
1929	KO					22					
1930	KK	EJC083									
1931	PB	1.06									
1932	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
1933	PI	.015	.015	.136	.176	.094	.050	.050	.050	.035	.035
1934	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
1935	PI	.010	.010	.010	.010	.010	.010				
1936	BA	.065									
1937	LU	.50	.71	39.65							
1938	IN	5									
1939	UI	0.	0.	0.	1.	1.	1.	1.	1.	1.	2.
1940	UI	2.	4.	4.	9.	9.	16.	16.	21.	21.	21.
1941	UI	27.	27.	33.	33.	39.	39.	45.	45.	52.	52.
1942	UI	58.	58.	64.	64.	70.	70.	66.	66.	66.	61.
1943	UI	61.	56.	56.	52.	52.	47.	47.	42.	42.	37.
1944	UI	37.	32.	32.	27.	27.	24.	24.	24.	21.	21.
1945	UI	19.	19.	17.	17.	15.	15.	14.	14.	12.	12.
1946	UI	11.	11.	11.	11.	10.	10.	10.	9.	9.	8.
1947	UI	8.	8.	8.	7.	7.	7.	7.	6.	6.	6.
1948	UI	6.	6.	6.	5.	5.	5.	5.	5.	5.	5.
1949	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	4.
1950	UI	4.	4.	4.	4.	3.	3.	3.	3.	3.	3.
1951	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
1952	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1953	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1954	UI	2.	2.	2.	2.	2.	1.	1.	1.	1.	1.
1955	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1956	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1957	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1958	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1959	UI	1.	1.	1.	1.	1.	1.	1.	0.	0.	0.
1960	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1961	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1962	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1963	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1964	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1965	KK	RJC084									
1966	KM	TO EJC 086									
1967	RD	700.	.0007	.0450		TRAP	6.00	1.00			
1968	KK	EJC086									

1969	BA	.12									
1970	LU	.38	.72	52.49							
1971	IN	5									
1972	UI	1.	1.	1.	1.	1.	1.	2.	2.	4.	
1973	UI	4.	7.	7.	7.	17.	17.	29.	29.	39.	39.
1974	UI	49.	49.	59.	59.	59.	70.	70.	82.	82.	93.
1975	UI	93.	104.	104.	116.	116.	127.	127.	127.	119.	119.
1976	UI	111.	111.	102.	102.	93.	93.	84.	84.	84.	75.
1977	UI	75.	67.	67.	58.	58.	49.	49.	43.	43.	38.
1978	UI	38.	38.	34.	34.	30.	30.	27.	27.	25.	25.
1979	UI	22.	22.	20.	20.	20.	19.	19.	17.	17.	16.
1980	UI	16.	15.	15.	14.	14.	14.	13.	13.	12.	12.
1981	UI	12.	12.	11.	11.	10.	10.	10.	10.	10.	9.
1982	UI	9.	9.	9.	8.	8.	8.	8.	8.	8.	8.
1983	UI	7.	7.	7.	7.	7.	6.	6.	6.	6.	6.
1984	UI	6.	6.	6.	6.	6.	5.	5.	5.	5.	5.
1985	UI	5.	5.	5.	4.	4.	4.	4.	4.	4.	4.
1986	UI	4.	4.	4.	4.	4.	4.	4.	3.	3.	3.
1987	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
1988	UI	3.	3.	3.	3.	2.	2.	2.	2.	2.	2.
1989	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
1990	UI	2.	2.	2.	2.	2.	2.	2.	2.	1.	1.
1991	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1992	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1993	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1994	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1995	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1996	UI	1.	1.	1.	1.	1.	1.	1.	0.	0.	0.
1997	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1998	UI	0.	0.								
1999	KK	CJC087									
2000	KM	(083-086)									
2001	HC	2									
2002	KO					22					
2003	KK	HC353									
2004	HC	2									
2005	KO					22					
2006	KK	CV752East Jordan Canal									
2007	RD	829.323 0.00050 0.030				TRAP	20.00	1.50			
2008	KO					22					
2009	KK	EJC090									
2010	PB	1.06									
2011	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
2012	PI	.015	.015	.136	.176	.094	.050	.050	.050	.035	.035
2013	PI	.035	.035	.035	.035	.015	.015	.015	.015	.015	.015
2014	PI	.010	.010	.010	.010	.010	.010				
2015	BA	.043									
2016	LU	.51	.70	47.90							
2017	IN	5									
2018	UI	0.	0.	0.	0.	0.	1.	1.	1.	1.	1.
2019	UI	1.	3.	3.	6.	6.	11.	11.	15.	15.	19.
2020	UI	19.	23.	23.	27.	27.	31.	31.	35.	35.	40.
2021	UI	40.	44.	44.	48.	48.	45.	45.	42.	42.	39.
2022	UI	39.	39.	35.	35.	32.	32.	29.	29.	26.	26.
2023	UI	22.	22.	19.	19.	16.	16.	14.	14.	13.	13.
2024	UI	11.	11.	10.	10.	9.	9.	8.	8.	8.	8.
2025	UI	7.	7.	7.	7.	6.	6.	6.	6.	5.	5.
2026	UI	5.	5.	5.	5.	5.	4.	4.	4.	4.	4.
2027	UI	4.	4.	4.	3.	3.	3.	3.	3.	3.	3.
2028	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
2029	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2030	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2031	UI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
2032	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2033	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2034	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2035	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2036	UI	1.	1.	1.	0.	0.	0.	0.	0.	0.	0.
2037	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

2038	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2039	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2040	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2041	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2042	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2043	KO								22			
2044	KK	HC354										
2045	HC	2										
2046	KO								22			
2047	KK	CV753	East Jordan Canal									
2048	RD	692.566	0.00050	0.030				TRAP	20.00	1.50		
2049	KO							22				
2050	KK	HC355										
2051	HC	2										
2052	KO								22			
2053	KK	SBC43										
2054	BA	0.2473										
2055	LS	0.1	98.000					57.458	0.1			
2056	UK	25	.02	.1	27.48							
2057	UK	70	.02	.4	72.52							
2058	RD	700	.02	.02	.03			TRAP	2	25		
2059	RD	5000	.01	.015				CIRC	2			
2060	KO							22				
2061	KK	CV824	East Jordan Canal									
2062	RD	2200.62	0.00050	0.030				TRAP	20.00	1.50		
2063	KO							22				
2064	KK	SBC44										
2065	BA	0.072										
2066	LS	0.1	98.000					68.503	0.1			
2067	UK	25	.02	.1	34.53							
2068	UK	70	.02	.4	65.47							
2069	RD	2400	.02	.02				TRAP	2	25		
2070	KO							22				
2071	KK	HC360										
2072	HC	2										
2073	KO								22			
2074	KK	CV864	East Jordan Canal									
2075	RD	3011.47	0.00050	0.030				TRAP	20.00	1.50		
2076	KO							22				
2077	KK	SB88										
2078	BA	0.1008										
2079	LS	0.1	98.000					68.990	0.1			
2080	UK	25	.02	.1	27.15							
2081	UK	70	.02	.4	72.85							
2082	RD	2150	.02	.02				TRAP	2	25		
2083	KO							22				
2084	KK	HC36										
2085	HC	2										
2086	KO								22			
2087	KK	CV808	East Jordan Canal									
2088	RD	251.887	0.00050	0.030				TRAP	20.00	1.50		
2089	KO							22				
2090	KK	SB880										
2091	BA	0.1068										
2092	LS	0.1	98.000					72.619	0.1			
2093	UK	25	.02	.1	38.56							
2094	UK	70	.02	.4	61.44							
2095	RD	950	.02	.02	.03			TRAP	2	25		
2096	RD	2450	.015	.02				CIRC	1.5			
2097	KO							22				

2098	KK	HC404						
2099	HC	2						
2100	KO					22		
2101	KK	CV872						
2102	RD	2200.07	0.00050	0.030		TRAP	20.00	1.50
2103	KO					22		
2104	KK	SB894						
2105	BA	0.0779						
2106	LS	0.1	98.000			78.559	0.1	
2107	UK	25	.02	.1	14.12			
2108	UK	60	.02	.4	85.88			
2109	RD	400	.01	.02	.008	TRAP	2	25
2110	RD	1500	.01	.02		TRAP	2	25
2111	KO					22		
2112	KK	CV860East Jordan Canal						
2113	RD	1048.21	0.00050	0.030		TRAP	20.00	1.50
2114	KO					22		
2115	KK	SB895						
2116	BA	0.0375						
2117	LS	0.1	98.000			63.619	0.1	
2118	UK	25	0.02	.1	14.87			
2119	UK	60	.02	.4	85.13			
2120	RD	800	.01	.02		TRAP	2	25
2121	KO					22		
2122	KK	HC408						
2123	HC	2						
2124	KO					22		
2125	KK	CV813East Jordan Canal						
2126	RD	3447.81	0.00050	0.030		TRAP	20.00	1.50
2127	KO					22		
2128	KK	SBC3						
2129	BA	0.1480						
2130	LS	0.1	98.000			63.897	0.1	
2131	UK	25	.02	.1	10.02			
2132	UK	70	.02	.4	89.98			
2133	RD	1200	.02	.02	.019	TRAP	2	25
2134	RD	2500	.02	.02		TRAP	2	25
2135	KO					22		
2136	KK	SBC23						
2137	BA	0.1314						
2138	LS	0.1	98.000			74.047	0.1	
2139	UK	25	.02	.1	35.66			
2140	UK	70	.02	.4	64.34			
2141	RD	1300	.02	.02	.014	TRAP	2	25
2142	RD	1950	.029	.02		TRAP	2	25
2143	KO					22		
2144	KK	HC362						
2145	HC	3						
2146	KO					22		
2147	KK	CV761East Jordan Canal						
2148	RD	1292.96	0.00050	0.030		TRAP	20.00	1.50
2149	KO					22		
2150	KK	SBC20						
2151	BA	0.0996						
2152	LS	0.1	98.000			65.287	0.1	
2153	UK	25	.02	.1	13.13			
2154	UK	70	.02	.1	86.87			
2155	RD	2300	.02	.02		TRAP	2	25
2156	KO					22		
2157	KK	HC363						
2158	HC	2						

2159	KO					22		
2160	KK	CV762East Jordan Canal						
2161	RD	1436.25 0.00050 0.030				TRAP	20.00	1.50
2162	KO					22		
2163	KK	SBC2						
2164	BA	0.1769						
2165	LS	0.1 98.000				63.877	0.1	
2166	UK	25 .02 .1		9.16				
2167	UK	70 .02 .4		90.84				
2168	RD	1300 .02 .02		.065		TRAP	2	25
2169	RD	2200 .015 .02				TRAP	2	25
2170	KO					22		
2171	KK	HC364						
2172	HC	2						
2173	KO					22		
2174	KK	CV763East Jordan Canal						
2175	RD	1161.54 0.00050 0.030				TRAP	20.00	1.50
2176	KO					22		
2177	KK	SBC30						
2178	BA	0.162						
2179	LS	0.1 98.000				63.928	0.1	
2180	UK	25 .02 .1		7.75				
2181	UK	70 .02 .4		92.25				
2182	RD	1800 .01 .03				TRAP	2	25
2183	KO					22		
2184	KK	HC365						
2185	HC	2						
2186	KO					22		
2187	KK	CV832East Jordan Canal						
2188	RD	3752.84 0.00050 0.030				TRAP	20.00	1.50
2189	KO					22		
2190	KK	SB573						
2191	BA	0.2316						
2192	LS	0.1 98.000				86.000	0.1	
2193	UK	30 .02 .1		60.00				
2194	UK	400 .01 .4		40.00				
2195	RD	3000 .01 .04				TRAP	2	2
2196	KO					22		
2197	KK	CV535Jordan Salt Lake Canal						
2198	RD	2082.32 0.00050 0.030				TRAP	12.00	1.00
2199	KO					22		
2200	KK	SB572						
2201	BA	0.1474						
2202	LS	0.1 98.000				60.000	0.1	
2203	UK	40 .02 .1		36.57				
2204	UK	350 .02 .4		63.43				
2205	RD	2000 .02 .02				TRAP	2	25
2206	KO					22		
2207	KK	SB57						
2208	BA	0.144						
2209	LS	0.1 98.000				86.000	0.1	
2210	UK	30 .02 .1		60.00				
2211	UK	400 .005 .4		40.00				
2212	RD	2000 .005 .04				TRAP	2	2
2213	KO					22		
2214	KK	HC25						
2215	HC	3						
2216	KO					22		
2217	KK	CV534Jordan Salt Lake Canal						
2218	RD	1644.53 0.00050 0.030				TRAP	12.00	1.00
2219	KO					22		

2220	KK	SB376							
2221	BA	0.0626							
2222	LS	0.1	98.000			86.000	0.1		
2223	UK	30	.02	.1	60.00				
2224	UK	400	.04	.4	40.00				
2225	RD	1000	.005	.04		TRAP	0	2	
2226	KO					22			
2227	KK	SB374							
2228	BA	0.1247							
2229	LS	0.1	98.000			63.036	0.1		
2230	UK	20	.02	.1	14.46				
2231	UK	200	.005	.4	85.54				
2232	RD	2500	.005	.02		TRAP	2	25	
2233	KO					22			
2234	KK	HC326							
2235	HC	2							
2236	KO					22			
2237	KK	CV474to Jordan Salt Lake Canal							
2238	RD	559.369	0.00300	0.015		CIRC	1.50	0.00	
2239	KO					22			
2240	KK	SB38							
2241	BA	0.0170							
2242	LS	0.1	98.000			86.000	0.1		
2243	UK	20	.02	.1	60.00				
2244	UK	200	.005	.4	40.00				
2245	RD	800	.005	.04		TRAP	0	2	
2246	KO					22			
2247	KK	SB382							
2248	BA	0.0503							
2249	LS	0.1	98.000			86.000	0.1		
2250	UK	30	.02	.1	60.00				
2251	UK	400	.0005	.4	40.00				
2252	RD	900	.005	.04		TRAP	0	2	
2253	KO					22			
2254	KK	HC252							
2255	HC	4							
2256	KO					22			
2257	KK	CV475Jordan Salt Lake Canal							
2258	RD	3104.84	0.00050	0.030		TRAP	12.00	1.00	
2259	KO					22			
2260	KK	SB574							
2261	BA	0.0292							
2262	LS	0.1	98.000			76.500	0.1		
2263	UK	30	.02	.1	11.00				
2264	UK	400	.005	.4	89.00				
2265	RD	520	.005	.04		TRAP	10	2	
2266	KO					22			
2267	KK	HC253							
2268	HC	2							
2269	KO					22			
2270	KK	CV471Jordan Salt Lake Canal							
2271	RD	751.670	0.00050	0.030		TRAP	12.00	1.00	
2272	KO					22			
2273	KK	SB575							
2274	BA	0.0334							
2275	LS	0.1	98.000			78.741	0.1		
2276	UK	30	.02	.1	11.00				
2277	UK	200	.007	.4	89.00				
2278	RD	500	.007	.04		TRAP	1	10	
2279	KO					22			
2280	KK	HC254							

2281	HC	2						
2282	KO					22		
2283	KK	CV470	Jordan Salt Lake Canal					
2284	RD	722.289	0.00050	0.030		TRAP	12.00	1.00
2285	KO					22		
2286	KK	SB576						
2287	BA	0.0316						
2288	LS	0.1	98.000			78.643	0.1	
2289	UK	50	.005	.1	29.99			
2290	UK	100	.005	.4	70.0			
2291	RD	1000	.007	.04		TRAP	2	5
2292	KO					22		
2293	KK	HC255						
2294	HC	2						
2295	KO					22		
2296	KK	CV472	Jordan Salt Lake Canal					
2297	RD	968.687	0.00050	0.030		TRAP	12.00	1.00
2298	KO					22		
2299	KK	SB577						
2300	BA	0.0448						
2301	LS	0.1	98.000			75.000	0.1	
2302	UK	20	.02	.1	11.00			
2303	UK	75	.01	.4	89.00			
2304	RD	500	.001	.02		TRAP	2	25
2305	KO					22		
2306	KK	HC256						
2307	HC	2						
2308	KO					22		
2309	KK	CV478	Jordan Salt Lake Canal					
2310	RD	460.627	0.00050	0.030		TRAP	12.00	1.00
2311	KO					22		
2312	KK	SB579						
2313	BA	0.0184						
2314	LS	0.1	98.000			86.000	0.1	
2315	UK	10	.02	.1	60.00			
2316	UK	300	.005	.4	40.00			
2317	RD	500	.005	.04		TRAP	10	2
2318	KO					22		
2319	KK	HC257						
2320	HC	2						
2321	KO					22		
2322	KK	CV479	Jordan Salt Lake Canal					
2323	RD	705.693	0.00050	0.030		TRAP	12.00	1.00
2324	KO					22		
2325	KK	SB578						
2326	BA	0.0333						
2327	LS	0.1	98.000			86.000	0.1	
2328	UK	40	.01	.1	60.00			
2329	UK	200	.01	.4	40.00			
2330	RD	1100	.005	.04		TRAP	10	2
2331	KO					22		
2332	KK	HC258						
2333	HC	2						
2334	KO					22		
2335	KK	CV480	Jordan Salt Lake Canal					
2336	RD	970.488	0.00050	0.030		TRAP	12.00	1.00
2337	KO					22		
2338	KK	SB609						
2339	BA	0.0287						
2340	LS	0.1	98.000			86.000	0.1	

2341	UK	20	.02	.1	60.00			
2342	UK	300	.005	.4	40.00			
2343	RD	1300	.005	.04		TRAP	10	2
2344	KO						22	
2345	KK	HC259						
2346	HC	2						
2347	KO						22	
2348	KK	CV865	Jordan Salt Lake Canal					
2349	RD	932.093	0.00050	0.030		TRAP	12.00	1.00
2350	KO						22	
2351	KK	DH4						
2352	DT	DR4						
2353	DI	0	25	50	100	200	400	800
2354	DQ	0	25	50	100	200	400	800
2355	KO						22	
2356	KK	CV866	Jordan Salt Lake Canal					
2357	RD	885.275	0.00050	0.030		TRAP	12.00	1.00
2358	KO						22	
2359	KK	SB358						
2360	BA	0.0185						
2361	LS	0.1	98.000			71.161	0.1	
2362	UK	30	.01	.1	30.33			
2363	UK	30	.005	.4	69.67			
2364	RD	1000	.005	.04		TRAP	10	2
2365	KO						22	
2366	KK	HC260						
2367	HC	2						
2368	KO						22	
2369	KK	CV482	Jordan Salt Lake Canal					
2370	RD	1885.79	0.00050	0.030		TRAP	12.00	1.00
2371	KO						22	
2372	KK	SB595						
2373	BA	0.0148						
2374	LS	0.1	98.000			74.000	0.1	
2375	UK	30	.02	.1	66.43			
2376	UK	20	.02	.4	33.57			
2377	RD	400	.005	.02		TRAP	2	25
2378	KO						22	
2379	KK	SB262						
2380	BA	0.0196						
2381	LS	0.1	98.000			74.000	0.1	
2382	UK	30	.01	.1	60.60			
2383	UK	25	.02	.4	39.40			
2384	RD	500	.005	.02		TRAP	2	25
2385	KO						22	
2386	KK	HC274						
2387	HC	2						
2388	KO						22	
2389	KK	CV486to	Jordan Salt Lake Canal					
2390	RD	613.371	0.01200	0.015		CIRC	1.25	0.00
2391	KO						22	
2392	KK	CV488to	Jordan Salt Lake Canal					
2393	RD	372.438	0.01000	0.015		CIRC	1.25	0.00
2394	KO						22	
2395	KK	CV487to	Jordan Salt Lake Canal					
2396	RD	597.840	0.00500	0.015		CIRC	1.25	0.00
2397	KO						22	
2398	KK	SB602						
2399	BA	0.047						

2400	LS	0.1	98.000			75.000	0.1			
2401	UK	40	.02	.1	59.10					
2402	UK	80	.02	.4	40.90					
2403	RD	500	.01	.02	.0025	TRAP	2	25		
2404	RD	1000	.01	.015		CIRC	1.25			
2405	KO					22				
2406	KK	HC262								
2407	HC	3								
2408	KO					22				
2409	KK	CV485	Jordan Salt Lake Canal							
2410	RD	1630.82	0.00050	0.030		TRAP	12.00	1.00		
2411	KO					22				
2412	KK	SB877								
2413	BA	0.0260								
2414	LS	0.1	98.000			86.000	0.1			
2415	UK	30	.02	.1	60.00					
2416	UK	250	.02	.4	40.00					
2417	RD	1300	.005	.04		TRAP	10	2		
2418	KO					22				
2419	KK	HC263								
2420	HC	2								
2421	KO					22				
2422	KK	CV489	Jordan Salt Lake Canal							
2423	RD	1288.49	0.00050	0.030		TRAP	12.00	1.00		
2424	KO					22				
2425	KK	SB206								
2426	BA	0.0387								
2427	LS	0.1	98.000			81.000	0.1			
2428	UK	25	.02	.1	35.00					
2429	UK	200	.01	.4	65.00					
2430	RD	400	.01	.04	.014	TRAP	2	2		
2431	RD	2000	.02	.04		TRAP	2	2		
2432	KO					22				
2433	KK	HC264								
2434	HC	2								
2435	KO					22				
2436	KK	CV490	Jordan Salt Lake Canal							
2437	RD	869.690	0.00050	0.030		TRAP	12.00	1.00		
2438	KO					22				
2439	KK	SB593								
2440	BA	0.0533								
2441	LS	0.1	98.000			65.000	0.1			
2442	UK	20	.02	.1	36.54					
2443	UK	60	.02	.4	63.46					
2444	RD	1700	.02	.015		CIRC	1.5			
2445	KO					22				
2446	KK	CV215	to Jordan Salt Lake Canal							
2447	RD	218.710	0.01000	0.015		CIRC	2.00	0.00		
2448	KO					22				
2449	KK	DB38								
2450	RS		FLOW							
2451	SV	0	0.066	0.138	0.215	0.299	0.389	0.484	0.759	0.83
2452	SE	0	0.5	1	1.5	2	2.5	3	4	4.5
2453	SQ	0	2.784	3.9375	4.8225	5.569	6.226	6.82	6.9905	17.05
2454	KO					22				
2455	KK	HC265								
2456	HC	2								
2457	KO					22				
2458	KK	CV491	Jordan Salt Lake Canal							
2459	RD	3566.19	0.00050	0.030		TRAP	12.00	1.00		
2460	KO					22				

2461	KK	HC266									
2462	HC	3									
2463	KO					22					
2464	KK	JSL010									
2465	PB	1.04									
2466	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
2467	PI	.015	.015	.131	.157	.093	.050	.050	.050	.034	.034
2468	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2469	PI	.010	.010	.010	.010	.010	.010				
2470	BA	.077									
2471	LU	.77	.49	4.93							
2472	IN	5									
2473	UI	1.	1.	2.	3.	5.	9.	22.	22.	37.	50.
2474	UI	64.	77.	91.	106.	121.	135.	150.	165.	154.	143.
2475	UI	132.	121.	121.	109.	98.	87.	75.	64.	55.	49.
2476	UI	44.	39.	35.	32.	29.	26.	25.	23.	23.	21.
2477	UI	19.	18.	17.	16.	15.	14.	13.	13.	12.	11.
2478	UI	11.	10.	10.	10.	9.	9.	9.	8.	8.	8.
2479	UI	7.	7.	7.	6.	6.	6.	6.	5.	5.	5.
2480	UI	5.	5.	4.	4.	4.	4.	4.	4.	3.	3.
2481	UI	3.	3.	3.	3.	3.	3.	2.	2.	2.	2.
2482	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2483	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2484	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2485	UI	1.	1.	1.	1.	1.	1.	1.	0.		
2486	KK	JSL020									
2487	BA	.107									
2488	LU	.68	.48	25.49							
2489	IN	5									
2490	UI	1.	1.	1.	1.	2.	2.	2.	2.	4.	8.
2491	UI	8.	18.	18.	31.	31.	41.	41.	52.	63.	63.
2492	UI	75.	75.	87.	87.	99.	99.	112.	124.	124.	136.
2493	UI	136.	127.	127.	118.	118.	109.	99.	99.	90.	90.
2494	UI	80.	80.	72.	72.	62.	53.	53.	46.	46.	40.
2495	UI	40.	36.	36.	32.	32.	29.	26.	26.	24.	24.
2496	UI	22.	22.	20.	20.	19.	17.	17.	16.	16.	15.
2497	UI	15.	14.	14.	13.	12.	12.	12.	12.	11.	11.
2498	UI	10.	10.	10.	9.	9.	9.	9.	8.	8.	8.
2499	UI	8.	8.	7.	7.	7.	7.	7.	7.	7.	7.
2500	UI	6.	6.	6.	6.	6.	5.	5.	5.	5.	5.
2501	UI	5.	5.	5.	5.	4.	4.	4.	4.	4.	4.
2502	UI	4.	4.	4.	4.	4.	3.	3.	3.	3.	3.
2503	UI	3.	3.	3.	3.	3.	3.	3.	2.	2.	2.
2504	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2505	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	1.
2506	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2507	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2508	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2509	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2510	UI	1.	1.	1.	1.	1.	1.	1.	1.	0.	0.
2511	UI	0.	0.	0.	0.	0.	0.	0.			
2512	KK	RJS02									
	*										
	*	NOTE: FULL PIPE CAPACITY IS 23.9-CFS.									
	*	HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING									
	*	OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.									
	*										
2513	RD	270.	.0148	.0150		CIRC	2.00	.00			
2514	KK	CCP1032									
2515	HC	2									
2516	KO					22					
2517	KK	CV727Jordan Salt Lake Canal									
2518	RD	1679.20	0.00050	0.030		TRAP	12.00	1.00			
2519	KO					22					
2520	KK	JSL030									
2521	PB	1.04									

2522	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
2523	PI	.015	.015	.131	.157	.093	.050	.050	.050	.034	.034
2524	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2525	PI	.010	.010	.010	.010	.010	.010				
2526	BA	.08									
2527	LU	.67	.45	17.70							
2528	IN	5									
2529	UI	1.	1.	1.	2.	2.	2.	4.	7.	7.	17.
2530	UI	29.	29.	39.	50.	61.	61.	72.	84.	84.	95.
2531	UI	107.	107.	118.	130.	122.	122.	113.	104.	104.	95.
2532	UI	86.	86.	77.	69.	59.	59.	51.	44.	44.	39.
2533	UI	34.	34.	31.	27.	25.	25.	23.	21.	21.	19.
2534	UI	18.	18.	16.	15.	14.	14.	13.	13.	13.	12.
2535	UI	11.	11.	11.	10.	9.	9.	9.	8.	8.	8.
2536	UI	8.	8.	7.	7.	7.	7.	7.	6.	6.	6.
2537	UI	6.	6.	5.	5.	5.	5.	5.	5.	5.	4.
2538	UI	4.	4.	4.	4.	4.	4.	4.	3.	3.	3.
2539	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
2540	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2541	UI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
2542	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2543	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2544	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2545	UI	1.	1.	1.	1.	0.	0.	0.	0.	0.	0.
2546	UI	0.									
2547	KO					22					
2548	KK	HC328									
2549	HC	2									
2550	KO					22					
2551	KK	CV728Jordan Salt Lake Canal									
2552	RD	615.597 0.00050 0.030				TRAP	12.00	1.00			
2553	KO					22					
2554	KK	JSL040									
2555	PB	1.04									
2556	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
2557	PI	.015	.015	.131	.157	.093	.050	.050	.050	.034	.034
2558	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2559	PI	.010	.010	.010	.010	.010	.010				
2560	BA	.070									
2561	LU	.65	.45	14.85							
2562	IN	5									
2563	UI	1.	1.	1.	1.	1.	2.	3.	3.	6.	6.
2564	UI	14.	23.	23.	31.	40.	40.	48.	57.	57.	66.
2565	UI	76.	76.	85.	85.	94.	103.	103.	97.	90.	90.
2566	UI	83.	76.	76.	69.	61.	61.	55.	55.	47.	40.
2567	UI	40.	35.	31.	31.	27.	24.	24.	22.	20.	20.
2568	UI	18.	17.	17.	15.	15.	14.	13.	13.	12.	11.
2569	UI	11.	11.	10.	10.	9.	9.	9.	8.	8.	8.
2570	UI	7.	7.	7.	7.	7.	6.	6.	6.	6.	6.
2571	UI	6.	5.	5.	5.	5.	5.	5.	5.	5.	4.
2572	UI	4.	4.	4.	4.	4.	4.	4.	3.	3.	3.
2573	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	2.
2574	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2575	UI	2.	2.	2.	2.	2.	2.	2.	2.	1.	1.
2576	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2577	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2578	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2579	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	0.
2580	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2581	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2582	KO					22					
2583	KK	HC329									
2584	HC	2									
2585	KO					22					
2586	KK	CV729Jordan Salt Lake Canal									
2587	RD	954.789 0.00050 0.030				TRAP	12.00	1.00			
2588	KO					22					

2589	KK	HC330									
2590	HC	2									
2591	KO					22					
2592	KK	JSL050									
2593	PB	1.04									
2594	PI	.010	.010	.010	.010	.010	.015	.015	.015	.015	.015
2595	PI	.015	.015	.131	.157	.093	.050	.050	.050	.034	.034
2596	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2597	PI	.010	.010	.010	.010	.010	.010				
2598	BA	.05									
2599	LU	.49	.40	22.0							
2600	IN	5									
2601	UI	0.	0.	1.	1.	2.	2.	3.	5.	11.	11.
2602	UI	19.	26.	26.	33.	40.	47.	47.	55.	62.	70.
2603	UI	70.	77.	85.	85.	80.	74.	68.	68.	62.	56.
2604	UI	50.	50.	45.	39.	39.	33.	29.	25.	25.	22.
2605	UI	20.	18.	18.	16.	15.	14.	14.	13.	12.	12.
2606	UI	11.	10.	9.	9.	9.	8.	8.	8.	7.	7.
2607	UI	7.	7.	6.	6.	6.	6.	5.	5.	5.	5.
2608	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	4.
2609	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2610	UI	3.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2611	UI	2.	2.	2.	2.	2.	2.	1.	1.	1.	1.
2612	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2613	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2614	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2615	UI	1.	1.	0.	0.	0.	0.	0.	0.	0.	0.
2616	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2617	UI	0.	0.	0.	0.	0.					
2618	KO					22					
2619	KK	CV730Jordan Salt Lake Canal									
2620	RD	980.614 0.00050 0.030				TRAP	12.00	1.00			
2621	KO					22					
2622	IN	5									
2623	KK	JSL060									
2624	PB	1.04									
2625	PI	.010	.010	.010	.010	.010	.010	.015	.015	.015	.015
2626	PI	.015	.015	.131	.157	.093	.050	.050	.050	.034	.034
2627	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2628	PI	.010	.010	.010	.010	.010	.010				
2629	BA	.054									
2630	LU	.50	.55	20.00							
2631	IN	5									
2632	KM										
2633	KM										
2634	KM										
2635	KM										
2636	KM										
2637	UI	1.	1.	1.	2.	2.	3.	6.	14.	24.	33.
2638	UI	42.	51.	51.	60.	70.	80.	89.	99.	109.	102.
2639	UI	95.	95.	87.	80.	72.	65.	58.	49.	42.	42.
2640	UI	37.	32.	29.	26.	23.	21.	19.	19.	17.	16.
2641	UI	15.	14.	13.	12.	11.	11.	11.	10.	9.	9.
2642	UI	8.	8.	7.	7.	7.	7.	6.	6.	6.	6.
2643	UI	5.	5.	5.	5.	5.	5.	4.	4.	4.	4.
2644	UI	4.	4.	4.	3.	3.	3.	3.	3.	3.	3.
2645	UI	3.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2646	UI	2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
2647	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2648	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2649	UI	1.	1.	1.	1.	0.	0.	0.	0.	0.	0.
2650	UI	0.	0.	0.	0.	0.	0.				
2651	KO					22					
2652	KK	HC33									
2653	HC	2									
2654	KO					22					

COMBINE SH/JSL MODEL WITH DRY CREEK

2655	KK	CV867	Jordan Salt Lake Canal								
2656	RD	1399.58	0.00050	0.030	TRAP	12.00	1.00				
2657	KO				22						
2658	KK	DH3									
2659	DT	DR3									
2660	DI	0	25	50	100	200	400	800			
2661	DQ	0	25	50	100	200	400	800			
2662	KO					22					
2663	KK	CV819	Jordan Salt Lake Canal								
2664	RD	2341.43	0.00050	0.030	TRAP	12.00	1.00				
2665	KO				22						
2666	KK	JSL200									
2667	PB	1.09									
2668	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2669	PI	.015	.015	.141	.211	.094	.050	.050	.050	.034	.034
2670	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2671	PI	.009	.009	.009	.009	.009	.009				
2672	BA	.073									
2673	LU	.80	.60	1.00							
2674	IN	5									
2675	UI	0.	0.	0.	0.	0.	1.	1.	1.	1.	1.
2676	UI	1.	2.	2.	4.	4.	4.	9.	9.	15.	15.
2677	UI	15.	20.	20.	26.	26.	26.	31.	31.	37.	37.
2678	UI	37.	43.	43.	48.	48.	48.	54.	54.	54.	60.
2679	UI	60.	66.	66.	66.	62.	62.	58.	58.	58.	53.
2680	UI	53.	48.	48.	48.	44.	44.	39.	39.	39.	35.
2681	UI	35.	30.	30.	30.	26.	26.	22.	22.	22.	20.
2682	UI	20.	17.	17.	17.	16.	16.	14.	14.	14.	13.
2683	UI	13.	12.	12.	12.	11.	11.	10.	10.	10.	9.
2684	UI	9.	8.	8.	8.	8.	8.	7.	7.	7.	7.
2685	UI	7.	6.	6.	6.	6.	6.	6.	6.	6.	5.
2686	UI	5.	5.	5.	5.	5.	5.	5.	5.	5.	4.
2687	UI	4.	4.	4.	4.	4.	4.	4.	4.	4.	4.
2688	UI	4.	4.	3.	3.	3.	3.	3.	3.	3.	3.
2689	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2690	UI	3.	3.	2.	2.	2.	2.	2.	2.	2.	2.
2691	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2692	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2693	UI	2.	2.	1.	1.	1.	1.	1.	1.	1.	1.
2694	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2695	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2696	KK	RJS20									
2697	RD	1600.	.0025	.0450	TRAP	6.00	1.00				
2698	KO				22						
2699	KK	HC332									
2700	HC	2									
2701	KO				22						
2702	KK	CV757	Jordan Salt Lake Canal								
2703	RD	689.358	0.00050	0.030	TRAP	12.00	1.00				
2704	KO				22						
2705	KK	JSL500									
2706	PB	1.09									
2707	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2708	PI	.015	.015	.141	.211	.094	.050	.050	.050	.034	.034
2709	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2710	PI	.009	.009	.009	.009	.009	.009				
2711	BA	.06									
2712	LU	.80	.60	1.00							
2713	IN	5									
2714	UI	0.	0.	0.	0.	0.	0.	1.	1.	1.	1.
2715	UI	1.	1.	1.	1.	1.	1.	3.	3.	3.	6.
2716	UI	6.	6.	11.	11.	11.	14.	14.	14.	18.	18.
2717	UI	22.	22.	22.	26.	26.	26.	30.	30.	30.	35.
2718	UI	35.	35.	39.	39.	39.	43.	43.	43.	47.	47.
2719	UI	47.	44.	44.	44.	41.	41.	41.	38.	38.	38.
2720	UI	35.	35.	35.	31.	31.	31.	28.	28.	28.	25.

2721	UI	25.	25.	21.	21.	21.	18.	18.	18.	16.	16.
2722	UI	16.	14.	14.	14.	12.	12.	12.	11.	11.	10.
2723	UI	10.	10.	9.	9.	9.	8.	8.	8.	8.	8.
2724	UI	8.	7.	7.	7.	6.	6.	6.	6.	6.	6.
2725	UI	6.	6.	6.	5.	5.	5.	5.	5.	5.	5.
2726	UI	5.	5.	4.	4.	4.	4.	4.	4.	4.	4.
2727	UI	4.	4.	4.	4.	3.	3.	3.	3.	3.	3.
2728	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2729	UI	3.	3.	3.	3.	2.	2.	2.	2.	2.	2.
2730	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2731	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2732	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2733	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2734	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2735	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2736	KK	RJS50									

*
* NOTE: FULL PIPE CAPACITY IS 116.1-CFS.
* HYDROGRAPHS SUBSTANTIALLY EXCEEDING THIS CAPACITY SHOULD BE ROUTED USING
* OTHER METHODS SUCH AS MODIFIED PULS FOR PIPE AND OVERFLOW (STREET) SYSTEMS.
*

2737	RD	900.	.0400	.0150		CIRC	3.00	.00			
2738	KK	JSL510									
2739	PB	1.09									
2740	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2741	PI	.015	.015	.140	.211	.094	.050	.050	.050	.034	.034
2742	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2743	PI	.009	.009	.009	.009	.009	.009				
2744	BA	.068									
2745	LU	.30	.64	47.50							
2746	IN	5									
2747	UI	0.	0.	0.	0.	0.	0.	0.	1.	1.	1.
2748	UI	1.	1.	2.	2.	2.	3.	3.	3.	7.	7.
2749	UI	7.	12.	12.	17.	17.	17.	21.	21.	21.	26.
2750	UI	26.	26.	31.	31.	31.	36.	36.	41.	41.	41.
2751	UI	46.	46.	46.	51.	51.	51.	55.	55.	55.	52.
2752	UI	52.	48.	48.	48.	44.	44.	44.	41.	41.	41.
2753	UI	37.	37.	37.	33.	33.	29.	29.	29.	25.	25.
2754	UI	25.	22.	22.	22.	19.	19.	19.	16.	16.	15.
2755	UI	15.	15.	13.	13.	13.	12.	12.	12.	11.	11.
2756	UI	11.	10.	10.	9.	9.	9.	8.	8.	8.	8.
2757	UI	8.	8.	7.	7.	7.	7.	7.	6.	6.	6.
2758	UI	6.	6.	6.	5.	5.	5.	5.	5.	5.	5.
2759	UI	5.	5.	5.	5.	4.	4.	4.	4.	4.	4.
2760	UI	4.	4.	4.	4.	4.	3.	3.	3.	3.	3.
2761	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2762	UI	3.	3.	3.	3.	3.	3.	3.	3.	2.	2.
2763	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2764	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2765	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2766	UI	2.	2.	1.	1.	1.	1.	1.	1.	1.	1.
2767	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2768	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.

2769	KK	COMMERCIAL DETENTION, DISCHARGE LIMITED TO 0.3 CFS/ACRE									
2770	RS	1	STOR	.00							
2771	SV	.0	.5	1.0	1.5						
2772	SQ	.0	7.0	7.0	7.0						
2773	SE	.0	1.0	2.0	3.0						

2774	KK	CCP1033									
2775	HC	2									
2776	KO					22					

2777	KK	HC333									
2778	HC	2									
2779	KO					22					

2780	KK	CV732Jordan Salt Lake Canal									
2781	RD	2151.40	0.00050	0.030		TRAP	12.00	1.00			
2782	KO					22					

2783	KK	JSL520									
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2784	PB	1.09									
2785	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2786	PI	.015	.015	.140	.211	.094	.050	.050	.050	.034	.034
2787	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2788	PI	.009	.009	.009	.009	.009	.009				
2789	BA	.053									
2790	LU	.78	.60	38.60							
2791	IN	5									
2792	UI	0.	0.	0.	0.	0.	0.	0.	1.	1.	1.
2793	UI	1.	1.	1.	1.	1.	2.	2.	2.	6.	6.
2794	UI	6.	10.	10.	13.	13.	13.	17.	17.	17.	20.
2795	UI	20.	20.	24.	24.	24.	28.	28.	28.	32.	32.
2796	UI	35.	35.	35.	39.	39.	39.	43.	43.	43.	40.
2797	UI	40.	40.	38.	38.	34.	34.	34.	32.	32.	32.
2798	UI	29.	29.	29.	26.	26.	26.	23.	23.	23.	20.
2799	UI	20.	17.	17.	17.	14.	14.	14.	13.	13.	13.
2800	UI	11.	11.	11.	10.	10.	9.	9.	9.	8.	8.
2801	UI	8.	8.	8.	8.	7.	7.	7.	6.	6.	6.
2802	UI	6.	6.	5.	5.	5.	5.	5.	5.	5.	5.
2803	UI	5.	4.	4.	4.	4.	4.	4.	4.	4.	4.
2804	UI	4.	4.	4.	4.	4.	3.	3.	3.	3.	3.
2805	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2806	UI	3.	3.	2.	2.	2.	2.	2.	2.	2.	2.
2807	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2808	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2809	UI	2.	2.	2.	2.	2.	2.	1.	1.	1.	1.
2810	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2811	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2812	UI	1.									
2813	KK	COMMERCIAL DETENTION, DISCHARGE LIMITED TO 0.3 CFS/ACRE									
2814	RS	1	STOR	.00							
2815	SV	.0	.5	1.0	1.5						
2816	SQ	.0	4.0	4.0	4.0						
2817	SE	.0	1.0	2.0	3.0						
2818	KO					22					
2819	KK	HC334									
2820	HC	2									
2821	KO					22					
2822	KK	CV733	Jordan Salt Lake Canal								
2823	RD	2927.79	0.00050	0.030		TRAP	12.00	1.00			
2824	KO					22					
2825	KK	HC335									
2826	HC	2									
2827	KO					22					
2828	KK	JSL550									
2829	PB	1.09									
2830	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2831	PI	.015	.015	.140	.211	.094	.050	.050	.050	.034	.034
2832	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2833	PI	.009	.009	.009	.009	.009	.009				
2834	BA	.102									
2835	LU	.40	.75	51.00							
2836	IN	5									
2837	UI	0.	0.	0.	0.	1.	1.	1.	1.	1.	2.
2838	UI	2.	2.	3.	3.	5.	5.	5.	11.	11.	11.
2839	UI	19.	19.	19.	26.	26.	33.	33.	33.	40.	40.
2840	UI	40.	48.	48.	55.	55.	55.	63.	63.	63.	71.
2841	UI	71.	78.	78.	78.	86.	86.	86.	81.	81.	81.
2842	UI	75.	75.	69.	69.	69.	63.	63.	63.	57.	57.
2843	UI	51.	51.	51.	46.	46.	46.	39.	39.	39.	34.
2844	UI	34.	29.	29.	29.	26.	26.	26.	23.	23.	20.
2845	UI	20.	20.	18.	18.	18.	17.	17.	17.	15.	15.
2846	UI	14.	14.	14.	13.	13.	13.	12.	12.	11.	11.
2847	UI	11.	10.	10.	10.	10.	10.	9.	9.	9.	8.
2848	UI	8.	8.	8.	8.	8.	7.	7.	7.	7.	7.
2849	UI	7.	7.	7.	6.	6.	6.	6.	6.	6.	6.
2850	UI	6.	5.	5.	5.	5.	5.	5.	5.	5.	5.
2851	UI	5.	5.	5.	5.	4.	4.	4.	4.	4.	4.

2852	UI	4.	4.	4.	4.	4.	4.	4.	4.	3.	3.
2853	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2854	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2855	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2856	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2857	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2858	UI	2.	2.	1.	1.	1.	1.	1.	1.	1.	1.
2859	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2860	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2861	KK COMMERCIAL DETENTION, DISCHARGE LIMITED TO 0.3 CFS/ACRE										
2862	RS	1	STOR	.00							
2863	SV	.0	.5	2.0	4.0						
2864	SQ	.0	20.0	20.0	20.0						
2865	SE	.0	1.0	2.0	3.0						
2866	KO					22					
2867	KK CV736Jordan Salt Lake Canal										
2868	RD	1114.34	0.00050	0.030		TRAP	12.00	1.00			
2869	KO					22					
2870	KK HC359										
2871	HC	2									
2872	KO					22					
2873	KK JSL560										
2874	PB	1.09									
2875	PI	.009	.009	.009	.009	.009	.009	.015	.015	.015	.015
2876	PI	.015	.015	.140	.211	.094	.050	.050	.050	.034	.034
2877	PI	.034	.034	.034	.034	.015	.015	.015	.015	.015	.015
2878	PI	.009	.009	.009	.009	.009	.009				
2879	BA	.122									
2880	LU	.39	.74	52.73							
2881	IN	5									
2882	UI	1.	1.	1.	1.	2.	2.	3.	5.	5.	9.
2883	UI	9.	21.	21.	36.	48.	48.	62.	62.	75.	75.
2884	UI	88.	102.	102.	117.	117.	131.	131.	145.	159.	159.
2885	UI	149.	149.	139.	139.	127.	117.	117.	106.	106.	94.
2886	UI	94.	84.	72.	72.	62.	62.	54.	54.	47.	42.
2887	UI	42.	38.	38.	34.	34.	31.	28.	28.	26.	26.
2888	UI	24.	24.	22.	20.	20.	19.	19.	18.	18.	16.
2889	UI	15.	15.	15.	15.	14.	14.	13.	12.	12.	11.
2890	UI	11.	11.	11.	10.	10.	10.	10.	10.	9.	9.
2891	UI	9.	8.	8.	8.	8.	8.	8.	7.	7.	7.
2892	UI	7.	7.	6.	6.	6.	6.	6.	6.	6.	5.
2893	UI	5.	5.	5.	5.	5.	5.	5.	5.	4.	4.
2894	UI	4.	4.	4.	4.	4.	4.	3.	3.	3.	3.
2895	UI	3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
2896	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2897	UI	2.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2898	UI	2.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2899	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2900	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2901	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
2902	UI	1.	1.	1.	1.	1.	1.	1.	1.	1.	0.
2903	KK COMMERCIAL DETENTION, DISCHARGE LIMITED TO 0.3 CFS/ACRE										
2904	RS	1	STOR	.00							
2905	SV	.0	.5	2.0	4.0						
2906	SQ	.0	29.0	29.0	29.0						
2907	SE	.0	1.0	2.0	3.0						
2908	KO					22					
2909	KK CV771Jordan Salt Lake Canal										
2910	RD	730.047	0.00050	0.030		TRAP	12.00	1.00			
2911	KO					22					
2912	KK SBC32										
2913	BA	0.0919									
2914	LS	0.1	98.000			66.523	0.1				
2915	UK	25	.02	.1	18.93						
2916	UK	70	.02	.4	81.07						
2917	RD	1800	.018	.02		TRAP	2	25			

2918	KO						22		
2919	KK	HC372							
2920	HC	2							
2921	KO						22		
2922	KK	CV772	Jordan Salt Lake Canal						
2923	RD	2393.10	0.00050	0.030			TRAP	12.00	1.00
2924	KO						22		
2925	KK	SBC33							
2926	BA	0.0613							
2927	LS	0.1	98.000				75.236	0.1	
2928	UK	25	.02	.1	30.96				
2929	UK	70	.02	.4	69.04				
2930	RD	1000	.02	.02			TRAP	2	25
2931	KO						22		
2932	KK	HC373							
2933	HC	2							
2934	KO						22		
2935	KK	CV773	Jordan Salt Lake Canal						
2936	RD	2116.01	0.00050	0.030			TRAP	12.00	1.00
2937	KO						22		
2938	KK	HC374							
2939	HC	2							
2940	KO						22		
2941	KK	SBC8							
2942	BA	0.1474							
2943	LS	0.1	98.000				61.642	0.1	
2944	UK	25	.02	.1	20.18				
2945	UK	70	.02	.4	79.82				
2946	RD	2880	.018	.015			CIRC	2.5	25
2947	KO						22		
2948	KK	CV774	Jordan Salt Lake Canal						
2949	RD	1747.54	0.00050	0.030			TRAP	12.00	1.00
2950	KO						22		
2951	KK	SBC47							
2952	BA	0.0966							
2953	LS	0.1	98.000				64.464	0.1	
2954	UK	25	.02	.1	31.32				
2955	UK	70	.02	.4	68.68				
2956	RD	650	.02	.02	.023		TRAP	2	25
2957	RD	3000	.018	.02			TRAP	2	25
2958	KO						22		
2959	KK	HC375							
2960	HC	2							
2961	KO						22		
2962	KK	CV775	Jordan Salt Lake Canal						
2963	RD	526.226	0.00050	0.030			TRAP	12.00	1.00
2964	KO						22		
2965	KK	SBC46							
2966	BA	0.1328							
2967	LS	0.1	98.000				57.337	0.1	
2968	UK	25	.02	.1	23.8				
2969	UK	70	.02	.4	76.19				
2970	RD	800	.02	.02	.036		TRAP	2	25
2971	RD	2500	.016	.02			TRAP	2	25
2972	KO						22		
2973	KK	HC376							
2974	HC	2							
2975	KO						22		
2976	KK	CV776	Jordan Salt Lake Canal						
2977	RD	1144.16	0.00050	0.030			TRAP	12.00	1.00

2978	KO							22		
2979	KK	SB879								
2980	BA	0.0980								
2981	LS	0.1	98.000			58.927		0.1		
2982	UK	25	.02	.1	27.00					
2983	UK	70	.02	.4	73.00					
2984	RD	800	.02	.02	.032	TRAP		2	25	
2985	RD	2500	.016	.02		TRAP		2	25	
2986	KO					22				
2987	KK	HC377								
2988	HC	2								
2989	KO					22				
2990	KK	CV777	Jordan Salt Lake Canal							
2991	RD	1698.26	0.00050	0.030		TRAP		12.00	1.00	
2992	KO					22				
2993	KK	SB878								
2994	BA	0.0859								
2995	LS	0.1	98.000			52.771		0.1		
2996	UK	25	.02	.1	19.40					
2997	UK	70	.02	.4	80.60					
2998	RD	900	.02	.02	.029	TRAP		2	25	
2999	RD	1600	.02	.02		TRAP		2	25	
3000	KO					22				
3001	KK	HC403								
3002	HC	2								
3003	KO					22				
3004	KK	CV826	Jordan Salt Lake Canal							
3005	RD	678.338	0.00050	0.030		TRAP		12.00	1.00	
3006	KO					22				
3007	KK	DH42								
3008	DT	DR42								
3009	DI	0	25	50	100	200		400	800	
3010	DQ	0	25	50	100	200		400	800	
3011	KO					22				
3012	KK	CV827	Jordan Salt Lake Canal							
3013	RD	553.719	0.00050	0.030		TRAP		12.00	1.00	
3014	KO					22				
3015	KK	SBC								
3016	BA	0.1008								
3017	LS	0.1	98.000			64.578		0.1		
3018	UK	25	.02	.1	28.37					
3019	UK	70	.02	.4	71.63					
3020	RD	1500	.014	.04		TRAP		1	2	
3021	KO					22				
3022	KK	HC378								
3023	HC	2								
3024	KO					22				
3025	KK	CV778	Jordan Salt Lake Canal							
3026	RD	1660.02	0.00050	0.030		TRAP		12.00	1.00	
3027	KO					22				
3028	KK	SBC10								
3029	BA	0.121								
3030	LS	0.1	98.000			70.703		0.1		
3031	UK	25	.02	.1	30.22					
3032	UK	70	.02	.4	69.78					
3033	RD	1800	.009	.02		CIRC		1.5	25	
3034	KO					22				
3035	KK	CV803	to Jordan Salt Lake Canal							
3036	RD	1216.18	0.00050	0.030		CIRC		3.50	1.00	
3037	KO					22				

3038	KK	HC402						
3039	HC	2						
3040	KO				22			
3041	KK	CV802	Jordan Salt Lake Canal					
3042	RD	1376.60	0.00050	0.030	TRAP	12.00	1.00	
3043	KO				22			
3044	KK	SBC2						
3045	BA	0.1047						
3046	LS	0.1	98.000		51.963	0.1		
3047	UK	25	.02	.1	27.20			
3048	UK	70	.02	.4	72.80			
3049	RD	700	.02	.02	.015	TRAP	2	25
3050	RD	1900	.013	.02		TRAP	2	25
3051	KO				22			
3052	KK	HC379						
3053	HC	2						
3054	KO				22			
3055	KK	CV830	Jordan Salt Lake Canal					
3056	RD	4324.49	0.00050	0.030	TRAP	12.00	1.00	
3057	KO				22			
3058	KK	DH1						
3059	DT	DR1						
3060	DI	0	25	50	100	200	400	800
3061	DQ	0	25	50	100	200	400	800
3062	KO				22			
3063	KK	CV831	Jordan Salt Lake Canal					
3064	RD	3184.80	0.00050	0.030	TRAP	12.00	1.00	
3065	KO				22			
3066	KK	SBC1						
3067	BA	0.0912						
3068	LS	0.1	98.000		73.552	0.1		
3069	UK	25	.02	.1	29.76			
3070	UK	70	.02	.4	70.24			
3071	RD	2200	.009	.02		TRAP	2	25
3072	KO				22			
3073	KK	HC380						
3074	HC	2						
3075	KO				22			
3076	KK	CV809	Jordan Salt Lake Canal					
3077	RD	1961.34	0.00050	0.030	TRAP	12.00	1.00	
3078	KO				22			
3079	KK	SB882						
3080	BA	0.1958						
3081	LS	0.1	98.000		69.681	0.1		
3082	UK	25	.02	.1	26.76			
3083	UK	70	.02	.4	73.24			
3084	RD	1000	.02	.02	.03	TRAP	2	25
3085	RD	5100	.0095	.04		TRAP	1	2
3086	KO				22			
3087	KK	HC405						
3088	HC	2						
3089	KO				22			
3090	KK	CV810	Jordan Salt Lake Canal					
3091	RD	1961.18	0.00050	0.030	TRAP	12.00	1.00	
3092	KO				22			
3093	KK	SB883						
3094	BA	0.1747						
3095	LS	0.1	98.000		72.607	0.1		
3096	UK	25	.02	.1	28.95			
3097	UK	70	.02	.4	71.05			
3098	RD	650	.02	.02	.019	TRAP	2	25

3099	RD	4500	.009	.015		CIRC	3	
3100	KO					22		
3101	KK	HC38						
3102	HC	2						
3103	KO					22		
3104	KK	CV781	Jordan Salt Lake Canal					
3105	RD	1611.44	0.00050	0.030		TRAP	12.00	1.00
3106	KO					22		
3107	KK	SB885						
3108	BA	0.1088						
3109	LS	0.1	98.000			69.967	0.1	
3110	UK	25	.02	.1	24.9			
3111	UK	70	.02	.4	75.09			
3112	RD	700	.02	.02	.013	TRAP	2	25
3113	RD	1300	.013	.02		TRAP	2	25
3114	KO					22		
3115	KK	SB884						
3116	BA	0.087						
3117	LS	0.1	98.000			66.641	0.1	
3118	UK	25	.02	.1	17.15			
3119	UK	70	.02	.4	82.85			
3120	RD	600	.02	.02	.01	TRAP	2	25
3121	RD	1850	.018	.02		TRAP	2	25
3122	KO					22		
3123	KK	HC382						
3124	HC	3						
3125	KO					22		
3126	KK	CV782	Jordan Salt Lake Canal					
3127	RD	486.150	0.00050	0.030		TRAP	12.00	1.00
3128	KO					22		
3129	KK	SBC12						
3130	BA	0.1032						
3131	LS	0.1	98.000			66.004	0.1	
3132	UK	25	.02	.1	16.89			
3133	UK	70	.02	.4	83.1			
3134	RD	500	.02	.02	.015	TRAP	2	25
3135	RD	3100	.013	.02		TRAP	2	25
3136	KO					22		
3137	KK	HC383						
3138	HC	2						
3139	KO					22		
3140	KK	CV783	Jordan Salt Lake Canal					
3141	RD	1298.99	0.00050	0.030		TRAP	12.00	1.00
3142	KO					22		
3143	KK	SBC3						
3144	BA	0.1389						
3145	LS	0.1	98.000			65.658	0.1	
3146	UK	25	.02	.1	11.32			
3147	UK	70	.02	.4	88.68			
3148	RD	650	.02	.02	.019	TRAP	2	25
3149	RD	3200	.013	.02		TRAP	2	25
3150	KO					22		
3151	KK	HC384						
3152	HC	2						
3153	KO					22		
3154	KK	CV784	Jordan Salt Lake Canal					
3155	RD	3328.37	0.00050	0.030		TRAP	12.00	1.00
3156	KO					22		
3157	KK	SBC4						
3158	BA	0.1240						
3159	LS	0.1	98.000			66.117	0.1	

3160	UK	25	.02	.1	8.94			
3161	UK	70	.02	.4	91.06			
3162	RD	700	.02	.02	.015	TRAP	2	25
3163	RD	1650	.01	.02		TRAP	2	25
3164	KO					22		
3165	KK	HC385						
3166	HC	2						
3167	KO					22		
3168	KK	CV834Jordan Salt Lake Canal						
3169	RD	1021.80	0.00050	0.030		TRAP	12.00	1.00
3170	KO					22		
3171	KK	DH15						
3172	DT	DR15						
3173	DI	0	25	50	100	200	400	800
3174	DQ	0	25	50	100	200	400	800
3175	KO					22		
3176	KK	CV835Jordan Salt Lake Canal						
3177	RD	454.062	0.00050	0.030		TRAP	12.00	1.00
3178	KO					22		
3179	KK	SBC42						
3180	BA	0.1496						
3181	LS	0.1	98.000			59.186	0.1	
3182	UK	25	.02	.1	20.83			
3183	UK	70	.02	.4	79.17			
3184	RD	600	.02	.02	.02	TRAP	2	25
3185	RD	3200	.038	.02		TRAP	2	25
3186	KO					22		
3187	KK	SBC4						
3188	BA	0.1429						
3189	LS	0.1	98.000			67.247	0.1	
3190	UK	25	.02	.1	16.30			
3191	UK	70	.02	.4	83.70			
3192	RD	400	.02	.02	.016	TRAP	2	25
3193	RD	3000	.022	.02		TRAP	2	25
3194	KO					22		
3195	KK	HC386						
3196	HC	3						
3197	KO					22		
3198	KK	CV836Jordan Salt Lake Canal						
3199	RD	2099.51	0.00050	0.030		TRAP	12.00	1.00
3200	KO					22		
3201	KK	SBC40						
3202	BA	0.0874						
3203	LS	0.1	98.000			69.479	0.1	
3204	UK	25	.02	.1	22.68			
3205	UK	70	.02	.4	77.32			
3206	RD	500	.02	.02	.01	TRAP	2	25
3207	RD	2500	.023	.02		TRAP	2	25
3208	KO					22		
3209	KK	HC387						
3210	HC	2						
3211	KO					22		
3212	KK	CV787Jordan Salt Lake Canal						
3213	RD	1789.28	0.00050	0.030		TRAP	12.00	1.00
3214	KO					22		
3215	KK	SBC5						
3216	BA	0.0824						
3217	LS	0.1	98.000			69.628	0.1	
3218	UK	25	.02	.1	26.32			
3219	UK	70	.02	.4	73.68			
3220	RD	600	.02	.02	.013	TRAP	2	25
3221	RD	1000	.024	.02		TRAP	2	25

3222	KO					22		
3223	KK	HC388						
3224	HC	2						
3225	KO					22		
3226	KK	CV838	Jordan Salt Lake Canal					
3227	RD	1389.92	0.00050	0.030		TRAP	12.00	1.00
3228	KO					22		
3229	KK	SBC6						
3230	BA	0.099						
3231	LS	0.1	98.000			67.025	0.1	
3232	UK	25	.02	.1	19.99			
3233	UK	70	.02	.4	80.0			
3234	RD	1400	.02	.02	.05	TRAP	2	25
3235	RD	2000	.029	.02		TRAP	2	25
3236	KO					22		
3237	KK	HC389						
3238	HC	2						
3239	KO					22		
3240	KK	CV789	Jordan Salt Lake Canal					
3241	RD	2452.77	0.00050	0.030		TRAP	12.00	1.00
3242	KO					22		
3243	KK	SBC39						
3244	BA	0.0898						
3245	LS	0.1	98.000			69.199	0.1	
3246	UK	25	.02	.1	23.03			
3247	UK	70	.02	.4	76.97			
3248	RD	500	.02	.02	.018	TRAP	2	25
3249	RD	2200	.037	.04		TRAP	1	2
3250	KO					22		
3251	KK	SBC7						
3252	BA	0.0893						
3253	LS	0.1	98.000			66.398	0.1	
3254	UK	25	.02	.1	15.37			
3255	UK	70	.02	.4	84.63			
3256	RD	1200	.04	.02		TRAP	2	25
3257	KO					22		
3258	KK	HC390						
3259	HC	3						
3260	KO					22		
3261	KK	CV840	Jordan Salt Lake Canal					
3262	RD	3180.67	0.00050	0.030		TRAP	12.00	1.00
3263	KO					22		
3264	KK	CV841	Jordan Salt Lake Canal					
3265	RD	133.046	0.00050	0.030		TRAP	12.00	1.00
3266	KO					22		
3267	KK	SBC38						
3268	BA	0.056						
3269	LS	0.1	98.000			66.742	0.1	
3270	UK	25	.02	.1	15.93			
3271	UK	70	.02	.4	84.07			
3272	RD	1300	.065	.04		TRAP	1	2
3273	KO					22		
3274	KK	HC39						
3275	HC	2						
3276	KO					22		
3277	KK	CV842	Jordan Salt Lake Canal					
3278	RD	692.136	0.00050	0.030		TRAP	12.00	1.00
3279	KO					22		
3280	KK	DH19						
3281	DT	DR19						

3282	DI	0	25	50	100	200	400	800
3283	DQ	0	25	50	100	200	400	800
3284	KO					22		
3285	KK	CV843	Jordan Salt Lake Canal					
3286	RD	805.949	0.00050	0.030		TRAP	12.00	1.00
3287	KO					22		
3288	KK	SBC34						
3289	BA	0.1443						
3290	LS	0.1	98.000			66.766	0.1	
3291	UK	25	.02	.1	18.69			
3292	UK	70	.02	.4	81.3			
3293	RD	3500	.04	.04		TRAP	1	2
3294	KO					22		
3295	KK	HC392						
3296	HC	2						
3297	KO					22		
3298	KK	CV844	Jordan Salt Lake Canal					
3299	RD	177.532	0.00050	0.030		TRAP	12.00	1.00
3300	KO					22		
3301	KK	CV845	Jordan Salt Lake Canal					
3302	RD	508.973	0.00050	0.030		TRAP	12.00	1.00
3303	KO					22		
3304	KK	SBC35						
3305	BA	0.0820						
3306	LS	0.1	98.000			69.749	0.1	
3307	UK	25	.02	.1	25.65			
3308	UK	70	.02	.4	74.35			
3309	RD	3800	.04	.02		TRAP	2	25
3310	KO					22		
3311	KK	HC393						
3312	HC	2						
3313	KO					22		
3314	KK	CV846	Jordan Salt Lake Canal					
3315	RD	1469.64	0.00050	0.030		TRAP	12.00	1.00
3316	KO					22		
3317	KK	DH2						
3318	DT	DR2						
3319	DI	0	25	50	100	200	400	800
3320	DQ	0	25	50	100	200	400	800
3321	KO					22		
3322	KK	CV847	Jordan Salt Lake Canal					
3323	RD	714.150	0.00050	0.030		TRAP	12.00	1.00
3324	KO					22		
3325	KK	SBC36						
3326	BA	0.0408						
3327	LS	0.1	98.000			79.620	0.1	
3328	UK	25	.02	.1	30.59			
3329	UK	70	.02	.4	69.4			
3330	RD	600	.068	.04		TRAP	1	2
3331	KO					22		
3332	KK	HC394						
3333	HC	2						
3334	KO					22		
3335	KK	CV794	Jordan Salt Lake Canal					
3336	RD	3372.27	0.00050	0.030		TRAP	12.00	1.00
3337	KO					22		
3338	KK	SBB92						
3339	BA	0.0152						
3340	LS	0.1	98.000			85.624	0.1	
3341	UK	25	.02	.1	72.33			

3342	UK	70	.02	.4	27.67			
3343	RD	300	.027	.02		TRAP	2	25
3344	KO					22		
3345	KK	HC395						
3346	HC	2						
3347	KO					22		
3348	KK	CV795	Jordan Salt Lake Canal					
3349	RD	2306.96	0.00050	0.030		TRAP	12.00	1.00
3350	KO					22		
3351	KK	SB889						
3352	BA	0.0392						
3353	LS	0.1	98.000			74.017	0.1	
3354	UK	25	.02	.1	36.03			
3355	UK	70	.02	.4	63.97			
3356	RD	875	.028	.02		TRAP	2	25
3357	KO					22		
3358	KK	HC396						
3359	HC	2						
3360	KO					22		
3361	KK	CV811	Jordan Salt Lake Canal					
3362	RD	327.074	0.00050	0.030		DEEP	4.00	1.00
3363	KO					22		
3364	KK	SB89						
3365	BA	0.0536						
3366	LS	0.1	98.000			72.117	0.1	
3367	UK	25	.02	.1	32.96			
3368	UK	70	.02	.4	67.04			
3369	RD	700	.02	.02	.02	TRAP	2	25
3370	RD	790	.03	.02		TRAP	2	25
3371	KO					22		
3372	KK	HC406						
3373	HC	2						
3374	KO					22		
3375	KK	CV812	Jordan Salt Lake Canal					
3376	RD	1643.38	0.00050	0.030		DEEP	4.00	1.00
3377	KO					22		
3378	KK	SBC37						
3379	BA	0.0917						
3380	LS	0.1	98.000			69.980	0.1	
3381	UK	25	.02	.1	28.3			
3382	UK	70	.02	.4	71.69			
3383	RD	600	.025	.02	.01	TRAP	2	25
3384	RD	2200	.008	.02		CIRC	3	
3385	KO					22		
3386	KK	HC397						
3387	HC	2						
3388	KO					22		
3389	KK	CV848	Jordan Salt Lake Canal					
3390	RD	649.746	0.00050	0.030		DEEP	4.00	1.00
3391	KO					22		
3392	KK	CV849	Jordan Salt Lake Canal					
3393	RD	2458.50	0.00050	0.030		DEEP	4.00	1.00
3394	KO					22		
3395	KK	SBC25						
3396	BA	0.0770						
3397	LS	0.1	98.000			83.154	0.1	
3398	UK	25	.02	.1	61.80			
3399	UK	70	.02	.4	38.20			
3400	RD	500	.02	.04	.008	TRAP	2	25
3401	RD	1200	.04	.04		CIRC	2	
3402	KO					22		

3403	KK	HC398						
3404	HC	2						
3405	KO					22		
3406	KK	CV850	Jordan Salt Lake Canal					
3407	RD	2061.98	0.00050	0.030		DEEP	4.00	1.00
3408	KO					22		
3409	KK	CV851	Jordan Salt Lake Canal					
3410	RD	1102.84	0.00050	0.030		DEEP	4.00	1.00
3411	KO					22		
3412	KK	SBC26						
3413	BA	0.1115						
3414	LS	0.1	98.000			76.578	0.1	
3415	UK	25	.02	0.1	12.02			
3416	UK	70	.02	.4	87.98			
3417	RD	700	.01	.02	.007	TRAP	2	25
3418	RD	1400	.02	.02		TRAP	2	25
3419	KO					22		
3420	KK	HC399						
3421	HC	2						
3422	KO					22		
3423	KK	CV852	Jordan Salt Lake Canal					
3424	RD	1816.31	0.00050	0.030		DEEP	4.00	1.00
3425	KO					22		
3426	KK	CV853	Jordan Salt Lake Canal					
3427	RD	179.587	0.00050	0.030		DEEP	4.00	1.00
3428	KO					22		
3429	KK	SBC27						
3430	BA	0.1592						
3431	LS	0.1	98.000			76.806	0.1	
3432	UK	25	.02	.1	13.89			
3433	UK	70	.02	.4	86.1			
3434	RD	1000	.02	.02		TRAP	2	25
3435	KO					22		
3436	KK	HC400						
3437	HC	2						
3438	KO					22		
3439	KK	CV854	Jordan Salt Lake Canal					
3440	RD	4591.21	0.00050	0.030		DEEP	4.00	1.00
3441	KO					22		
3442	KK	CV855	Jordan Salt Lake Canal					
3443	RD	430.389	0.00050	0.030		DEEP	4.00	1.00
3444	KO					22		
3445	KK	SBC28						
3446	BA	0.1700						
3447	LS	0.1	98.000			71.392	0.1	
3448	UK	25	.02	.1	10.00			
3449	UK	65	.02	.4	90.00			
3450	RD	1800	.01	.02		TRAP	2	25
3451	KO					22		
3452	KK	HC40						
3453	HC	2						
3454	KO					22		
3455	KK	CV856	Jordan Salt Lake Canal					
3456	RD	1440.59	0.00050	0.030		DEEP	4.00	1.00
3457	KO					22		
3458	KK	CV857	Jordan Salt Lake Canal					
3459	RD	741.144	0.00050	0.030		DEEP	4.00	1.00
3460	KO					22		
3461	ZZ							

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW	
6	SBC19		
	V		
	V		
20	CV859		
	.		
23	.	SBC18	
	.	.	
	.	.	
31	HC368.....		
	V		
	V		
34	CV767		
	.		
	.		
37	.	SBC17	
	.	.	
	.	.	
45	HC369.....		
	V		
	V		
48	CV768		
	.		
	.		
51	.	SBC16	
	.	.	
	.	.	
59	HC370.....		
	V		
	V		
62	CV769		
	.		
	.		
66	.	-----> DR35	
65	DH35		
	.		
	.		
70	.	SBC15	
	.	.	
	.	.	
77	HC371.....		
	V		
	V		
80	CV870		
	.		
	.		
83	.	SB419	
	.	V	
	.	V	
91	.	CV539	
	.	.	
	.	.	
94	.	SB647	
	.	.	
	.	.	
101	.	.	SB851
	.	.	.
	.	.	.
108	.	HC212.....	
	.	V	
	.	V	
111	.	CV538	
	.	V	
	.	V	

114	.	CV537	.	
	.	.	.	
117	.		SB483	
	.		V	
	.		V	
124	.		DB29	
	.		V	
	.		V	
130	.		CV686	
	.		.	
	.		.	
133	.			SB484
	.		.	
	.		.	
140	.		HC310.....	
	.		V	
	.		V	
143	.		CV687	
	.		V	
	.		V	
146	.		CV688	
	.		V	
	.		V	
149	.		CV689	
	.		V	
	.		V	
152	.		CV690	
	.		V	
	.		V	
155	.		DB78	
	.		.	
	.		.	
161	.			SB479
	.		.	
	.		.	
168	.		HC311.....	
	.		V	
	.		V	
171	.		CV715	
	.		.	
	.		.	
174	.			SB475
	.		.	
	.		.	
181	.		HC312.....	
	.		V	
	.		V	
184	.		CV717	
	.		.	
	.		.	
187	.			SB477
	.		.	V
	.		.	V
195	.			DB80
	.		.	
	.		.	
201	.			SB478
	.		.	
	.		.	
208	.			HC314.....
	.		.	V
	.		.	V
211	.			CV713
	.		.	V
	.		.	V
214	.			DB79
	.		.	
	.		.	
220	.			SB837

227	.	.	.	HC313.....	.
	.	.	.	V	.
	.	.	.	V	.
230	.	.	.	CV716	.

233	.	.	.	HC17.....	.
	.	.	.	V	.
	.	.	.	V	.
236	.	.	.	CV726	.

239	.	.	.	SB474	.
	.	.	.	V	.
	.	.	.	V	.
247	.	.	.	CV674	.
	.	.	.	V	.
	.	.	.	V	.
250	.	.	.	CV675	.
	.	.	.	V	.
	.	.	.	V	.
253	.	.	.	CV673	.
	.	.	.	V	.
	.	.	.	V	.
256	.	.	.	CV685	.
	.	.	.	V	.
	.	.	.	V	.
259	.	.	.	CV684	.
	.	.	.	V	.
	.	.	.	V	.
262	.	.	.	CV676	.

265	SB649
	V
	V
273	CV677

276	.	.	.	HC216.....	.
	.	.	.	V	.
	.	.	.	V	.
279	.	.	.	CV678	.
	.	.	.	V	.
	.	.	.	V	.
282	.	.	.	CV679	.

285	SB648
	V
	V
293	CV680

296	.	.	.	HC215.....	.
	.	.	.	V	.
	.	.	.	V	.
299	.	.	.	CV681	.
	.	.	.	V	.
	.	.	.	V	.
302	.	.	.	CV682	.

305	SB788

312	.	.	.	HC214.....	.
	.	.	.	V	.

315	.	.	V				
	.	.	DB81				
	.	.	V				
	.	.	V				
321	.	.	CV683				
	.	.	.				
	.	.	.				
324	.	HC213.....	.				
	.	.	V				
	.	.	V				
327	.	CV385	.				
	.	.	.				
330	.	.	SB789				
	.	.	.				
	.	.	.				
338	.	.	.	SB787			
			
			
346	SB786		
		
		
353	SB785	
	
	
361	.	HC217.....	.				
	.	.	V				
	.	.	V				
364	.	CV348	.				
	.	.	.				
367	.	.	SB874				
	.	.	.				
	.	.	.				
374	.	HCL.....	.				
	.	.	V				
	.	.	V				
377	.	CV347	.				
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380	.	.	SB896				
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387	.	.	CV346				
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	.	.	.				
390	.	HC2.....	.				
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	.	.	V				
393	.	CV345	.				
	.	.	.				
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397	.	----->	DR36				
396	.	DH36	.				
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401	.	.	SB169				
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408	.	HC218.....	.				
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411	.	CV276	.				
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414	.	.	SB804				
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421	.	.	CV272				
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424	.	HC3.....	.
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427	.	CV273	.
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430	.	SB805	.
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437	.	CV206	.
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440	.	DB35	.
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446	.	CV207	.
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449	.	.	SB806
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457	.	HC219.....	.
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460	.	CV696	.
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	.	V	.
463	.	CV342	.
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	.	V	.
466	.	CV695	.
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	.	.	.
469	.	HC4.....	.
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	.	V	.
472	.	CV340	.
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475	.	SB776	.
	.	V	.
	.	V	.
482	.	CV339	.
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485	.	.	SB875
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	.	.	.
492	.	HC220.....	.
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495	.	CV338	.
	.	V	.
	.	V	.
498	.	CV337	.
	.	.	.
	.	.	.
501	.	.	SB775
	.	.	.
	.	.	.
509	.	HC221.....	.
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	.	V	.
512	.	CV336	.
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	.	.	.
515	.	.	SB773
	.	.	.
	.	.	.

522	.	HC222.....		
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525	.	CV335		
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528	.	SB772		
	.	V		
	.	V		
536	.	CV239		
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	.	.		
539	.	.	SB763	
	.	.	.	
	.	.	.	
546	.	.	.	SB771

553	.	.	HC325.....	
	.	.	V	
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556	.	.	CV246	
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	.	.	V	
559	.	.	CV237	
	.	.	V	
	.	.	V	
562	.	.	CV238	
	.	.	.	
	.	.	.	
565	.	.	HC224.....	
	.	.	V	
	.	.	V	
568	.	.	CV240	
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	.	.	.	
571	.	.	SB770	
	.	.	.	
	.	.	.	
578	.	HC223.....		
	.	V		
	.	V		
581	.	CV234		
	.	.		
	.	.		
584	.	SB769		
	.	.		
	.	.		
592	.	HC226.....		
	.	V		
	.	V		
595	.	CV228		
	.	.		
	.	.		
598	.	SB60		
	.	V		
	.	V		
605	.	CV135		
	.	V		
	.	V		
608	.	CV136		
	.	.		
	.	.		
611	.	.	SB762	
	.	.	.	
	.	.	.	
618	.	HC227.....		
	.	V		
	.	V		
621	.	CV138		

		V	
		V	
624		CV139	
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		.	
627		.	SB764
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634		HC228.....	
		V	
		V	
637		CV140	
		V	
		V	
640		CV141	
		.	
		.	
643		.	SB766
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650		HC229.....	
		V	
		V	
653		CV129	
		V	
		V	
656		CV130	
		V	
		V	
659		DB10	
		V	
		V	
665		CV131	
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		.	
668		.	SB767
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676		.	SB768
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683		HC230.....	
		V	
		V	
686		CV132	
		V	
		V	
689		CV133	
		V	
		V	
692		CV134	
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695		.	SB756
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702		HC225.....	
		V	
		V	
705		CV227	
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708		.	SB754
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715		HC231.....	
		V	
		V	
718		CV232	
		.	

721	.	.	SB548	.	.	.
729	.	HC232
	.	V		.	.	.
	.	V		.	.	.
732	.	CV384		.	.	.
735	.	.	TP010	.	.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
741	.	.	RTP011	.	.	.
743	.	.	.	TP020	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
749	.	.	RTP021	.	.	.
751	TP030	.
757	.	.	CTP031
	.	.	V	.	.	.
	.	.	V	.	.	.
759	.	.	RTP033	.	.	.
761	.	.	.	TP100	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
767	.	.	RTP101	.	.	.
769	TP110	.
775	TP120

781	.	.	.	TP121
	.	.	.	V	.	.
	.	.	.	V	.	.
783	.	.	ALTHDB	.	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
788	.	.	RTP123	.	.	.
790	TP130	.
796	.	.	CTP131
	.	.	V	.	.	.
	.	.	V	.	.	.
798	.	.	RTP133	.	.	.
800	.	.	.	TP200	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
806	.	.	RTP201	.	.	.
808	TP210	.

814	.	.	CTP211.....		
	.	.	.		
816	.	.		TP300	
	.	.		V	
	.	.		V	
822	.	.		RTP301	
	.	.		.	
824	.	.			TP310

830	.	.		CTP311.....	
	.	.		V	
	.	.		V	
832	.	.		RTP312	
	.	.		.	
834	.	.			TP330
	.	.		.	V
	.	.		.	V
840	.	.			RTP331

842	.	.			
	.	.		.	TP320

848	.	.		CTP321.....	
	.	.		V	
	.	.		V	
850	.	.		RTP323	
	.	.		V	
	.	.		V	
852	.	.		RTP324	
	.	.		.	
854	.	.		CTP325.....	
	.	.		V	
	.	.		V	
856	.	.		STMDET	
	.	.		.	
861	.	.			TP450

867	.	.		CTP451.....	
	.	.		V	
	.	.		V	
869	.	.		RTP452	
	.	.		.	
871	.	.			TP400
	.	.		.	V
	.	.		.	V
877	.	.			RTP401

879	.	.			
	.	.		.	TP410

885	.	.		CTP411.....	
	.	.		V	
	.	.		V	
887	.	.			RTP412

889	.	.			
	.	.		.	TP430

895	.	.			
	.	.		.	TP420

901	.	.	.	CTP421.....	.
	.	.	.	V	.
	.	.	.	V	.
903	.	.	.	RTP423	.
	.	.	.	V	.
	.	.	.	V	.
905	.	.	.	AELDET	.
	.	.	.	V	.
	.	.	.	V	.
910	.	.	.	RTP425	.

912	TP440

918	.	.	.	CTP441.....	.
	.	.	.	V	.
	.	.	.	V	.
920	.	.	.	RTP442	.

922	TP460

928	.	.	.	CTP461.....	.
	.	.	.	V	.
	.	.	.	V	.
930	.	.	.	RTP463	.

932	TP470

938	.	.	.	CTP471.....	.
	.	.	.	V	.
	.	.	.	V	.
940	.	.	.	TPDET	.
	.	.	.	V	.
	.	.	.	V	.
946	.	.	.	CV150	.

949	SB744

956	SB745

963	.	.	.	HC235.....	.
	.	.	.	V	.
	.	.	.	V	.
966	.	.	.	CV151	.
	.	.	.	V	.
	.	.	.	V	.
969	.	.	.	CV152	.

972	SB749
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	V
980	.	.	.	CV264	.
	.	.	.	V	.
	.	.	.	V	.
983	.	.	.	CV156	.

986	SB750

994	.	.	.	HC240.....	.
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	.	.	.	V	.
997	.	.	.	CV158	.
	.	.	.	V	.
	.	.	.	V	.
1000	.	.	.	CV160	.

1003	SB748

1011	SB747

1019	.	.	.	HC237.....	.
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	.	.	.	V	.
1022	.	.	.	DB3	.
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	.	.	.	V	.
1028	.	.	.	CV162	.

1031	SB752

1038	.	.	.	HC238.....	.
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	.	.	.	V	.
1041	.	.	.	DB4	.
	.	.	.	V	.
	.	.	.	V	.
1047	.	.	.	CV154	.

1050	SB746

1057	.	.	.	HC236.....	.
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	.	.	.	V	.
1060	.	.	.	DB5	.

1066	SB753

1073	.	.	.	HC239.....	.
	.	.	.	V	.
	.	.	.	V	.
1076	.	.	.	CV155	.

1079	SB549

1086	.	.	.	HC233.....	.
	.	.	.	V	.
	.	.	.	V	.
1089	.	.	.	CV383	.

1092	SB741

1099	.	.	.	HC243.....	.
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	.	.	.	V	.

1102	.	CV723	.	
	.	.	.	
1105	.		SB735	
	.		V	
	.		V	
1113	.		CV382	
	.		.	
1116	.		.	SB739
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	.		.	.
1123	.		HC244.....	
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	.		V	
1126	.		CV178	
	.		.	
	.		.	SB738
1129	.		.	V
	.		.	V
1137	.		.	CV179
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1140	.		HC245.....	
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	.		V	
1143	.		CV176	
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	.		.	SB740
1146	.		.	V
	.		.	V
1154	.		.	CV164
	.		.	V
	.		.	V
1157	.		.	CV165
	.		.	V
	.		.	V
1160	.		.	CV166
	.		.	V
	.		.	V
1163	.		.	CV167
	.		.	.
	.		.	.
1166	.		HC246.....	
	.		V	
	.		V	
1169	.		DB6	
	.		V	
	.		V	
1175	.		CV168	
	.		.	
	.		.	
1178	.	HC242.....	.	
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1181	.	CV724	.	
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1184	.		.	.
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1191	.	HC241.....	.	
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1194	.	CV378	.	
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1197	.		SB736	

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1205	.	.	CV182	
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1208	.	.	.	SB737
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1216	.	.	HC249.....	
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1219	.	.	CV184	
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1222	.	.	CV186	
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1225	.	.	.	SB655
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	.	.	.	V
1233	.	.	.	CV189
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1236	.	.	HC248.....	
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	.	.	V	
1239	.	.	DB7	
	.	.	V	
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1245	.	.	CV380	
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1248	.	.	HC247.....	
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1251	.	.	CV379	
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1254	.	.	.	SB255
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1261	.	.	HC250.....	
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1264	.	.	CV499	
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1268	.	.	----->	DR37
1267	.	.	DH37	
	.	.	.	
1272	.	.	EJS010	
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1307	.	.	RJS011	
	.	.	.	
1309	.	.	.	EJS020
	.	.	.	
1334	.	.	CJS021.....	
	.	.	V	
	.	.	V	
1336	.	.	RJS022	
	.	.	V	
	.	.	V	
1338	.	.	RJS023	
	.	.	.	
1340	.	.	.	EJS030

1358	EJS040
1387	CJS102.....
1390	.	HC338.....	.	.	
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1393	.	CV863	.	.	
1396	.	.	CP010	.	
	.	.	V	.	
	.	.	V	.	
1428	.	.	RCP011	.	
1430	.	.	.	CP020	
1454	CP030
1486	.	.	CCP031.....	.	
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	.	.	V	.	
1488	.	.	RCP032	.	
	.	.	V	.	
	.	.	V	.	
1490	.	.	CRSTD	.	
	.	.	V	.	
	.	.	V	.	
1496	.	.	RCP034	.	
1498	.	.	.	EJS050	
1531	.	.	CCP102.....	.	
1534	.	HC339.....	.	.	
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1537	.	CV738	.	.	
1540	.	.	EJS060	.	
1562	.	HC340.....	.	.	
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	.	V	.	.	
1565	.	CV739	.	.	
1568	HC341.....	.	.	.	
1571	.	EJS090	.	.	
	.	V	.	.	
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1600	.	CV815	.	.	
1604	.	.	----->	DR4	
1603	.	DH4	.	.	

1608	V		
	V		
	CV816		
1611		EJC003	
		V	
		V	
1640		RJC004	
1642			EJC005
1665		CJC006
1668		HC343
	V		
	V		
1671		CV742	
1674		EJC010	
		V	
		V	
1713		REJ011	
		V	
		V	
1715		REJ012	
1717			EJC020
1741		CEJ021
1744		HC344
1747		CV743	
1750		EJC040	
		V	
		V	
1779		CV745	
1782			EJC050
1804		HC347
		V	
		V	
1807		CV820	
1811			-----> DR6
1810		DH6	
		V	
		V	
1815		CV821	
1818			EJC055
			V
			V
1851			RJC056

1855	.	.	HC348.....
	.	.	V
	.	.	V
1858	.	.	CV747
	.	.	.
1861	.	HC349.....	.
	.	.	.
1864	.	EJC075	.
	.	V	.
	.	V	.
1892	.	CV750	.
	.	.	.
1895	.	.	EJC080
	.	.	.
1924	.	HC352.....	.
	.	V	.
	.	V	.
1927	.	CV751	.
	.	.	.
1930	.	.	EJC083
	.	.	V
	.	.	V
1965	.	.	RJC084
	.	.	.
1968	.	.	EJC086
	.	.	.
1999	.	.	CJC087.....
	.	.	.
2003	.	HC353.....	.
	.	V	.
	.	V	.
2006	.	CV752	.
	.	.	.
2009	.	.	EJC090
	.	.	.
2044	.	HC354.....	.
	.	V	.
	.	V	.
2047	.	CV753	.
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2050	.	HC355.....	.
	.	.	.
2053	.	SBC43	.
	.	V	.
	.	V	.
2061	.	CV824	.
	.	.	.
2064	.	.	SBC44
	.	.	.
2071	.	HC360.....	.
	.	V	.
	.	V	.
2074	.	CV864	.
	.	.	.

2077	.	.	SB881	.

2084	.	HC361.....	.	.
	.	V	.	.
	.	V	.	.
2087	.	CV808	.	.

2090	.	.	SB880	.

2098	.	HC404.....	.	.
	.	V	.	.
	.	V	.	.
2101	.	CV872	.	.

2104	.	.	SB894	.
	.	.	V	.
	.	.	V	.
2112	.	.	CV860	.

2115	.	.	.	SB895

2122	.	.	HC408.....	.
	.	.	V	.
	.	.	V	.
2125	.	.	CV813	.

2128	.	.	.	SBC31

2136	.	.	.	SBC23

2144	.	.	HC362.....	.
	.	.	V	.
	.	.	V	.
2147	.	.	CV761	.

2150	.	.	.	SBC20

2157	.	.	HC363.....	.
	.	.	V	.
	.	.	V	.
2160	.	.	CV762	.

2163	.	.	.	SBC21

2171	.	.	HC364.....	.
	.	.	V	.
	.	.	V	.
2174	.	.	CV763	.

2177	.	.	.	SBC30

2184	.	.	HC365.....	.
	.	.	V	.
	.	.	V	.

2187	.	CV832	.	
2190	.		.	SB573
	.		.	V
	.		.	V
2197	.		.	CV535
2200	.		.	SB572
2207	.		.	SB571
2214	.		.	HC251.....
	.		.	V
	.		.	V
2217	.		.	CV534
2220	.		.	SB376
2227	.		.	SB374
2234	.		.	HC326.....
	.		.	V
	.		.	V
2237	.		.	CV474
2240	.		.	SB381
2247	.		.	SB382
2254	.		.	HC252.....
	.		.	V
	.		.	V
2257	.		.	CV475
2260	.		.	SB574
2267	.		.	HC253.....
	.		.	V
	.		.	V
2270	.		.	CV471
2273	.		.	SB575
2280	.		.	HC254.....
	.		.	V
	.		.	V
2283	.		.	CV470
2286	.		.	SB576
2293	.		.	HC255.....
	.		.	V
	.		.	V
2296	.		.	CV472

2299	SB577	.

2306	HC256.....	.
	V	.
	V	.
2309	CV478	.

2312	SB579	.

2319	HC257.....	.
	V	.
	V	.
2322	CV479	.

2325	SB578	.

2332	HC258.....	.
	V	.
	V	.
2335	CV480	.

2338	SB609	.

2345	HC259.....	.
	V	.
	V	.
2348	CV865	.

2352	----->	DR41
2351	DH41	.
	V	.
	V	.
2356	CV866	.

2359	SB358	.

2366	HC260.....	.
	V	.
	V	.
2369	CV482	.

2372	SB595	.

2379	SB262

2386	HC274.....	.
	V	.
	V	.
2389	CV486	.
	V	.
	V	.
2392	CV488	.
	V	.
	V	.
2395	CV487	.

2398	SB602
2406	.	.	.	HC262	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2409	.	.	.	CV485	.	.
2412	SB877	.
2419	.	.	.	HC263	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2422	.	.	.	CV489	.	.
2425	SB206	.
2433	.	.	.	HC264	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2436	.	.	.	CV490	.	.
2439	SB593	.
	V	.
	V	.
2446	CV215	.
	V	.
	V	.
2449	DB38	.
2455	.	.	.	HC265	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2458	.	.	.	CV491	.	.
2461	.	.	.	HC266	.	.
2464	.	.	.	JSL010	.	.
2486	.	.	.	JSL020	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2512	.	.	.	RJS021	.	.
2514	.	.	.	CCP103	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2517	.	.	.	CV727	.	.
2520	.	.	.	JSL030	.	.
2548	.	.	.	HC328	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
2551	.	.	.	CV728	.	.

2554	.	.	.	JSL040	.

2583	.	.	HC329.....	.	.
	.	.	V	.	.
	.	.	V	.	.
2586	.	.	CV729	.	.

2589	.	.	HC330.....	.	.

2592	.	.	JSL050	.	.
	.	.	V	.	.
	.	.	V	.	.
2619	.	.	CV730	.	.

2623	.	.	.	JSL060	.

2652	.	.	HC331.....	.	.
	.	.	V	.	.
	.	.	V	.	.
2655	.	.	CV867	.	.

2659	.	.	.	----->	DR3
2658	.	.	DH3	.	.
	.	.	V	.	.
	.	.	V	.	.
2663	.	.	CV819	.	.

2666	.	.	.	JSL200	.
	.	.	.	V	.
	.	.	.	V	.
2696	.	.	.	RJS201	.

2699	.	.	HC332.....	.	.
	.	.	V	.	.
	.	.	V	.	.
2702	.	.	CV757	.	.

2705	.	.	.	JSL500	.
	.	.	.	V	.
	.	.	.	V	.
2736	.	.	.	RJS501	.

2738	JSL510
	V
	V
2769	COMMER

2774	.	.	.	CCP103.....	.

2777	.	.	HC333.....	.	.
	.	.	V	.	.
	.	.	V	.	.
2780	.	.	CV732	.	.

2783	.	.	.	JSL520	.
	.	.	.	V	.

2813	.	.	.	V
	.	.	.	COMMER

2819	.	.	HC334
	.	.	V	
	.	.	V	
2822	.	.	CV733	
	.	.	.	
2825	.	.	HC335
	.	.	.	
2828	.	.	JSL550	
	.	.	V	
	.	.	V	
2861	.	.	COMMER	
	.	.	V	
	.	.	V	
2867	.	.	CV736	
	.	.	.	
2870	.	.	HC359
	.	.	.	
2873	.	.	JSL560	
	.	.	V	
	.	.	V	
2903	.	.	COMMER	
	.	.	V	
	.	.	V	
2909	.	.	CV771	
	.	.	.	
2912	.	.	.	SBC32

2919	.	.	HC372
	.	.	V	
	.	.	V	
2922	.	.	CV772	
	.	.	.	
2925	.	.	.	SBC33

2932	.	.	HC373
	.	.	V	
	.	.	V	
2935	.	.	CV773	
	.	.	.	
2938	.	.	HC374
	.	.	.	
2941	.	.	SBC8	
	.	.	V	
	.	.	V	
2948	.	.	CV774	
	.	.	.	
2951	.	.	.	SBC47

2959	.	.	HC375
	.	.	V	
	.	.	V	
2962	.	.	CV775	
	.	.	.	

2965	.	.	.	SBC46

2973	.	.	HC376.....	.
	.	.	V	.
	.	.	V	.
2976	.	.	CV776	.

2979	.	.	.	SB879

2987	.	.	HC377.....	.
	.	.	V	.
	.	.	V	.
2990	.	.	CV777	.

2993	.	.	.	SB878

3001	.	.	HC403.....	.
	.	.	V	.
	.	.	V	.
3004	.	.	CV826	.

3008	.	.	.	-----> DR42
3007	.	.	DH42	.
	.	.	V	.
	.	.	V	.
3012	.	.	CV827	.

3015	.	.	.	SBC1

3022	.	.	HC378.....	.
	.	.	V	.
	.	.	V	.
3025	.	.	CV778	.

3028	.	.	.	SBC10
	.	.	.	V
	.	.	.	V
3035	.	.	.	CV803

3038	.	.	HC402.....	.
	.	.	V	.
	.	.	V	.
3041	.	.	CV802	.

3044	.	.	.	SBC2

3052	.	.	HC379.....	.
	.	.	V	.
	.	.	V	.
3055	.	.	CV830	.

3059	.	.	.	-----> DR11
3058	.	.	DH11	.
	.	.	V	.
	.	.	V	.
3063	.	.	CV831	.

3066	.	.	.	SBC11	.

3073	.	.	HC380
	.	.	V		.
	.	.	V		.
3076	.	.	CV809		.

3079	.	.	.	SB882	.

3087	.	.	HC405
	.	.	V		.
	.	.	V		.
3090	.	.	CV810		.

3093	.	.	.	SB883	.

3101	.	.	HC381
	.	.	V		.
	.	.	V		.
3104	.	.	CV781		.

3107	.	.	.	SB885	.

3115	SB884

3123	.	.	HC382
	.	.	V		.
	.	.	V		.
3126	.	.	CV782		.

3129	.	.	.	SBC12	.

3137	.	.	HC383
	.	.	V		.
	.	.	V		.
3140	.	.	CV783		.

3143	.	.	.	SBC3	.

3151	.	.	HC384
	.	.	V		.
	.	.	V		.
3154	.	.	CV784		.

3157	.	.	.	SBC4	.

3165	.	.	HC385
	.	.	V		.
	.	.	V		.
3168	.	.	CV834		.

3172	.	.	----->	DR15	.
3171	.	.	DH15		.
	.	.	V		.

3176	.	.	V		
	.	.	CV835		
	.	.	.		
3179	.	.	.	SBC42	
	
3187	SBC41

3195	.	.	HC386	
	.	.	V		
	.	.	V		
3198	.	.	CV836		
	.	.	.		
3201	.	.	.	SBC40	
	
3209	.	.	HC387	
	.	.	V		
	.	.	V		
3212	.	.	CV787		
	.	.	.		
3215	.	.	.	SBC5	
	
3223	.	.	HC388	
	.	.	V		
	.	.	V		
3226	.	.	CV838		
	.	.	.		
3229	.	.	.	SBC6	
	
3237	.	.	HC389	
	.	.	V		
	.	.	V		
3240	.	.	CV789		
	.	.	.		
3243	.	.	.	SBC39	
	
3251	SBC7

3258	.	.	HC390	
	.	.	V		
	.	.	V		
3261	.	.	CV840		
	.	.	V		
	.	.	V		
3264	.	.	CV841		
	.	.	.		
3267	.	.	.	SBC38	
	
3274	.	.	HC391	
	.	.	V		
	.	.	V		
3277	.	.	CV842		
	.	.	.		
3281	.	.	.	----->	DR19
3280	.	.	DH19		
	.	.	V		

3285	.	.	V	
	.	.	CV843	
	.	.	.	
3288	.	.	.	SBC34

3295	.	.	HC392.....	.
	.	.	V	
	.	.	V	
3298	.	.	CV844	
	.	.	V	
	.	.	V	
3301	.	.	CV845	
	.	.	.	
3304	.	.	.	SBC35

3311	.	.	HC393.....	.
	.	.	V	
	.	.	V	
3314	.	.	CV846	
	.	.	.	
3318	.	.	----->	DR21
3317	.	.	DH21	
	.	.	V	
	.	.	V	
3322	.	.	CV847	
	.	.	.	
3325	.	.	.	SBC36

3332	.	.	HC394.....	.
	.	.	V	
	.	.	V	
3335	.	.	CV794	
	.	.	.	
3338	.	.	.	SB892

3345	.	.	HC395.....	.
	.	.	V	
	.	.	V	
3348	.	.	CV795	
	.	.	.	
3351	.	.	.	SB889

3358	.	.	HC396.....	.
	.	.	V	
	.	.	V	
3361	.	.	CV811	
	.	.	.	
3364	.	.	.	SB891

3372	.	.	HC406.....	.
	.	.	V	
	.	.	V	
3375	.	.	CV812	
	.	.	.	
3378	.	.	.	SBC37

3386	.	.	HC397.....	.
	.	.	V	.
	.	.	V	.
3389	.	.	CV848	.
	.	.	V	.
	.	.	V	.
3392	.	.	CV849	.

3395	.	.	.	SBC25

3403	.	.	HC398.....	.
	.	.	V	.
	.	.	V	.
3406	.	.	CV850	.
	.	.	V	.
	.	.	V	.
3409	.	.	CV851	.

3412	.	.	.	SBC26

3420	.	.	HC399.....	.
	.	.	V	.
	.	.	V	.
3423	.	.	CV852	.
	.	.	V	.
	.	.	V	.
3426	.	.	CV853	.

3429	.	.	.	SBC27

3436	.	.	HC400.....	.
	.	.	V	.
	.	.	V	.
3439	.	.	CV854	.
	.	.	V	.
	.	.	V	.
3442	.	.	CV855	.

3445	.	.	.	SBC28

3452	.	.	HC401.....	.
	.	.	V	.
	.	.	V	.
3455	.	.	CV856	.
	.	.	V	.
	.	.	V	.
3458	.	.	CV857	.

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SBC19	134.	1.18	11.	11.	11.	.52		
ROUTED TO	CV859	70.	1.18	10.	10.	10.	.52		
HYDROGRAPH AT	SBC18	68.	1.18	5.	5.	5.	.16		
2 COMBINED AT	HC368	138.	1.18	15.	15.	15.	.68		
ROUTED TO	CV767	33.	1.58	13.	13.	13.	.68		
HYDROGRAPH AT	SBC17	71.	1.25	8.	8.	8.	.19		
2 COMBINED AT	HC369	95.	1.25	21.	21.	21.	.87		
ROUTED TO	CV768	44.	2.02	19.	19.	19.	.87		
HYDROGRAPH AT	SBC16	107.	1.30	14.	14.	14.	.36		
2 COMBINED AT	HC370	129.	1.30	33.	33.	33.	1.23		
ROUTED TO	CV769	59.	2.23	26.	26.	26.	1.23		
DIVERSION TO	DR35	59.	.00	26.	26.	26.	1.23		
HYDROGRAPH AT	DH35	0.	.00	0.	0.	0.	1.23		
HYDROGRAPH AT	SBC15	158.	1.18	14.	14.	14.	.30		
2 COMBINED AT	HC371	158.	1.18	14.	14.	14.	1.53		
ROUTED TO	CV870	67.	1.18	12.	12.	12.	1.53		
HYDROGRAPH AT	SB419	68.	1.38	13.	13.	13.	.14		
ROUTED TO	CV539	42.	1.50	12.	12.	12.	.14		
HYDROGRAPH AT	SB647	24.	1.17	2.	2.	2.	.04		
HYDROGRAPH AT	SB851	4.	1.18	0.	0.	0.	.02		
3 COMBINED AT	HC212	47.	1.50	15.	15.	15.	.20		
ROUTED TO	CV538	37.	1.68	14.	14.	14.	.20		
ROUTED TO	CV537	34.	1.88	13.	13.	13.	.20		
HYDROGRAPH AT	SB483	7.	1.17	1.	1.	1.	.02		
ROUTED TO	DB29	2.	1.28	1.	1.	1.	.02	1.74	1.28
ROUTED TO	CV686	2.	1.28	1.	1.	1.	.02		
HYDROGRAPH AT	SB484	20.	1.18	2.	2.	2.	.05		
2 COMBINED AT	HC310	22.	1.18	2.	2.	2.	.07		
ROUTED TO	CV687	22.	1.18	2.	2.	2.	.07		
ROUTED TO	CV688	21.	1.18	2.	2.	2.	.07		
ROUTED TO	CV689	21.	1.20	2.	2.	2.	.07		
ROUTED TO	CV690	21.	1.20	2.	2.	2.	.07		
ROUTED TO	DB78	4.	1.73	2.	2.	2.	.07	2.75	1.75
HYDROGRAPH AT	SB479	9.	1.17	1.	1.	1.	.02		
2 COMBINED AT	HC311	11.	1.17	3.	3.	3.	.09		
ROUTED TO	CV715	11.	1.18	3.	3.	3.	.09		
HYDROGRAPH AT	SB475	44.	1.17	3.	3.	3.	.03		
2 COMBINED AT	HC312	54.	1.17	6.	6.	6.	.12		
ROUTED TO	CV717	54.	1.18	6.	6.	6.	.12		
HYDROGRAPH AT	SB477	16.	1.17	1.	1.	1.	.03		

ROUTED TO	DB80	4.	1.32	1.	1.	1.	.03	2.57	1.32
HYDROGRAPH AT	SB478	12.	1.17	1.	1.	1.	.04		
2 COMBINED AT	HC314	15.	1.17	2.	2.	2.	.07		
ROUTED TO	CV713	15.	1.18	2.	2.	2.	.07		
ROUTED TO	DE79	4.	2.05	2.	2.	2.	.07	2.47	2.07
HYDROGRAPH AT	SB837	25.	1.17	3.	3.	3.	.04		
2 COMBINED AT	HC313	27.	1.17	5.	5.	5.	.11		
ROUTED TO	CV716	27.	1.23	5.	5.	5.	.11		
2 COMBINED AT	HC17	74.	1.20	11.	11.	11.	.23		
ROUTED TO	CV726	73.	1.20	11.	11.	11.	.23		
HYDROGRAPH AT	SB474	31.	1.15	2.	2.	2.	.07		
ROUTED TO	CV674	30.	1.17	2.	2.	2.	.07		
ROUTED TO	CV675	30.	1.17	2.	2.	2.	.07		
ROUTED TO	CV673	30.	1.18	2.	2.	2.	.07		
ROUTED TO	CV685	29.	1.18	2.	2.	2.	.07		
ROUTED TO	CV684	29.	1.20	2.	2.	2.	.07		
ROUTED TO	CV676	29.	1.20	2.	2.	2.	.07		
HYDROGRAPH AT	SB649	26.	1.15	2.	2.	2.	.05		
ROUTED TO	CV677	26.	1.17	2.	2.	2.	.05		
2 COMBINED AT	HC216	50.	1.18	4.	4.	4.	.12		
ROUTED TO	CV678	49.	1.18	4.	4.	4.	.12		
ROUTED TO	CV679	49.	1.18	4.	4.	4.	.12		
HYDROGRAPH AT	SB648	9.	1.18	1.	1.	1.	.02		
ROUTED TO	CV680	9.	1.18	1.	1.	1.	.02		
2 COMBINED AT	HC215	58.	1.18	5.	5.	5.	.14		
ROUTED TO	CV681	57.	1.18	5.	5.	5.	.14		
ROUTED TO	CV682	56.	1.20	5.	5.	5.	.14		
HYDROGRAPH AT	SB788	9.	1.17	1.	1.	1.	.04		
3 COMBINED AT	HC214	136.	1.20	16.	16.	16.	.41		
ROUTED TO	DB81	52.	1.33	16.	16.	16.	.41	1.32	1.33
ROUTED TO	CV683	52.	1.33	16.	16.	16.	.41		
2 COMBINED AT	HC213	74.	1.67	29.	29.	29.	.61		
ROUTED TO	CV385	59.	2.28	24.	24.	24.	.61		
HYDROGRAPH AT	SB789	0.	2.03	0.	0.	0.	.09		
HYDROGRAPH AT	SB787	12.	1.25	1.	1.	1.	.04		
HYDROGRAPH AT	SB786	10.	1.22	1.	1.	1.	.07		
HYDROGRAPH AT	SB785	4.	1.43	1.	1.	1.	.05		
5 COMBINED AT	HC217	63.	2.22	27.	27.	27.	.86		
ROUTED TO	CV348	55.	2.67	23.	23.	23.	.86		
HYDROGRAPH AT	SB874	15.	1.23	2.	2.	2.	.06		
2 COMBINED AT	HC1	58.	2.67	25.	25.	25.	.92		
ROUTED TO	CV347	57.	2.72	24.	24.	24.	.92		
HYDROGRAPH AT	SB896	14.	1.25	2.	2.	2.	.06		
ROUTED TO	CV346	13.	1.25	2.	2.	2.	.06		
2 COMBINED AT	HC2	59.	2.70	26.	26.	26.	.98		

ROUTED TO	CV345	58.	2.82	25.	25.	25.	.98		
DIVERSION TO	DR36	58.	.00	25.	25.	25.	.98		
HYDROGRAPH AT	DH36	0.	.00	0.	0.	0.	.98		
HYDROGRAPH AT	SB169	10.	1.18	1.	1.	1.	.05		
2 COMBINED AT	HC218	10.	1.18	1.	1.	1.	1.03		
ROUTED TO	CV276	2.	2.77	1.	1.	1.	1.03		
HYDROGRAPH AT	SB804	3.	1.30	1.	1.	1.	.03		
ROUTED TO	CV272	3.	1.33	1.	1.	1.	.03		
2 COMBINED AT	HC3	3.	1.33	1.	1.	1.	1.06		
ROUTED TO	CV273	2.	3.35	1.	1.	1.	1.06		
HYDROGRAPH AT	SB805	5.	1.30	1.	1.	1.	.05		
ROUTED TO	CV206	3.	1.42	1.	1.	1.	.05		
ROUTED TO	DB35	3.	1.58	1.	1.	1.	.05	.57	1.58
ROUTED TO	CV207	3.	1.63	1.	1.	1.	.05		
HYDROGRAPH AT	SB806	7.	1.27	1.	1.	1.	.05		
2 COMBINED AT	HC219	7.	1.27	2.	2.	2.	.10		
ROUTED TO	CV696	7.	1.28	2.	2.	2.	.10		
ROUTED TO	CV342	7.	1.32	2.	2.	2.	.10		
ROUTED TO	CV695	7.	1.32	2.	2.	2.	.10		
2 COMBINED AT	HC4	7.	1.32	3.	3.	3.	1.16		
ROUTED TO	CV340	5.	2.25	2.	2.	2.	1.16		
HYDROGRAPH AT	SB776	8.	1.18	1.	1.	1.	.04		
ROUTED TO	CV339	8.	1.20	1.	1.	1.	.04		
HYDROGRAPH AT	SB875	6.	1.27	1.	1.	1.	.04		
2 COMBINED AT	HC220	13.	1.20	2.	2.	2.	.09		
ROUTED TO	CV338	13.	1.25	2.	2.	2.	.09		
ROUTED TO	CV337	13.	1.27	2.	2.	2.	.09		
HYDROGRAPH AT	SB775	12.	1.27	2.	2.	2.	.05		
2 COMBINED AT	HC221	25.	1.27	4.	4.	4.	.14		
ROUTED TO	CV336	25.	1.30	4.	4.	4.	.14		
HYDROGRAPH AT	SB773	9.	1.25	1.	1.	1.	.04		
3 COMBINED AT	HC222	33.	1.28	7.	7.	7.	1.34		
ROUTED TO	CV335	11.	2.28	6.	6.	6.	1.34		
HYDROGRAPH AT	SB772	12.	1.20	2.	2.	2.	.05		
ROUTED TO	CV239	12.	1.25	2.	2.	2.	.05		
HYDROGRAPH AT	SB763	23.	1.17	2.	2.	2.	.03		
HYDROGRAPH AT	SB771	49.	1.18	4.	4.	4.	.03		
2 COMBINED AT	HC325	71.	1.18	6.	6.	6.	.07		
ROUTED TO	CV246	71.	1.18	6.	6.	6.	.07		
ROUTED TO	CV237	63.	1.20	6.	6.	6.	.07		
ROUTED TO	CV238	54.	1.23	6.	6.	6.	.07		
2 COMBINED AT	HC224	66.	1.23	8.	8.	8.	.11		
ROUTED TO	CV240	63.	1.25	8.	8.	8.	.11		
HYDROGRAPH AT	SB770	3.	1.20	0.	0.	0.	.01		
3 COMBINED AT	HC223	66.	1.25	14.	14.	14.	1.46		

ROUTED TO	CV234	30.	1.28	13.	13.	13.	1.46		
HYDROGRAPH AT	SB769	5.	1.30	1.	1.	1.	.02		
2 COMBINED AT	HC226	34.	1.30	14.	14.	14.	1.48		
ROUTED TO	CV228	26.	2.10	13.	13.	13.	1.48		
HYDROGRAPH AT	SB60	26.	1.18	3.	3.	3.	.03		
ROUTED TO	CV135	25.	1.20	3.	3.	3.	.03		
ROUTED TO	CV136	25.	1.20	3.	3.	3.	.03		
HYDROGRAPH AT	SB762	29.	1.17	2.	2.	2.	.03		
2 COMBINED AT	HC227	49.	1.17	5.	5.	5.	.06		
ROUTED TO	CV138	48.	1.18	5.	5.	5.	.06		
ROUTED TO	CV139	48.	1.18	5.	5.	5.	.06		
HYDROGRAPH AT	SB764	36.	1.17	3.	3.	3.	.03		
2 COMBINED AT	HC228	84.	1.18	8.	8.	8.	.09		
ROUTED TO	CV140	83.	1.18	8.	8.	8.	.09		
ROUTED TO	CV141	81.	1.20	8.	8.	8.	.09		
HYDROGRAPH AT	SB766	42.	1.18	4.	4.	4.	.04		
2 COMBINED AT	HC229	122.	1.18	12.	12.	12.	.13		
ROUTED TO	CV129	121.	1.20	12.	12.	12.	.13		
ROUTED TO	CV130	120.	1.20	12.	12.	12.	.13		
ROUTED TO	DB10	19.	2.07	11.	11.	11.	.13	2.99	2.08
ROUTED TO	CV131	19.	2.08	11.	11.	11.	.13		
HYDROGRAPH AT	SB767	8.	1.27	1.	1.	1.	.02		
HYDROGRAPH AT	SB768	22.	1.17	2.	2.	2.	.03		
3 COMBINED AT	HC230	36.	1.18	14.	14.	14.	.17		
ROUTED TO	CV132	37.	1.22	14.	14.	14.	.17		
ROUTED TO	CV133	36.	1.22	14.	14.	14.	.17		
ROUTED TO	CV134	36.	1.23	14.	14.	14.	.17		
HYDROGRAPH AT	SB756	10.	1.17	1.	1.	1.	.03		
3 COMBINED AT	HC225	52.	2.05	27.	27.	27.	1.68		
ROUTED TO	CV227	51.	2.07	26.	26.	26.	1.68		
HYDROGRAPH AT	SB754	6.	1.32	1.	1.	1.	.04		
2 COMBINED AT	HC231	53.	2.07	27.	27.	27.	1.71		
ROUTED TO	CV232	52.	2.13	26.	26.	26.	1.71		
HYDROGRAPH AT	SB548	7.	1.27	1.	1.	1.	.05		
2 COMBINED AT	HC232	53.	2.12	27.	27.	27.	1.76		
ROUTED TO	CV384	50.	2.38	24.	24.	24.	1.76		
HYDROGRAPH AT	TP010	17.	1.35	3.	3.	3.	.08		
ROUTED TO	RTP011	17.	1.38	3.	3.	3.	.08		
HYDROGRAPH AT	TP020	12.	1.32	2.	2.	2.	.04		
ROUTED TO	RTP021	12.	1.33	2.	2.	2.	.04		
HYDROGRAPH AT	TP030	21.	1.23	2.	2.	2.	.05		
3 COMBINED AT	CTP031	41.	1.27	7.	7.	7.	.16		
ROUTED TO	RTP033	41.	1.30	7.	7.	7.	.16		
HYDROGRAPH AT	TP100	20.	1.27	3.	3.	3.	.06		
ROUTED TO	RTP101	20.	1.30	3.	3.	3.	.06		

HYDROGRAPH AT	TP110	21.	1.27	3.	3.	3.	.06		
HYDROGRAPH AT	TP120	8.	1.48	2.	2.	2.	.04		
3 COMBINED AT	TP121	46.	1.28	7.	7.	7.	.16		
ROUTED TO	ALTHDB	11.	2.13	7.	7.	7.	.16	98.94	2.15
ROUTED TO	RTP123	11.	2.22	6.	6.	6.	.16		
HYDROGRAPH AT	TP130	15.	1.28	2.	2.	2.	.06		
3 COMBINED AT	CTP131	61.	1.30	16.	16.	16.	.39		
ROUTED TO	RTP133	61.	1.30	16.	16.	16.	.39		
HYDROGRAPH AT	TP200	35.	1.17	3.	3.	3.	.09		
ROUTED TO	RTP201	35.	1.18	3.	3.	3.	.09		
HYDROGRAPH AT	TP210	23.	1.18	2.	2.	2.	.04		
3 COMBINED AT	CTP211	84.	1.28	21.	21.	21.	.52		
HYDROGRAPH AT	TP300	28.	1.22	3.	3.	3.	.05		
ROUTED TO	RTP301	27.	1.22	3.	3.	3.	.05		
HYDROGRAPH AT	TP310	2.	1.22	0.	0.	0.	.03		
2 COMBINED AT	CTP311	29.	1.22	3.	3.	3.	.08		
ROUTED TO	RTP312	29.	1.23	3.	3.	3.	.08		
HYDROGRAPH AT	TP330	26.	1.23	3.	3.	3.	.06		
ROUTED TO	RTP331	25.	1.25	3.	3.	3.	.06		
HYDROGRAPH AT	TP320	2.	1.27	0.	0.	0.	.04		
3 COMBINED AT	CTP321	55.	1.25	7.	7.	7.	.18		
ROUTED TO	RTP323	47.	1.28	7.	7.	7.	.18		
ROUTED TO	RTP324	47.	1.30	7.	7.	7.	.18		
2 COMBINED AT	CTP325	131.	1.28	28.	28.	28.	.69		
ROUTED TO	STMDDET	28.	2.52	20.	20.	20.	.69	61.83	2.58
HYDROGRAPH AT	TP450	8.	1.40	2.	2.	2.	.04		
2 COMBINED AT	CTP451	35.	1.42	21.	21.	21.	.74		
ROUTED TO	RTP452	35.	1.42	21.	21.	21.	.74		
HYDROGRAPH AT	TP400	37.	1.27	5.	5.	5.	.13		
ROUTED TO	RTP401	37.	1.28	5.	5.	5.	.13		
HYDROGRAPH AT	TP410	4.	1.48	1.	1.	1.	.04		
2 COMBINED AT	CTP411	40.	1.28	6.	6.	6.	.17		
ROUTED TO	RTP412	40.	1.33	6.	6.	6.	.17		
HYDROGRAPH AT	TP430	29.	1.28	4.	4.	4.	.09		
HYDROGRAPH AT	TP420	35.	1.18	3.	3.	3.	.06		
3 COMBINED AT	CTP421	79.	1.30	13.	13.	13.	.32		
ROUTED TO	RTP423	79.	1.32	13.	13.	13.	.32		
ROUTED TO	AELDET	29.	1.65	13.	13.	13.	.32	47.69	1.65
ROUTED TO	RTP425	29.	1.68	13.	13.	13.	.32		
HYDROGRAPH AT	TP440	50.	1.18	4.	4.	4.	.09		
2 COMBINED AT	CTP441	73.	1.20	17.	17.	17.	.41		
ROUTED TO	RTP442	69.	1.23	17.	17.	17.	.41		
HYDROGRAPH AT	TP460	9.	1.42	2.	2.	2.	.05		
3 COMBINED AT	CTP461	104.	1.23	40.	40.	40.	1.20		
ROUTED TO	RTP463	104.	1.25	40.	40.	40.	1.20		

HYDROGRAPH AT	TP470	27.	1.28	4.	4.	4.	.09		
2 COMBINED AT	CTP471	129.	1.25	44.	44.	44.	1.29		
ROUTED TO	TFDET	31.	3.85	15.	15.	15.	1.29	100.13	3.85
ROUTED TO	CV150	31.	3.88	15.	15.	15.	1.29		
HYDROGRAPH AT	SB744	12.	1.17	1.	1.	1.	.02		
HYDROGRAPH AT	SB745	7.	1.17	1.	1.	1.	.02		
3 COMBINED AT	HC235	33.	1.17	17.	17.	17.	1.33		
ROUTED TO	CV151	32.	1.18	17.	17.	17.	1.33		
ROUTED TO	CV152	31.	3.90	16.	16.	16.	1.33		
HYDROGRAPH AT	SB749	17.	1.17	1.	1.	1.	.03		
ROUTED TO	CV264	17.	1.20	1.	1.	1.	.03		
ROUTED TO	CV156	17.	1.20	1.	1.	1.	.03		
HYDROGRAPH AT	SB750	11.	1.17	1.	1.	1.	.02		
2 COMBINED AT	HC240	25.	1.18	2.	2.	2.	.05		
ROUTED TO	CV158	25.	1.20	2.	2.	2.	.05		
ROUTED TO	CV160	25.	1.20	2.	2.	2.	.05		
HYDROGRAPH AT	SB748	15.	1.17	1.	1.	1.	.04		
HYDROGRAPH AT	SB747	30.	1.18	2.	2.	2.	.06		
3 COMBINED AT	HC237	66.	1.18	6.	6.	6.	.14		
ROUTED TO	DB3	18.	1.35	6.	6.	6.	.14	2.89	1.35
ROUTED TO	CV162	18.	1.38	6.	6.	6.	.14		
HYDROGRAPH AT	SB752	20.	1.17	2.	2.	2.	.03		
2 COMBINED AT	HC238	27.	1.18	7.	7.	7.	.17		
ROUTED TO	DB4	22.	1.35	7.	7.	7.	.17	5.71	1.35
ROUTED TO	CV154	22.	1.38	7.	7.	7.	.17		
HYDROGRAPH AT	SB746	22.	1.17	2.	2.	2.	.03		
3 COMBINED AT	HC236	51.	1.25	26.	26.	26.	1.53		
ROUTED TO	DB5	46.	1.52	24.	24.	24.	1.53	5.36	1.52
HYDROGRAPH AT	SB753	5.	1.25	1.	1.	1.	.04		
2 COMBINED AT	HC239	48.	1.40	25.	25.	25.	1.57		
ROUTED TO	CV155	48.	1.48	24.	24.	24.	1.57		
HYDROGRAPH AT	SB549	3.	1.32	1.	1.	1.	.03		
3 COMBINED AT	HC233	91.	2.13	49.	49.	49.	3.35		
ROUTED TO	CV383	86.	2.40	44.	44.	44.	3.35		
HYDROGRAPH AT	SB741	3.	1.52	1.	1.	1.	.04		
2 COMBINED AT	HC243	88.	2.38	45.	45.	45.	3.39		
ROUTED TO	CV723	85.	2.58	41.	41.	41.	3.39		
HYDROGRAPH AT	SB735	12.	1.22	1.	1.	1.	.03		
ROUTED TO	CV382	12.	1.23	1.	1.	1.	.03		
HYDROGRAPH AT	SB739	15.	1.28	2.	2.	2.	.04		
2 COMBINED AT	HC244	26.	1.25	3.	3.	3.	.07		
ROUTED TO	CV178	25.	1.28	3.	3.	3.	.07		
HYDROGRAPH AT	SB738	16.	1.22	2.	2.	2.	.05		
ROUTED TO	CV179	16.	1.23	2.	2.	2.	.05		
2 COMBINED AT	HC245	39.	1.27	5.	5.	5.	.11		

ROUTED TO	CV176	39.	1.28	5.	5.	5.	.11		
HYDROGRAPH AT	SB740	10.	1.27	1.	1.	1.	.03		
ROUTED TO	CV164	10.	1.28	1.	1.	1.	.03		
ROUTED TO	CV165	10.	1.32	1.	1.	1.	.03		
ROUTED TO	CV166	10.	1.32	1.	1.	1.	.03		
ROUTED TO	CV167	10.	1.33	1.	1.	1.	.03		
2 COMBINED AT	HC246	47.	1.30	6.	6.	6.	.14		
ROUTED TO	DB6	19.	1.52	6.	6.	6.	.14	2.70	1.52
ROUTED TO	CV168	18.	1.55	6.	6.	6.	.14		
2 COMBINED AT	HC242	94.	2.42	47.	47.	47.	3.53		
ROUTED TO	CV724	93.	2.52	45.	45.	45.	3.53		
HYDROGRAPH AT	SB657	5.	1.38	1.	1.	1.	.02		
2 COMBINED AT	HC241	94.	2.50	46.	46.	46.	3.55		
ROUTED TO	CV378	88.	2.80	39.	39.	39.	3.55		
HYDROGRAPH AT	SB736	14.	1.18	1.	1.	1.	.03		
ROUTED TO	CV182	11.	1.22	1.	1.	1.	.03		
HYDROGRAPH AT	SB737	17.	1.17	1.	1.	1.	.04		
2 COMBINED AT	HC249	26.	1.18	2.	2.	2.	.06		
ROUTED TO	CV184	24.	1.22	2.	2.	2.	.06		
ROUTED TO	CV186	24.	1.23	2.	2.	2.	.06		
HYDROGRAPH AT	SB655	20.	1.20	2.	2.	2.	.04		
ROUTED TO	CV189	11.	1.23	2.	2.	2.	.04		
2 COMBINED AT	HC248	35.	1.23	4.	4.	4.	.10		
ROUTED TO	DB7	13.	1.45	4.	4.	4.	.10	2.32	1.45
ROUTED TO	CV380	12.	1.50	4.	4.	4.	.10		
2 COMBINED AT	HC247	93.	2.67	44.	44.	44.	3.66		
ROUTED TO	CV379	91.	2.80	42.	42.	42.	3.66		
HYDROGRAPH AT	SB255	10.	1.18	1.	1.	1.	.01		
2 COMBINED AT	HC250	92.	2.80	42.	42.	42.	3.67		
ROUTED TO	CV499	88.	3.12	36.	36.	36.	3.67		
DIVERSION TO	DR37	88.	.00	36.	36.	36.	3.67		
HYDROGRAPH AT	DH37	0.	.00	0.	0.	0.	3.67		
HYDROGRAPH AT	EJS010	21.	1.78	7.	7.	7.	.11		
ROUTED TO	RJS011	21.	1.80	7.	7.	7.	.11		
HYDROGRAPH AT	EJS020	7.	1.50	2.	2.	2.	.05		
2 COMBINED AT	CJS021	26.	1.77	9.	9.	9.	.15		
ROUTED TO	RJS022	26.	1.82	9.	9.	9.	.15		
ROUTED TO	RJS023	23.	2.03	8.	8.	8.	.15		
HYDROGRAPH AT	EJS030	7.	1.47	2.	2.	2.	.04		
HYDROGRAPH AT	EJS040	21.	1.72	7.	7.	7.	.05		
3 COMBINED AT	CJS102	44.	1.88	17.	17.	17.	.24		
2 COMBINED AT	HC338	44.	1.88	17.	17.	17.	3.91		
ROUTED TO	CV863	35.	2.43	14.	14.	14.	3.91		
HYDROGRAPH AT	CP010	14.	1.70	4.	4.	4.	.07		
ROUTED TO	RCP011	14.	1.72	4.	4.	4.	.07		

HYDROGRAPH AT	CP020	3.	1.47	1.	1.	1.	.04
HYDROGRAPH AT	CP030	5.	2.15	2.	2.	2.	.10
3 COMBINED AT	CCP031	18.	1.75	7.	7.	7.	.22
ROUTED TO	RCP032	18.	1.77	7.	7.	7.	.22
ROUTED TO	CRSTD	18.	1.77	7.	7.	7.	.22
ROUTED TO	RCP034	18.	1.78	7.	7.	7.	.22
HYDROGRAPH AT	EJS050	18.	1.68	4.	4.	4.	.09
2 COMBINED AT	CCP102	36.	1.72	11.	11.	11.	.31
2 COMBINED AT	HC339	54.	2.27	24.	24.	24.	4.22
ROUTED TO	CV738	52.	2.38	23.	23.	23.	4.22
HYDROGRAPH AT	EJS060	8.	1.38	2.	2.	2.	.07
2 COMBINED AT	HC340	54.	2.35	25.	25.	25.	4.29
ROUTED TO	CV739	50.	2.70	21.	21.	21.	4.29
2 COMBINED AT	HC341	67.	1.18	34.	34.	34.	5.82
HYDROGRAPH AT	EJS090	0.	1.93	0.	0.	0.	.04
ROUTED TO	CV815	0.	3.92	0.	0.	0.	.04
DIVERSION TO	DR4	0.	.00	0.	0.	0.	.04
HYDROGRAPH AT	DH4	0.	.00	0.	0.	0.	.04
ROUTED TO	CV816	0.	.00	0.	0.	0.	.04
HYDROGRAPH AT	EJC003	27.	1.60	7.	7.	7.	.09
ROUTED TO	RJC004	23.	1.80	7.	7.	7.	.09
HYDROGRAPH AT	EJC005	15.	1.58	4.	4.	4.	.05
2 COMBINED AT	CJC006	35.	1.75	10.	10.	10.	.14
2 COMBINED AT	HC343	35.	1.75	10.	10.	10.	.18
ROUTED TO	CV742	26.	2.10	9.	9.	9.	.18
HYDROGRAPH AT	EJC010	6.	1.83	2.	2.	2.	.02
ROUTED TO	REJ011	6.	1.90	2.	2.	2.	.02
ROUTED TO	REJ012	5.	2.67	2.	2.	2.	.02
HYDROGRAPH AT	EJC020	13.	1.90	5.	5.	5.	.06
2 COMBINED AT	CEJ021	13.	2.32	6.	6.	6.	.08
2 COMBINED AT	HC344	38.	2.12	15.	15.	15.	.26
2 COMBINED AT	CV743	96.	2.40	49.	49.	49.	6.08
HYDROGRAPH AT	EJC040	30.	1.58	8.	8.	8.	.05
ROUTED TO	CV745	22.	1.82	8.	8.	8.	.05
HYDROGRAPH AT	EJC050	23.	1.42	5.	5.	5.	.05
2 COMBINED AT	HC347	33.	1.67	13.	13.	13.	.10
ROUTED TO	CV820	29.	1.97	12.	12.	12.	.10
DIVERSION TO	DR6	29.	.00	12.	12.	12.	.10
HYDROGRAPH AT	DH6	0.	.00	0.	0.	0.	.10
ROUTED TO	CV821	0.	.00	0.	0.	0.	.10
HYDROGRAPH AT	EJC055	31.	1.70	9.	9.	9.	.11
ROUTED TO	RJC056	29.	1.80	9.	9.	9.	.11
2 COMBINED AT	HC348	29.	1.80	9.	9.	9.	.22
ROUTED TO	CV747	25.	1.97	9.	9.	9.	.22
2 COMBINED AT	HC349	116.	2.18	58.	58.	58.	6.29

76.55

1.77

HYDROGRAPH AT	EJC075	10.	1.55	2.	2.	2.	.03
ROUTED TO	CV750	8.	1.68	2.	2.	2.	.03
HYDROGRAPH AT	EJC080	26.	1.60	7.	7.	7.	.09
2 COMBINED AT	HC352	33.	1.62	9.	9.	9.	.12
ROUTED TO	CV751	20.	2.17	8.	8.	8.	.12
HYDROGRAPH AT	EJC083	14.	1.78	4.	4.	4.	.06
ROUTED TO	RJC084	11.	1.90	4.	4.	4.	.06
HYDROGRAPH AT	EJC086	36.	1.77	11.	11.	11.	.12
2 COMBINED AT	CJC087	47.	1.80	15.	15.	15.	.19
2 COMBINED AT	HC353	61.	1.88	23.	23.	23.	.31
ROUTED TO	CV752	55.	2.05	22.	22.	22.	.31
HYDROGRAPH AT	EJC090	11.	1.73	4.	4.	4.	.04
2 COMBINED AT	HC354	64.	2.00	26.	26.	26.	.35
ROUTED TO	CV753	60.	2.15	25.	25.	25.	.35
2 COMBINED AT	HC355	176.	2.18	82.	82.	82.	6.64
HYDROGRAPH AT	SBC43	59.	1.25	9.	9.	9.	.25
ROUTED TO	CV824	19.	1.97	7.	7.	7.	.25
HYDROGRAPH AT	SBC44	22.	1.23	3.	3.	3.	.07
2 COMBINED AT	HC360	26.	1.97	10.	10.	10.	.32
ROUTED TO	CV864	20.	2.58	7.	7.	7.	.32
HYDROGRAPH AT	SB881	25.	1.22	3.	3.	3.	.10
2 COMBINED AT	HC361	25.	1.22	11.	11.	11.	.42
ROUTED TO	CV808	23.	2.58	11.	11.	11.	.42
HYDROGRAPH AT	SB880	40.	1.22	5.	5.	5.	.11
2 COMBINED AT	HC404	57.	1.22	16.	16.	16.	.53
ROUTED TO	CV872	25.	2.90	13.	13.	13.	.53
HYDROGRAPH AT	SB894	9.	1.27	2.	2.	2.	.08
ROUTED TO	CV860	4.	1.82	2.	2.	2.	.08
HYDROGRAPH AT	SB895	6.	1.18	1.	1.	1.	.04
2 COMBINED AT	HC408	6.	1.18	3.	3.	3.	.12
ROUTED TO	CV813	4.	3.42	1.	1.	1.	.12
HYDROGRAPH AT	SBC31	11.	1.33	2.	2.	2.	.15
HYDROGRAPH AT	SBC23	42.	1.25	6.	6.	6.	.13
3 COMBINED AT	HC362	50.	1.27	9.	9.	9.	.39
ROUTED TO	CV761	20.	1.65	8.	8.	8.	.39
HYDROGRAPH AT	SBC20	11.	1.25	2.	2.	2.	.10
2 COMBINED AT	HC363	24.	1.62	9.	9.	9.	.49
ROUTED TO	CV762	21.	2.08	8.	8.	8.	.49
HYDROGRAPH AT	SBC21	13.	1.30	2.	2.	2.	.18
2 COMBINED AT	HC364	25.	2.05	10.	10.	10.	.67
ROUTED TO	CV763	22.	2.25	9.	9.	9.	.67
HYDROGRAPH AT	SBC30	9.	1.30	2.	2.	2.	.16
2 COMBINED AT	HC365	25.	2.20	11.	11.	11.	.83
ROUTED TO	CV832	19.	3.13	6.	6.	6.	.83
HYDROGRAPH AT	SB573	116.	1.25	19.	19.	19.	.23

ROUTED TO	CV535	41.	1.65	16.	16.	16.	.23
HYDROGRAPH AT	SB572	50.	1.22	7.	7.	7.	.15
HYDROGRAPH AT	SB571	69.	1.25	11.	11.	11.	.14
3 COMBINED AT	HC251	133.	1.25	34.	34.	34.	.52
ROUTED TO	CV534	75.	1.98	31.	31.	31.	.52
HYDROGRAPH AT	SB376	35.	1.20	5.	5.	5.	.06
HYDROGRAPH AT	SB374	11.	1.35	2.	2.	2.	.12
2 COMBINED AT	HC326	42.	1.20	8.	8.	8.	.19
ROUTED TO	CV474	38.	1.25	8.	8.	8.	.19
HYDROGRAPH AT	SB381	10.	1.18	1.	1.	1.	.02
HYDROGRAPH AT	SB382	28.	1.20	4.	4.	4.	.05
4 COMBINED AT	HC252	116.	1.27	44.	44.	44.	.78
ROUTED TO	CV475	89.	2.08	38.	38.	38.	.78
HYDROGRAPH AT	SB574	3.	1.20	0.	0.	0.	.03
2 COMBINED AT	HC253	90.	2.08	38.	38.	38.	.81
ROUTED TO	CV471	85.	2.17	37.	37.	37.	.81
HYDROGRAPH AT	SB575	4.	1.20	1.	1.	1.	.03
2 COMBINED AT	HC254	86.	2.17	37.	37.	37.	.84
ROUTED TO	CV470	82.	2.30	36.	36.	36.	.84
HYDROGRAPH AT	SB576	7.	1.28	1.	1.	1.	.03
2 COMBINED AT	HC255	84.	2.27	37.	37.	37.	.87
ROUTED TO	CV472	80.	2.43	35.	35.	35.	.87
HYDROGRAPH AT	SB577	4.	1.23	1.	1.	1.	.04
2 COMBINED AT	HC256	81.	2.45	36.	36.	36.	.92
ROUTED TO	CV478	79.	2.52	35.	35.	35.	.92
HYDROGRAPH AT	SB579	11.	1.17	1.	1.	1.	.02
2 COMBINED AT	HC257	80.	2.50	36.	36.	36.	.94
ROUTED TO	CV479	78.	2.60	34.	34.	34.	.94
HYDROGRAPH AT	SB578	16.	1.25	3.	3.	3.	.03
2 COMBINED AT	HC258	81.	2.58	37.	37.	37.	.97
ROUTED TO	CV480	78.	2.72	35.	35.	35.	.97
HYDROGRAPH AT	SB609	14.	1.25	2.	2.	2.	.03
2 COMBINED AT	HC259	79.	2.70	37.	37.	37.	1.00
ROUTED TO	CV865	77.	2.85	34.	34.	34.	1.00
DIVERSION TO	DR41	77.	.00	34.	34.	34.	1.00
HYDROGRAPH AT	DH41	0.	.00	0.	0.	0.	1.00
ROUTED TO	CV866	0.	.00	0.	0.	0.	1.00
HYDROGRAPH AT	SB358	4.	1.27	1.	1.	1.	.02
2 COMBINED AT	HC260	4.	1.27	1.	1.	1.	1.02
ROUTED TO	CV482	2.	2.37	1.	1.	1.	1.02
HYDROGRAPH AT	SB595	10.	1.18	1.	1.	1.	.01
HYDROGRAPH AT	SB262	12.	1.18	2.	2.	2.	.02
2 COMBINED AT	HC274	22.	1.18	3.	3.	3.	.03
ROUTED TO	CV486	22.	1.20	3.	3.	3.	.03
ROUTED TO	CV488	21.	1.22	3.	3.	3.	.03

ROUTED TO	CV487	20.	1.23	3.	3.	3.	.03		
HYDROGRAPH AT	SB602	27.	1.20	4.	4.	4.	.05		
3 COMBINED AT	HC262	46.	1.22	7.	7.	7.	1.10		
ROUTED TO	CV485	15.	1.70	6.	6.	6.	1.10		
HYDROGRAPH AT	SB877	12.	1.27	2.	2.	2.	.03		
2 COMBINED AT	HC263	19.	1.62	8.	8.	8.	1.12		
ROUTED TO	CV489	18.	2.07	7.	7.	7.	1.12		
HYDROGRAPH AT	SB206	12.	1.25	2.	2.	2.	.04		
2 COMBINED AT	HC264	21.	2.03	9.	9.	9.	1.16		
ROUTED TO	CV490	20.	2.13	9.	9.	9.	1.16		
HYDROGRAPH AT	SB593	21.	1.17	2.	2.	2.	.05		
ROUTED TO	CV215	21.	1.17	2.	2.	2.	.05		
ROUTED TO	DB38	5.	1.58	2.	2.	2.	.05	1.83	1.60
2 COMBINED AT	HC265	25.	2.10	11.	11.	11.	1.22		
ROUTED TO	CV491	20.	2.92	7.	7.	7.	1.22		
3 COMBINED AT	HC266	64.	3.02	26.	26.	26.	2.58		
HYDROGRAPH AT	JSL010	3.	1.43	1.	1.	1.	.08		
HYDROGRAPH AT	JSL020	14.	1.67	5.	5.	5.	.11		
ROUTED TO	RJS021	14.	1.67	5.	5.	5.	.11		
2 COMBINED AT	CCP103	16.	1.67	5.	5.	5.	.18		
ROUTED TO	CV727	12.	2.07	5.	5.	5.	.18		
HYDROGRAPH AT	JSL030	8.	1.55	2.	2.	2.	.08		
2 COMBINED AT	HC328	17.	1.92	7.	7.	7.	.26		
ROUTED TO	CV728	16.	2.08	7.	7.	7.	.26		
HYDROGRAPH AT	JSL040	6.	1.58	2.	2.	2.	.07		
2 COMBINED AT	HC329	20.	1.93	9.	9.	9.	.33		
ROUTED TO	CV729	19.	2.22	8.	8.	8.	.33		
2 COMBINED AT	HC330	77.	2.92	34.	34.	34.	2.91		
HYDROGRAPH AT	JSL050	10.	1.62	2.	2.	2.	.05		
ROUTED TO	CV730	7.	1.87	2.	2.	2.	.05		
HYDROGRAPH AT	JSL060	8.	1.53	2.	2.	2.	.05		
2 COMBINED AT	HC331	13.	1.73	4.	4.	4.	.11		
ROUTED TO	CV867	10.	2.07	4.	4.	4.	.11		
DIVERSION TO	DR3	10.	.00	4.	4.	4.	.11		
HYDROGRAPH AT	DH3	0.	.00	0.	0.	0.	.11		
ROUTED TO	CV819	0.	.00	0.	0.	0.	.11		
HYDROGRAPH AT	JSL200	0.	1.85	0.	0.	0.	.07		
ROUTED TO	RJS201	0.	2.52	0.	0.	0.	.07		
2 COMBINED AT	HC332	0.	2.52	0.	0.	0.	.18		
ROUTED TO	CV757	0.	3.20	0.	0.	0.	.18		
HYDROGRAPH AT	JSL500	0.	2.00	0.	0.	0.	.06		
ROUTED TO	RJS501	0.	2.03	0.	0.	0.	.06		
HYDROGRAPH AT	JSL510	20.	1.93	6.	6.	6.	.07		
ROUTED TO	COMMER	7.	1.97	4.	4.	4.	.07	1.91	2.95
2 COMBINED AT	CCP103	7.	2.03	4.	4.	4.	.13		

2 COMBINED AT	HC333	7.	3.13	4.	4.	4.	.31		
ROUTED TO	CV732	7.	3.73	3.	3.	3.	.31		
HYDROGRAPH AT	JSL520	9.	1.95	3.	3.	3.	.05		
ROUTED TO	COMMER	4.	2.57	2.	2.	2.	.05	1.06	2.97
2 COMBINED AT	HC334	11.	3.45	5.	5.	5.	.36		
ROUTED TO	CV733	11.	3.98	2.	2.	2.	.36		
2 COMBINED AT	HC335	81.	3.08	37.	37.	37.	3.27		
HYDROGRAPH AT	JSL550	28.	1.92	9.	9.	9.	.10		
ROUTED TO	COMMER	20.	2.00	9.	9.	9.	.10	1.07	2.32
ROUTED TO	CV736	20.	2.70	8.	8.	8.	.10		
2 COMBINED AT	HC359	98.	2.98	45.	45.	45.	3.37		
HYDROGRAPH AT	JSL560	46.	1.63	12.	12.	12.	.12		
ROUTED TO	COMMER	29.	1.63	12.	12.	12.	.12	1.17	1.95
ROUTED TO	CV771	29.	2.40	12.	12.	12.	.12		
HYDROGRAPH AT	SBC32	19.	1.20	2.	2.	2.	.09		
2 COMBINED AT	HC372	32.	2.02	14.	14.	14.	.21		
ROUTED TO	CV772	29.	2.48	12.	12.	12.	.21		
HYDROGRAPH AT	SBC33	24.	1.17	3.	3.	3.	.06		
2 COMBINED AT	HC373	32.	2.50	15.	15.	15.	.28		
ROUTED TO	CV773	29.	2.78	12.	12.	12.	.28		
2 COMBINED AT	HC374	126.	2.93	57.	57.	57.	3.65		
HYDROGRAPH AT	SBC8	38.	1.18	4.	4.	4.	.15		
ROUTED TO	CV774	9.	1.75	3.	3.	3.	.15		
HYDROGRAPH AT	SBC47	28.	1.28	4.	4.	4.	.10		
2 COMBINED AT	HC375	29.	1.28	7.	7.	7.	.24		
ROUTED TO	CV775	18.	1.60	7.	7.	7.	.24		
HYDROGRAPH AT	SBC46	31.	1.27	4.	4.	4.	.13		
2 COMBINED AT	HC376	46.	1.28	11.	11.	11.	.38		
ROUTED TO	CV776	25.	1.73	10.	10.	10.	.38		
HYDROGRAPH AT	SB879	25.	1.27	3.	3.	3.	.10		
2 COMBINED AT	HC377	36.	1.30	14.	14.	14.	.47		
ROUTED TO	CV777	30.	2.08	12.	12.	12.	.47		
HYDROGRAPH AT	SB878	18.	1.23	2.	2.	2.	.09		
2 COMBINED AT	HC403	34.	2.05	14.	14.	14.	.56		
ROUTED TO	CV826	32.	2.12	14.	14.	14.	.56		
DIVERSION TO	DR42	32.	.00	14.	14.	14.	.56		
HYDROGRAPH AT	DH42	0.	.00	0.	0.	0.	.56		
ROUTED TO	CV827	0.	.00	0.	0.	0.	.56		
HYDROGRAPH AT	SBC1	33.	1.18	4.	4.	4.	.10		
2 COMBINED AT	HC378	33.	1.18	4.	4.	4.	.66		
ROUTED TO	CV778	9.	1.75	3.	3.	3.	.66		
HYDROGRAPH AT	SBC10	46.	1.18	5.	5.	5.	.12		
ROUTED TO	CV803	16.	1.18	4.	4.	4.	.12		
2 COMBINED AT	HC402	19.	1.58	7.	7.	7.	.78		
ROUTED TO	CV802	17.	2.05	7.	7.	7.	.78		

HYDROGRAPH AT	SBC2	29.	1.25	4.	4.	4.	.10
2 COMBINED AT	HC379	29.	1.25	10.	10.	10.	.89
ROUTED TO	CV830	18.	2.82	7.	7.	7.	.89
DIVERSION TO	DR11	18.	.00	7.	7.	7.	.89
HYDROGRAPH AT	DH11	0.	.00	0.	0.	0.	.89
ROUTED TO	CV831	0.	.00	0.	0.	0.	.89
HYDROGRAPH AT	SBC11	25.	1.25	4.	4.	4.	.09
2 COMBINED AT	HC380	25.	1.25	4.	4.	4.	.98
ROUTED TO	CV809	8.	1.95	3.	3.	3.	.98
HYDROGRAPH AT	SB882	32.	1.43	7.	7.	7.	.20
2 COMBINED AT	HC405	33.	1.47	10.	10.	10.	1.17
ROUTED TO	CV810	21.	2.08	8.	8.	8.	1.17
HYDROGRAPH AT	SB883	53.	1.23	7.	7.	7.	.17
2 COMBINED AT	HC381	53.	1.23	15.	15.	15.	1.35
ROUTED TO	CV781	31.	2.12	14.	14.	14.	1.35
HYDROGRAPH AT	SB885	31.	1.22	4.	4.	4.	.11
HYDROGRAPH AT	SB884	16.	1.25	2.	2.	2.	.09
3 COMBINED AT	HC382	47.	1.23	19.	19.	19.	1.54
ROUTED TO	CV782	39.	2.07	18.	18.	18.	1.54
HYDROGRAPH AT	SBC12	14.	1.33	2.	2.	2.	.10
2 COMBINED AT	HC383	44.	2.05	21.	21.	21.	1.65
ROUTED TO	CV783	41.	2.12	19.	19.	19.	1.65
HYDROGRAPH AT	SBC3	12.	1.37	2.	2.	2.	.14
2 COMBINED AT	HC384	44.	2.10	21.	21.	21.	1.79
ROUTED TO	CV784	38.	2.70	16.	16.	16.	1.79
HYDROGRAPH AT	SBC4	10.	1.28	1.	1.	1.	.12
2 COMBINED AT	HC385	39.	2.67	17.	17.	17.	1.91
ROUTED TO	CV834	37.	2.87	16.	16.	16.	1.91
DIVERSION TO	DR15	37.	.00	16.	16.	16.	1.91
HYDROGRAPH AT	DH15	0.	.00	0.	0.	0.	1.91
ROUTED TO	CV835	0.	.00	0.	0.	0.	1.91
HYDROGRAPH AT	SBC42	32.	1.25	4.	4.	4.	.15
HYDROGRAPH AT	SBC41	22.	1.27	3.	3.	3.	.14
3 COMBINED AT	HC386	52.	1.25	7.	7.	7.	2.20
ROUTED TO	CV836	16.	1.85	6.	6.	6.	2.20
HYDROGRAPH AT	SBC40	20.	1.25	3.	3.	3.	.09
2 COMBINED AT	HC387	21.	1.73	9.	9.	9.	2.29
ROUTED TO	CV787	19.	2.17	7.	7.	7.	2.29
HYDROGRAPH AT	SBC5	27.	1.20	3.	3.	3.	.08
2 COMBINED AT	HC388	27.	1.20	10.	10.	10.	2.37
ROUTED TO	CV838	20.	2.33	9.	9.	9.	2.37
HYDROGRAPH AT	SBC6	20.	1.25	3.	3.	3.	.10
2 COMBINED AT	HC389	24.	2.12	12.	12.	12.	2.47
ROUTED TO	CV789	21.	2.68	9.	9.	9.	2.47
HYDROGRAPH AT	SBC39	23.	1.22	3.	3.	3.	.09

HYDROGRAPH AT	SBC7	18.	1.17	2.	2.	2.	.09
3 COMBINED AT	HC390	39.	1.20	14.	14.	14.	2.65
ROUTED TO	CV840	22.	3.12	10.	10.	10.	2.65
ROUTED TO	CV841	22.	3.13	10.	10.	10.	2.65
HYDROGRAPH AT	SBC38	11.	1.17	1.	1.	1.	.06
2 COMBINED AT	HC391	23.	3.05	11.	11.	11.	2.71
ROUTED TO	CV842	22.	3.18	10.	10.	10.	2.71
DIVERSION TO	DR19	22.	.00	10.	10.	10.	2.71
HYDROGRAPH AT	DH19	0.	.00	0.	0.	0.	2.71
ROUTED TO	CV843	0.	.00	0.	0.	0.	2.71
HYDROGRAPH AT	SBC34	27.	1.23	4.	4.	4.	.14
2 COMBINED AT	HC392	27.	1.23	4.	4.	4.	2.85
ROUTED TO	CV844	23.	1.23	3.	3.	3.	2.85
ROUTED TO	CV845	12.	1.30	3.	3.	3.	2.85
HYDROGRAPH AT	SBC35	19.	1.27	3.	3.	3.	.08
2 COMBINED AT	HC393	31.	1.27	6.	6.	6.	2.93
ROUTED TO	CV846	14.	1.78	5.	5.	5.	2.93
DIVERSION TO	DR21	14.	.00	5.	5.	5.	2.93
HYDROGRAPH AT	DH21	0.	.00	0.	0.	0.	2.93
ROUTED TO	CV847	0.	.00	0.	0.	0.	2.93
HYDROGRAPH AT	SBC36	17.	1.17	2.	2.	2.	.04
2 COMBINED AT	HC394	17.	1.17	2.	2.	2.	2.97
ROUTED TO	CV794	3.	2.73	1.	1.	1.	2.97
HYDROGRAPH AT	SB892	15.	1.17	2.	2.	2.	.02
2 COMBINED AT	HC395	15.	1.17	3.	3.	3.	2.99
ROUTED TO	CV795	4.	3.30	2.	2.	2.	2.99
HYDROGRAPH AT	SB889	19.	1.17	2.	2.	2.	.04
2 COMBINED AT	HC396	19.	1.17	4.	4.	4.	3.03
ROUTED TO	CV811	13.	1.18	4.	4.	4.	3.03
HYDROGRAPH AT	SB891	22.	1.18	2.	2.	2.	.05
2 COMBINED AT	HC406	35.	1.18	6.	6.	6.	3.08
ROUTED TO	CV812	10.	2.03	5.	5.	5.	3.08
HYDROGRAPH AT	SBC37	30.	1.22	3.	3.	3.	.09
2 COMBINED AT	HC397	35.	1.22	9.	9.	9.	3.17
ROUTED TO	CV848	20.	1.23	8.	8.	8.	3.17
ROUTED TO	CV849	14.	2.27	6.	6.	6.	3.17
HYDROGRAPH AT	SBC25	58.	1.20	7.	7.	7.	.08
2 COMBINED AT	HC398	58.	1.20	13.	13.	13.	3.25
ROUTED TO	CV850	23.	2.08	12.	12.	12.	3.25
ROUTED TO	CV851	21.	2.47	11.	11.	11.	3.25
HYDROGRAPH AT	SBC26	13.	1.25	3.	3.	3.	.11
2 COMBINED AT	HC399	25.	2.55	13.	13.	13.	3.36
ROUTED TO	CV852	23.	2.75	11.	11.	11.	3.36
ROUTED TO	CV853	23.	2.77	11.	11.	11.	3.36
HYDROGRAPH AT	SBC27	28.	1.17	4.	4.	4.	.16

2 COMBINED AT	HC400	29.	2.62	15.	15.	15.	3.52
ROUTED TO	CV854	25.	3.35	10.	10.	10.	3.52
ROUTED TO	CV855	25.	3.42	9.	9.	9.	3.52
HYDROGRAPH AT	SBC28	17.	1.23	3.	3.	3.	.17
2 COMBINED AT	HC401	26.	3.33	12.	12.	12.	3.69
ROUTED TO	CV856	25.	3.63	10.	10.	10.	3.69
ROUTED TO	CV857	24.	3.75	9.	9.	9.	3.69

APPENDIX B

STORM DRAINAGE MODEL DESCRIPTION

MODEL DESCRIPTION

Executive Summary

Hansen, Allen & Luce has developed the Hydrologic Model Interface (HALHMI) Version 0.9 for storm drainage master planning. HALHMI is part of a complete GIS-water modeling solution for engineers called Water Suite. Water Suite also includes wastewater collection models (HALWCMI), and water supply models (HALPNI). HALWCMI and HAL-PNI are currently under development.

HALHMI v. 0.9 is being used in the current Murray City storm drainage master plan to create a hydrologic model of the City. HALHMI allows graphical manipulation of the storm drainage model with an interface to the U.S. Army Corps of Engineers' HEC-1 analysis code. HALHMI v. 0.9 has the following features:

- Create and edit storm drain pipes, ditches, detention basins, confluences, diversions, and subbasins within the model.
- Interface to an industry standard hydrologic model, HEC-1. HEC-1 is produced by the U.S. Army Corps of Engineers – Hydrologic Engineering Center and has long been an industry standard in hydrologic models.
- Graphically display pipes and ditches where the estimated flow exceeds capacity.
- Allow multiple What If? Analysis. The number of different scenarios is limited only by available system memory.
- All of these features are tied together in a graphical user interface (GUI) which runs seamlessly inside of ArcView 3.0.
- Graphical data are in ArcView shapefile format. Attribute data are in dBASE IV format. These open data standards insure that the model data is accessible now and will be accessible in the future.

The following sections describe HALHMI v. 0.9 in more detail:

- I. ArcView Application Framework
- II. The Scene View
 - A. Menus, Tools and Buttons
 1. The *Scene* Menu
 - a. Hydrologic Modeling Tools
 - Edit Connectivity
 - Edit Attributes
 - b. Hydrologic Modeling Buttons
 - Compute Area and Length
 - Inspect Subbasins
 - Compute Capacity
 - Display Connectivity
 - HEC-1 Write
 - c. Utilities Menu

I. ArcView Application Framework

HALHMI v. 0.9 is built on the ArcView desktop GIS by ESRI. All of the native ArcView functionality is available in HALHMI. HALHMI is simply an extension of ArcView to support hydrologic modeling in an urban environment. To use HALHMI from scratch, first load the HALHMI extension from an ArcView project. Figure 1 shows a project window with one scenario loaded. The project window is the default ArcView project window with the addition of the Storm Drain document type. On the right side of the project window is the list of storm drain scenario documents. In Figure 1, CN409-9 is the only scenario in the storm drain scenario list.

There are three buttons in the project window that allow an entry point to the model functionality. The *New* button creates a new scenario without any data in it. The *Open* button opens the selected scenarios. The *Copy* button copies the selected scenario to a new scenario for What-If? analysis.

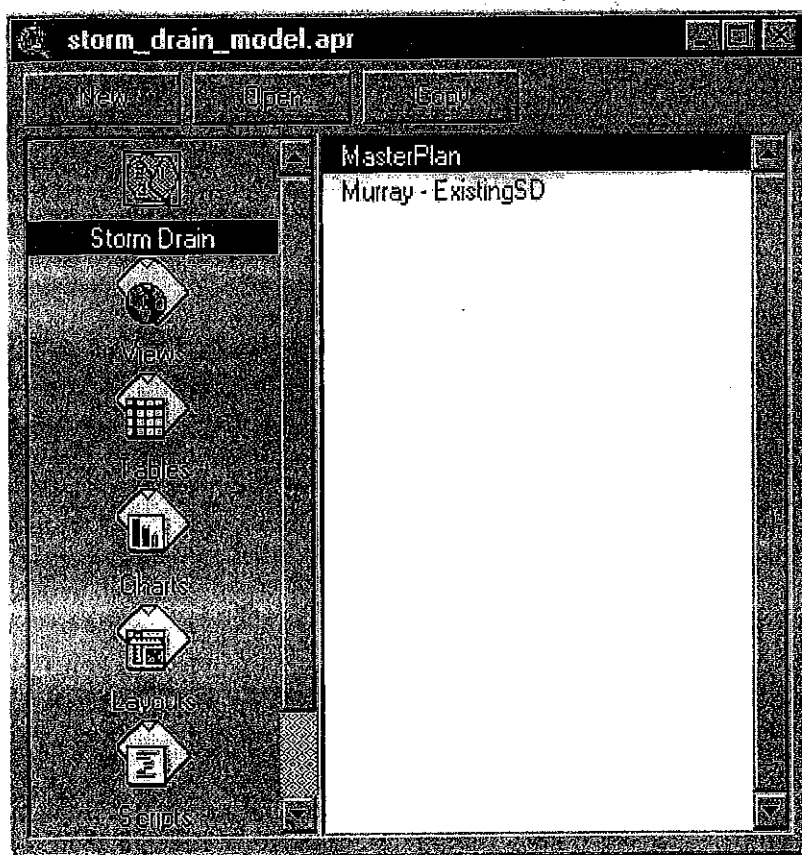


Figure 1 Project window with the HALHMI extension loaded.

II. The Scene View

A view of a typical scenario is shown in Figure 2. Any number of themes may be added to the scenario, but the following themes must exist in the scene:

- Subbasin – Polygon theme representing the discrete runoff areas in the City.
- Conveyance – PolyLine theme representing the pipes, ditches, culverts, and gutters that carry the water from the subbasins to the discharge point.
- Detention – Polygon theme representing the detention basins within the City.
- Confluence – Point theme representing the place where two or more flows come together.
- Diversion – Point theme representing the place where a single flow is divided into two flows.
- Major Basin – Polygon theme representing the boundaries of major drainage basins. The modeling software uses major basins to organize the various HEC-1 and data files within the model. Each model must have at least one major basin, and can have an unlimited number.

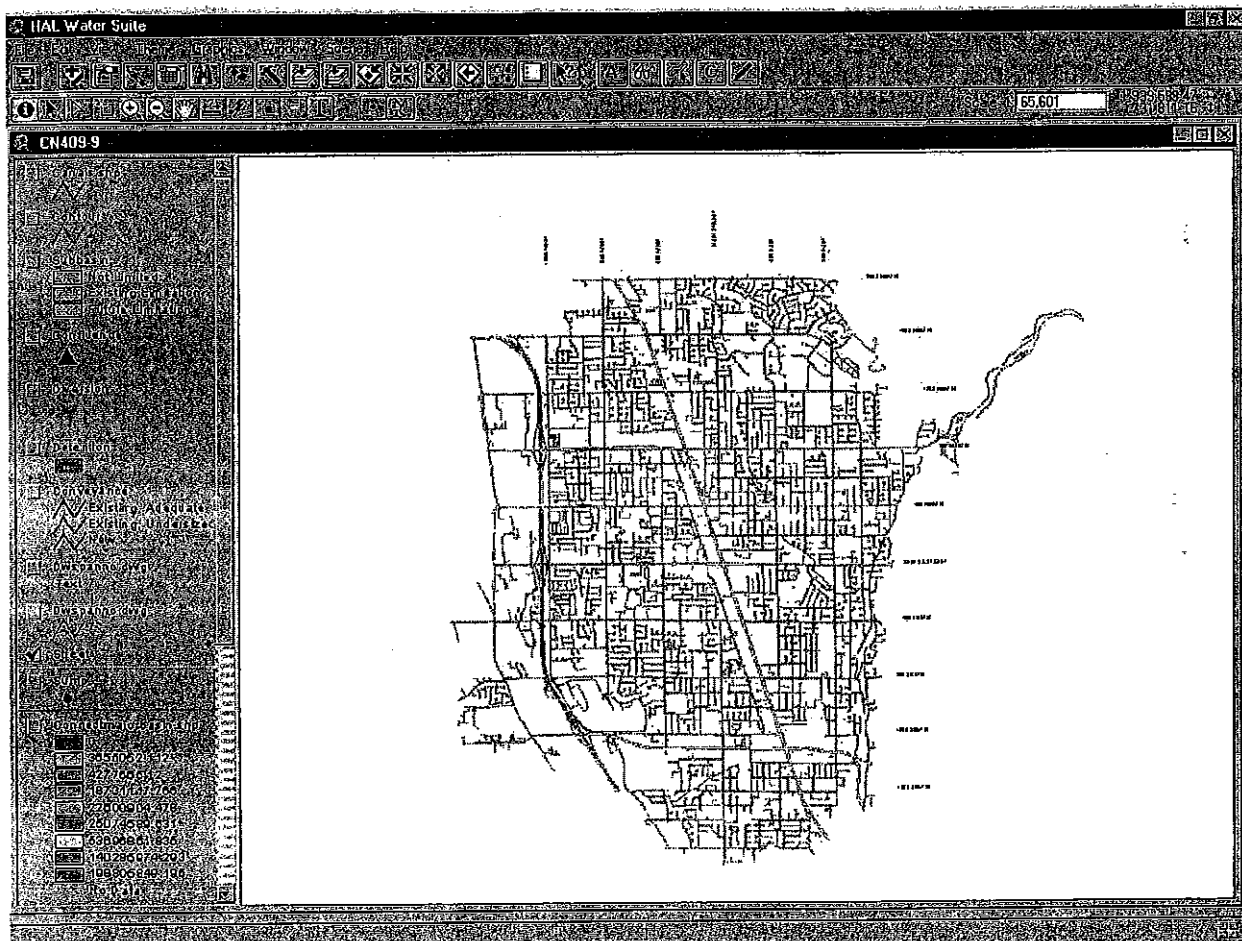


Figure 2 The Scene View

Menus, Tools and Buttons

There are several Hydrologic tools and buttons available from the Scene view.

The *Scene* Menu

The *Scene* menu, shown in Figure 3, contains the core functionality of HMI v. 0.9.

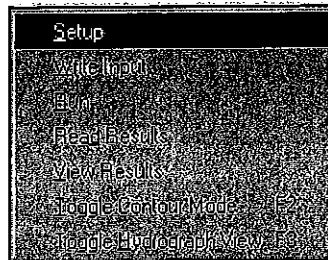




Figure 3 The *Scene* Menu

- The *Setup* option allows the user to specify the directory where the model looks for scenario data.
- *Write Input* copies and converts the GIS data to an analysis engine input data file.
- *Run* tells the analysis engine to run the model using the current input file.
- *Read Results* converts the hydrographs generated by the analysis engine to a data table compatible with ArcView and generates a summary table containing the peaks for each model entity.
- *View Results* compares the peaks for each conveyance with its capacity, marks the potential capacity problems in red, and labels each conveyance with its peak flow and capacity (Figure 4).
- *Toggle Contour Mode* looks for a theme named "Contour" and allows the user to click anywhere on the scene. The nearest contour elevation will be shown in the message bar on the lower left corner of the ArcView screen.
- *Toggle Hydrograph View* allows the user to select model entities and display the corresponding hydrographs in real time. *Shift-click* selects multiple entities to compare several hydrographs. The hydrograph view is shown in Figure 5 and Figure 6.


Hydrologic Modeling Tools


The two additional tools used for hydrologic modeling are: *Edit Connectivity* and *Edit Attributes*. The *Edit Connectivity*,  tool allows the user to select a model entity and change its order in relation to the other entities in the model. This connectivity number indicates the order in which HEC-1 will process the entity.


The *Edit Attributes*,  tool allows the user to select an entity and change its attributes. These attribute dialog boxes are illustrated in Figures 7 to 10.


Hydrologic Modeling Buttons





There are several customized buttons in the Scene view. These buttons operate on the selected features of the target themes. Compute Area and Length, , computes the area, perimeter, and length of the selected features of the active themes. If the theme is a line or polyline theme, invoking the button computes the length of each selected feature. If the theme is a polygon theme, then the button computes the area and perimeter.


Inspect Subbasins, , allows the user to inspect each selected subbasin to modify the values that went into computing the subbasin attributes. This button presents the user with a series of dialog boxes where the user can enter area and length information about individual subbasins.

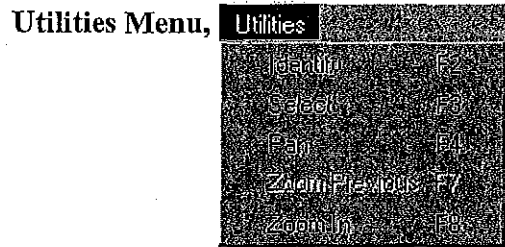
Compute Capacity, , computes the capacity of the selected conveyances based on the Mannings equation at full flow.

Resize pipes, , resizes the pipes to have sufficient capacity for the peak. The user is prompted for a Mannings N value for the new pipes and to select the summary files from disk that contain the peak flows. If desired, the program will select the appropriate standard pipe sizes and generate a cost estimate from a standard pipe size and cost table. The resulting pipe diameter is stored in the NewDim1 attribute of the Conveyance table.

Display Connectivity, , labels each model entity with its connectivity number.

HEC-1 Write, , is a shortcut to the menu item, *Write Input*.

Inspect feature, , zooms into each selected feature of the active theme. This is useful when several features of a theme require close inspection.



The Utilities menu contains several keyboard shortcuts for doing common tool tasks such as:

- Identify - F2
- Select - F3
- Pan - F4
- Zoom Previous - F7
- Zoom In - F8

Each of these menu items selects the requested tool, and keeps track of the previously selected tool. For example, if the label tool were selected, and the user pressed F2, the identify tool would be selected. If the user pressed F2 again, the label tool would be selected.

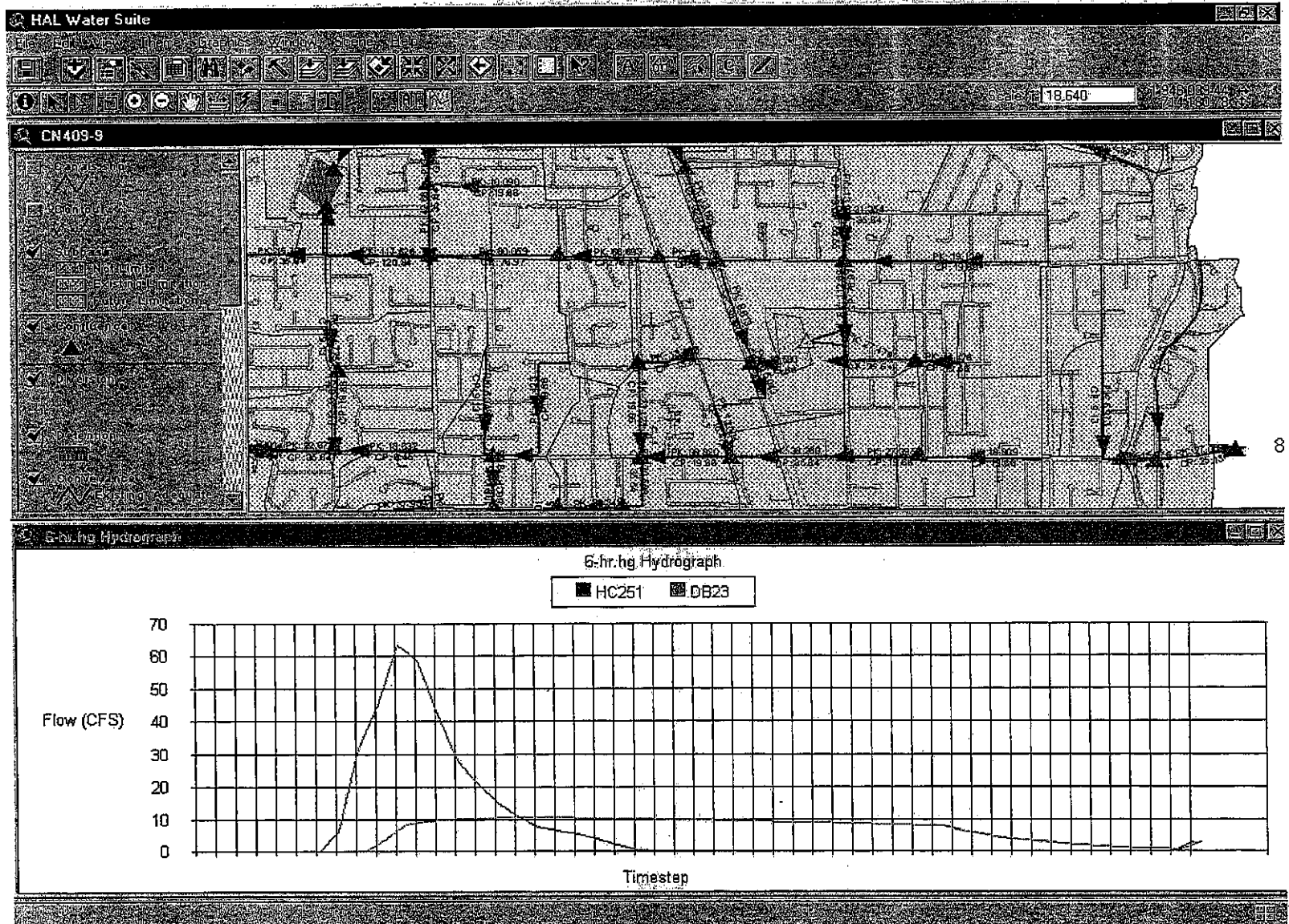


Figure 14 Hydrograph view showing inflow to-a detention basin (HC251) and outflow from the same detention basin (DB23).

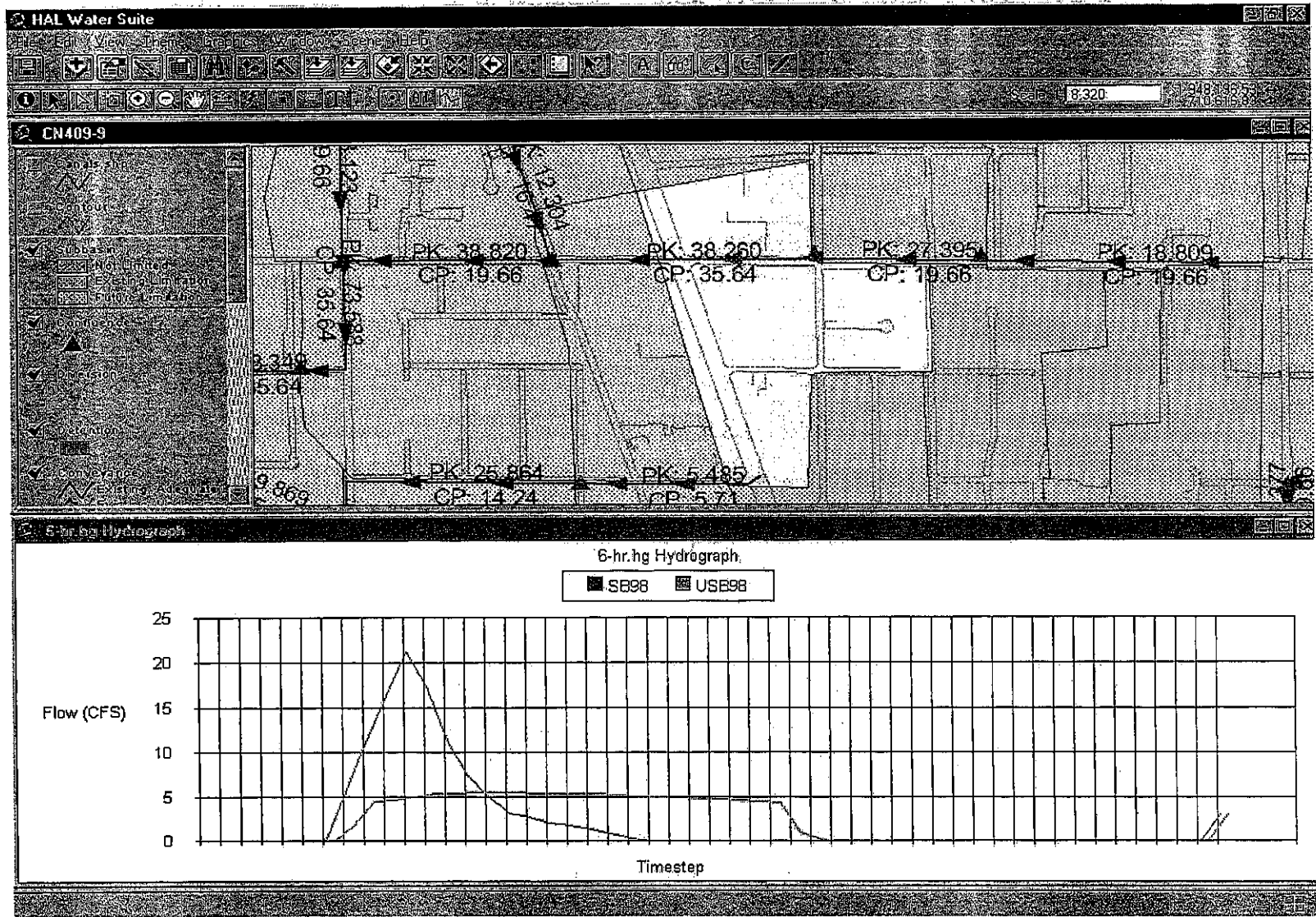


Figure 15 Hydrograph view showing the effect of on-site detention on the runoff from a controlled subbasin. SB98 is the uncontrolled runoff and USG98 is the runoff with on-site detention.

Edit Sub-Basin Attributes

Sub-Basin ID: SB98

Description:

Use Input (overrides computer generated values)

Area: 12584 Acres

Impervious Area		Pervious Area	
Curve Number	98	Curve Number	59.095
Total Area	69.18	Total Area	30.82
Overland Flow Length (ft)	35	Overland Flow Length (ft)	100
Representative Slope (ft/ft)	0.02	Representative Slope (ft/ft)	0.02
Roughness Coefficient	0.1	Roughness Coefficient	.4

Sub-Basin Routing

Enable Secondary Channel

Secondary Channel		Main Channel	
Length (ft)	<input type="text"/>	Length (ft)	1800
Slope	<input type="text"/>	Slope	.01
Roughness	<input type="text"/>	Roughness	0.025
Conduit Slope (ft/ft)	<input type="text"/>	Shape	TRAP
Shape	<input type="text"/>	Bottom Width (ft)	2
Channelment	<input type="text"/>	Side Slope	25
Side Slope	<input type="text"/>		

OK Cancel Help

Figure 16 Edit Subbasin attributes dialog box.

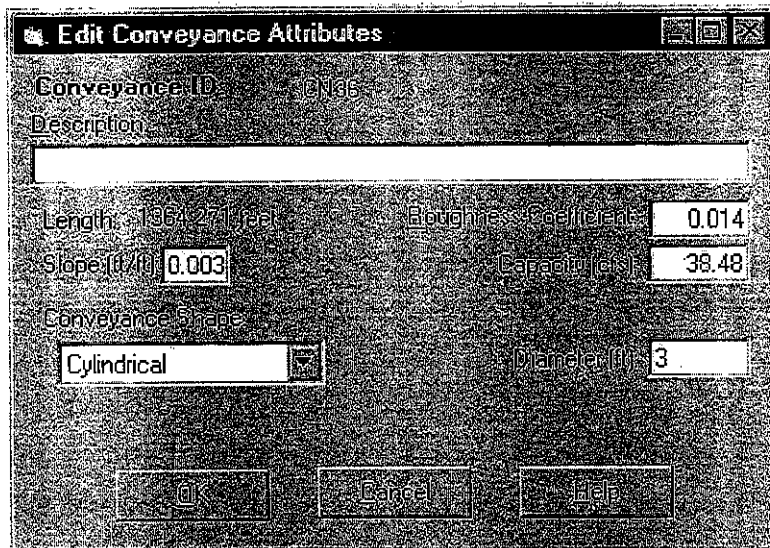


Figure 17 Edit Conveyance attributes dialog box.

Detention Basin Attributes

Detention Basin ID: DB23

Description: Proposed Detention Basin - Scera Park (7 acres)

Initial Condition Type: STOR Initial Condition Value: 0.0

Volume Data

P1	P2	P3	P4	P5	P6	P7	P8	P9	P10
0.0	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10.0	11	12	13	14					

Elevation Data

P1	P2	P3	P4	P5	P6	P7	P8	P9	P10
0.0	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10.0	11	12	13	14					

Flow Data

P1	P2	P3	P4	P5	P6	P7	P8	P9	P10
0.0	0.5	8	9	9.5	10	10.5	11	11.5	12
12.5	13	13.5	14	50					

Done Help

Figure 18 Edit Detention Basin attributes dialog box.

Confluence

Station ID: HC252

Description:

Number of Hydrographs to Combine:

3

Done Help

Figure 19 Edit Confluence attributes dialog box.

APPENDIX C

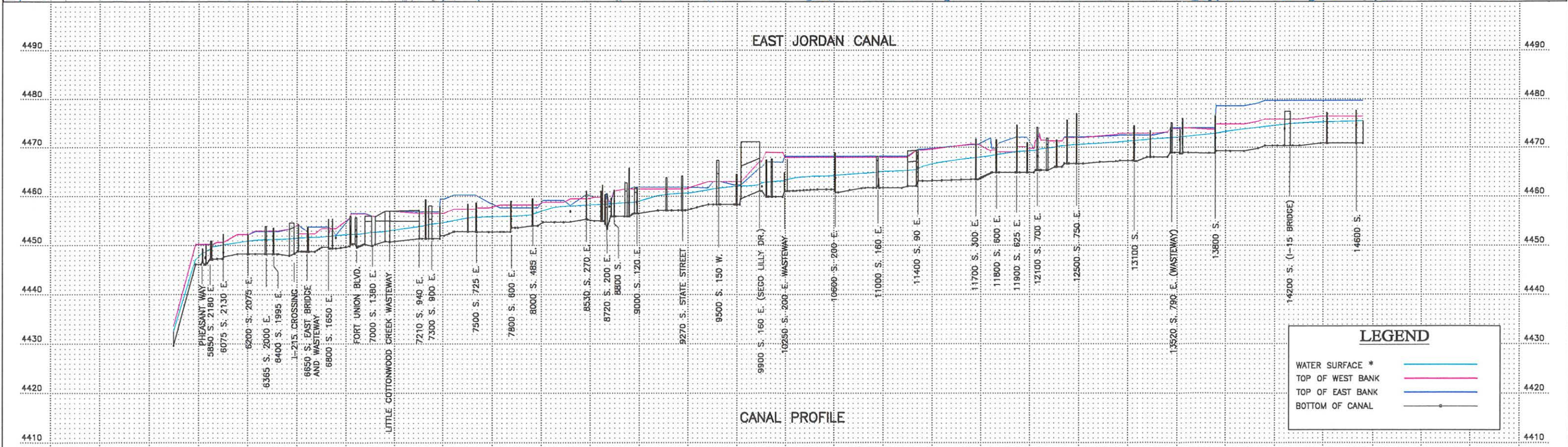
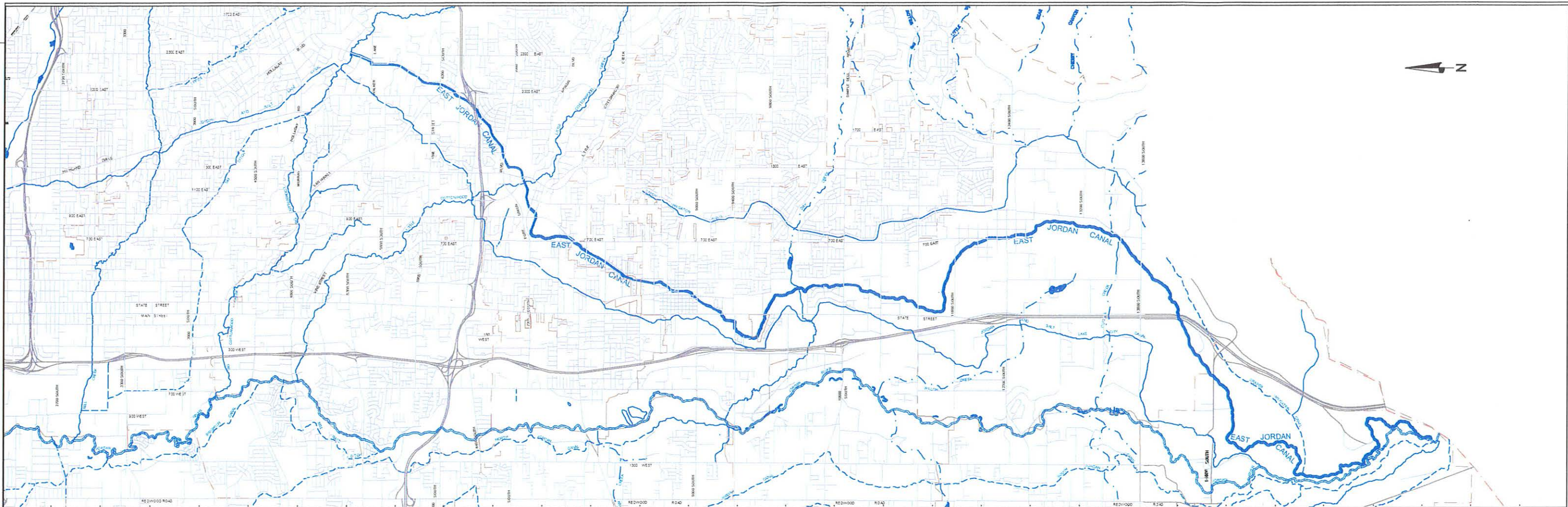
WATER SURFACE PROFILES



APPENDIX C
WATER SURFACE PROFILES

TABLE OF CONTENTS

TITLE	NUMBER OF SHEETS
East Jordan Canal Plan & Profile	1
East Jordan Canal Flow Profiles	3
Jordan & Salt Lake Canal Plan & Profile	1
Jordan & Salt Lake Canal Flow Profiles	4
Upper Canal Plan & Profile	1
Upper Canal Flow Profiles	2



* THE WATER SURFACE PROFILE ASSUMES OVERFLOW MODIFICATIONS AND SANDY CITY STORM DRAIN MASTER PLAN PROJECTS ARE COMPLETE. FLOW IS THE COMBINATION OF NORMAL IRRIGATION BASE FLOW AND A 10-YEAR, 3-HOUR STORM EVENT FLOW.

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FILE DATE: 2.3.2003 13:25:06 (GAR)



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DRAFTED	2
CHECKED GJP	1
DATE DECEMBER 2002	NO. DATE

NO.	DATE	REVISIONS	BY	APVD.

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1" = 10'

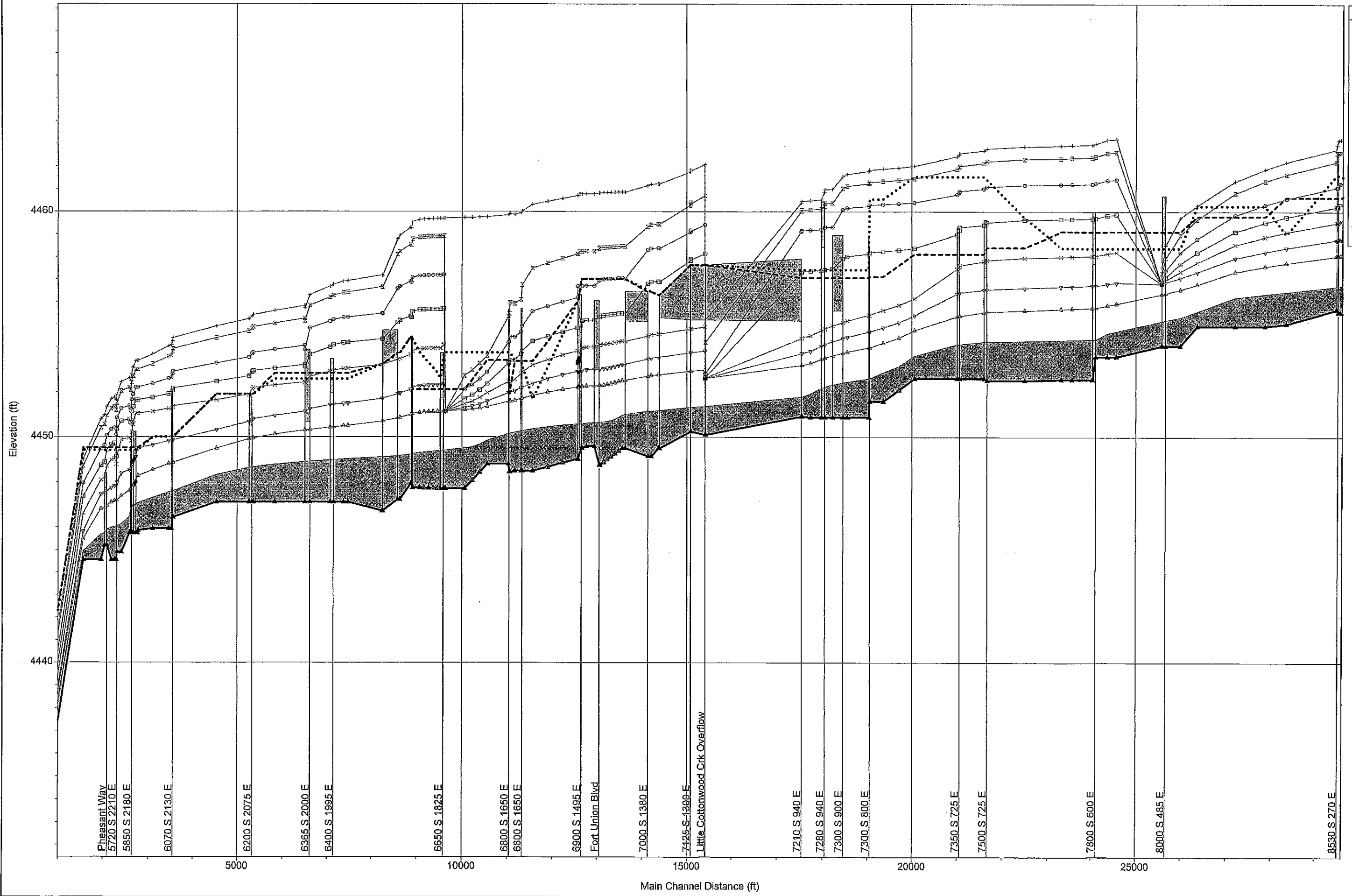
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BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY.



**EASTSIDE CANAL STUDY
EAST JORDAN CANAL
PLAN & PROFILE**

EAST JORDAN CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

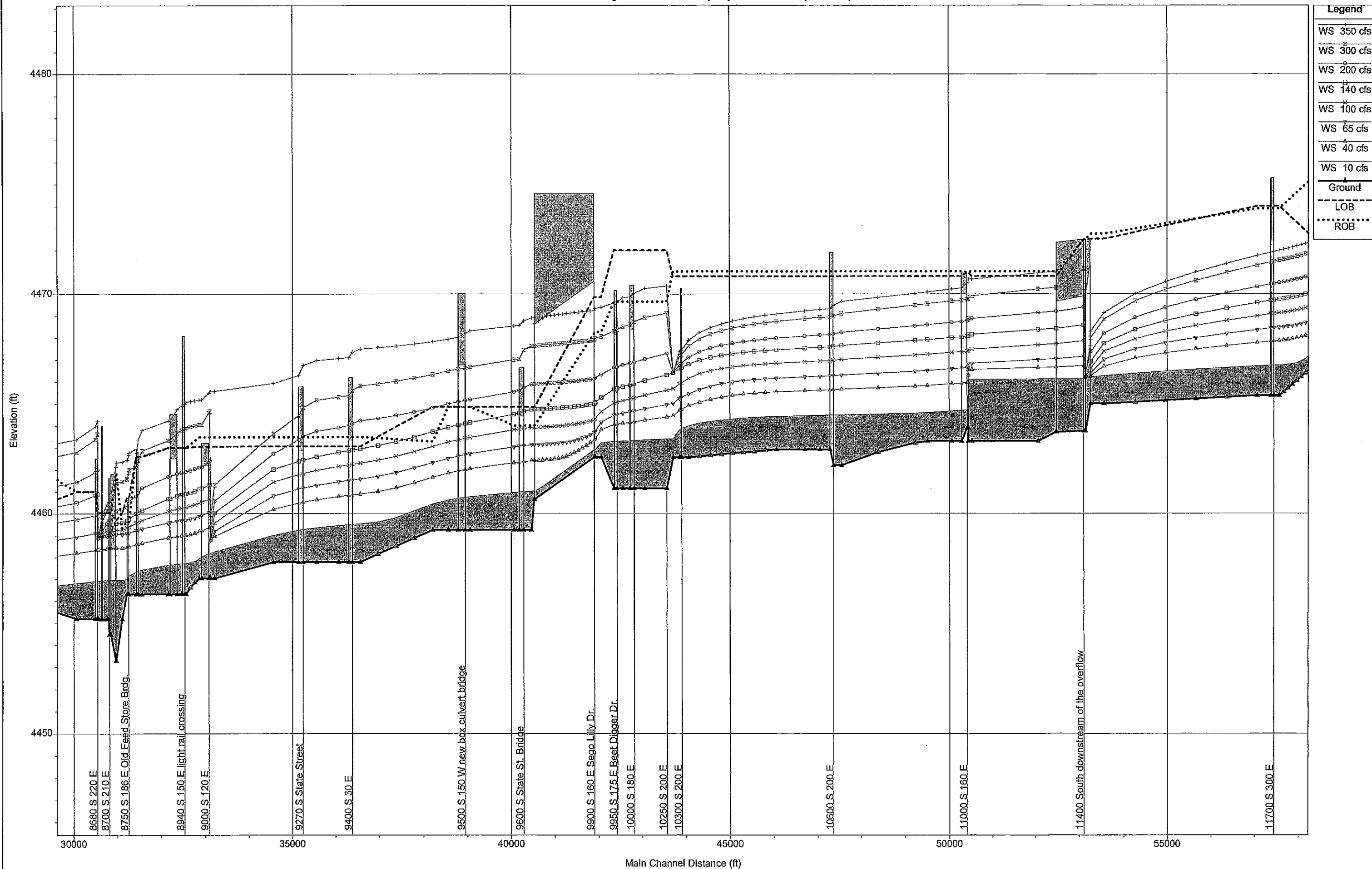


Legend	
WS 350 cfs	○
WS 300 cfs	□
WS 200 cfs	△
WS 140 cfs	▽
WS 100 cfs	×
WS 65 cfs	+
WS 40 cfs	·
WS 10 cfs	·
Ground	▲
LOB	---
ROB	···

1 in Horiz. = 2000 ft 1 in Vert. = 4 ft

EAST JORDAN CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

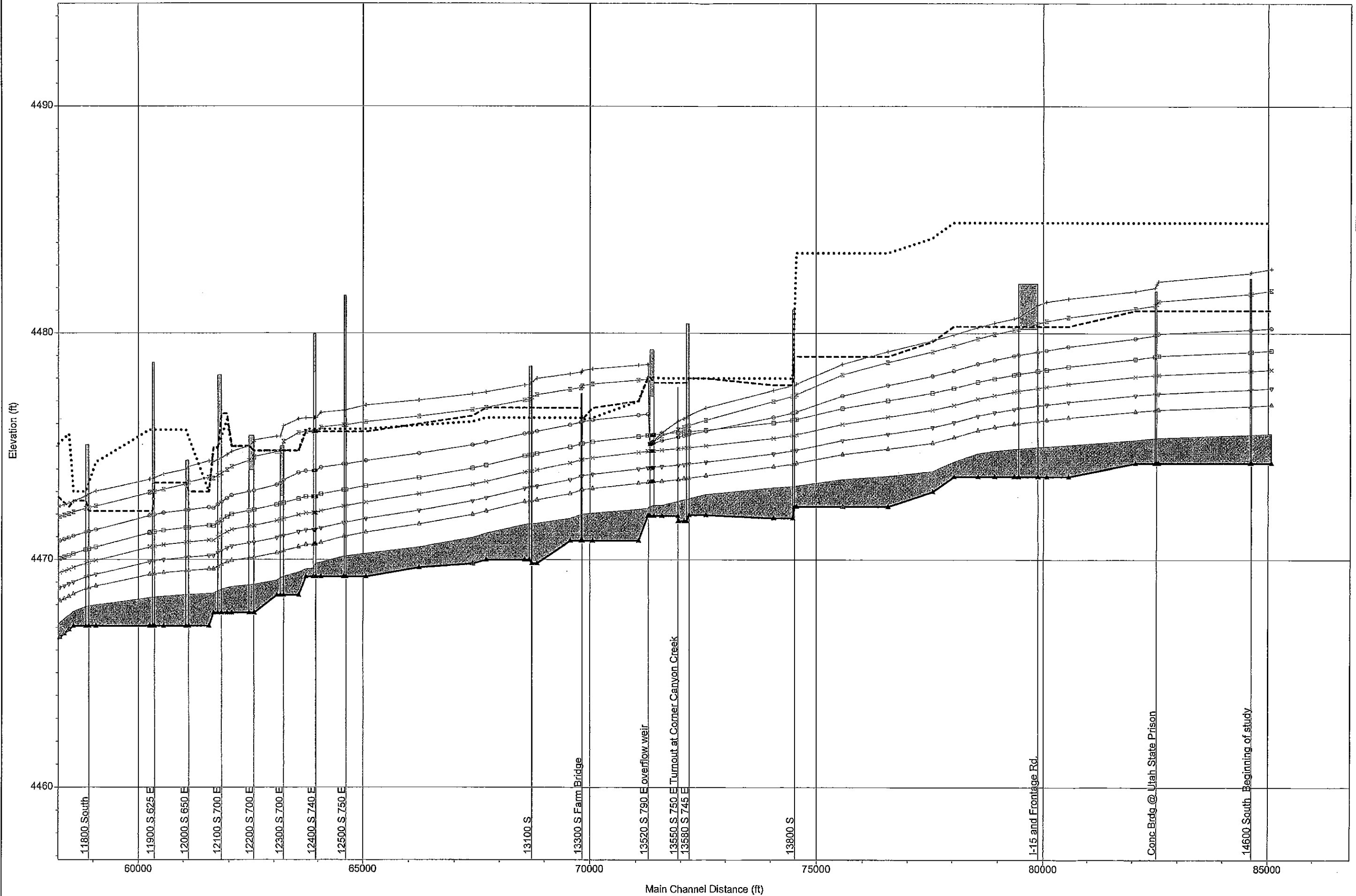


Legend	
WS 350 cfs	(Solid line with squares)
WS 300 cfs	(Dashed line with circles)
WS 200 cfs	(Dotted line with diamonds)
WS 140 cfs	(Dash-dot line with crosses)
WS 100 cfs	(Long-dash line with pluses)
WS 65 cfs	(Short-dash line with asterisks)
WS 40 cfs	(Dash-dot-dot line with triangles)
WS 10 cfs	(Solid line with inverted triangles)
Ground	(Solid line with triangles)
LOB	(Dashed line)
ROB	(Dotted line)

1 in Horiz. = 2000 ft 1 in Vert. = 4 ft

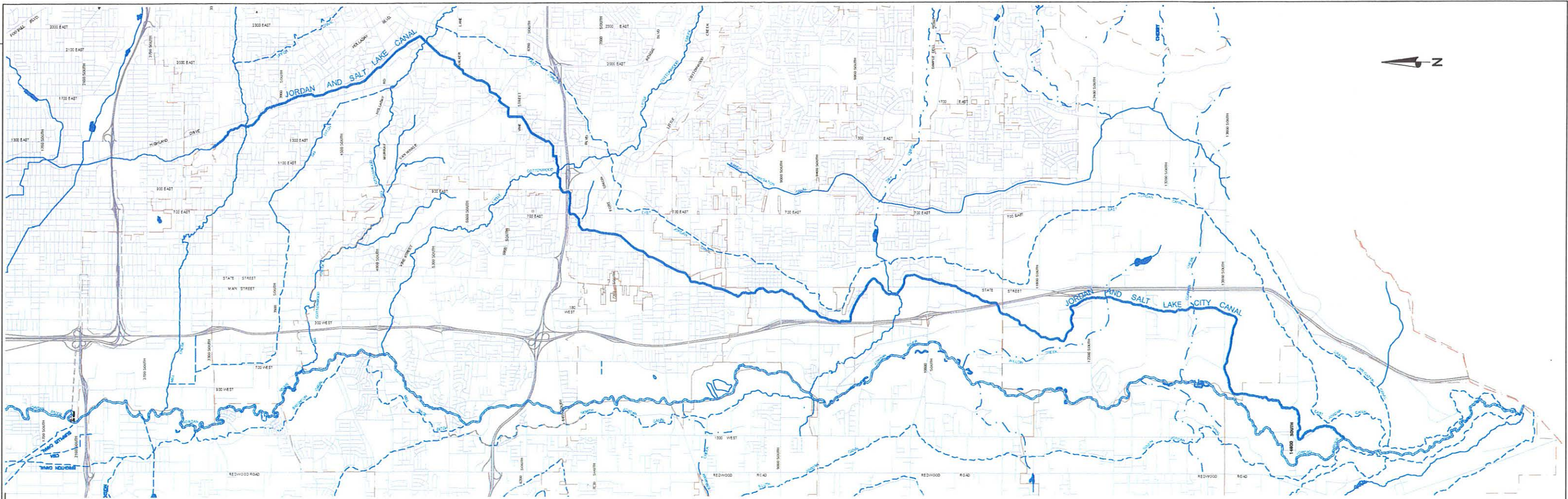
EAST JORDAN CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

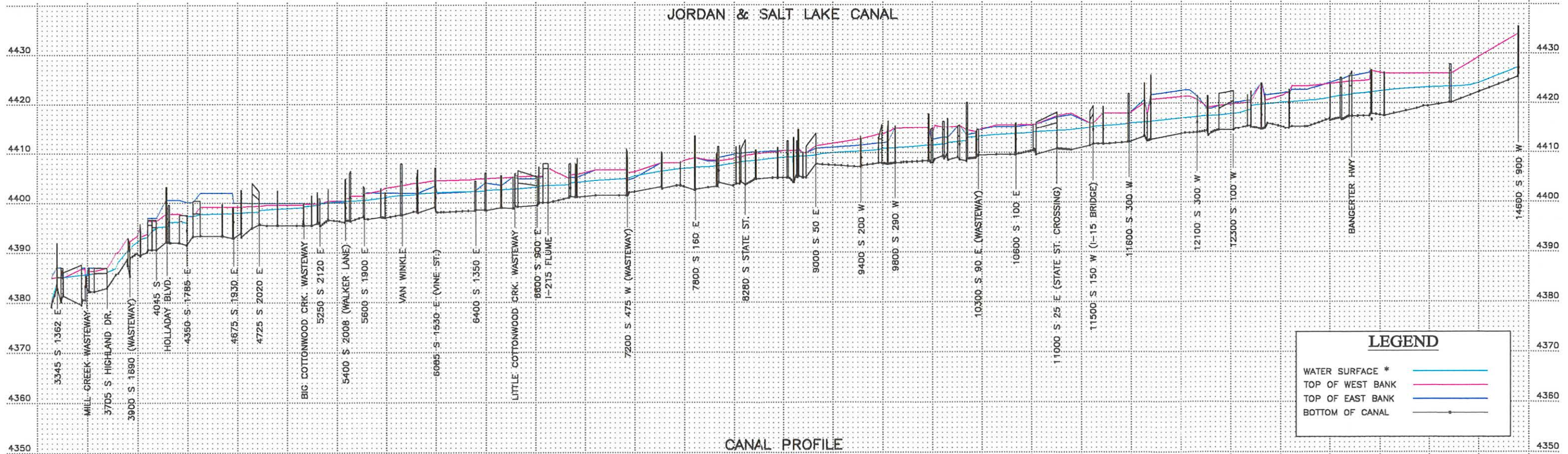


Legend	
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WS 300 cfs	(Symbol: Square)
WS 200 cfs	(Symbol: Triangle)
WS 140 cfs	(Symbol: Cross)
WS 100 cfs	(Symbol: Diamond)
WS 65 cfs	(Symbol: Inverted Triangle)
WS 40 cfs	(Symbol: Triangle)
WS 10 cfs	(Symbol: Circle)
Ground	(Symbol: Triangle)
LOB	(Symbol: Dashed Line)
ROB	(Symbol: Dotted Line)

1 in Horiz. = 2000 ft 1 in Vert. = 4 ft



JORDAN & SALT LAKE CANAL



CANAL PROFILE

LEGEND

- WATER SURFACE * —
- TOP OF WEST BANK —
- TOP OF EAST BANK —
- BOTTOM OF CANAL —

* THE WATER SURFACE PROFILE ASSUMES OVERFLOW MODIFICATIONS AND SANDY CITY STORM DRAIN MASTER PLAN PROJECTS ARE COMPLETE. FLOW IS THE COMBINATION OF NORMAL IRRIGATION BASE FLOW AND A 10-YEAR, 3-HOUR STORM EVENT FLOW.

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FILE DATE: 2.3.2002 14:12:58 (CAH)



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DRAFTED CAH	2
CHECKED GJP	1
DATE DECEMBER 2002	NO. DATE

REVISIONS

BY APVD

SCALE
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VERTICAL
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VERIFY SCALE
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IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY.

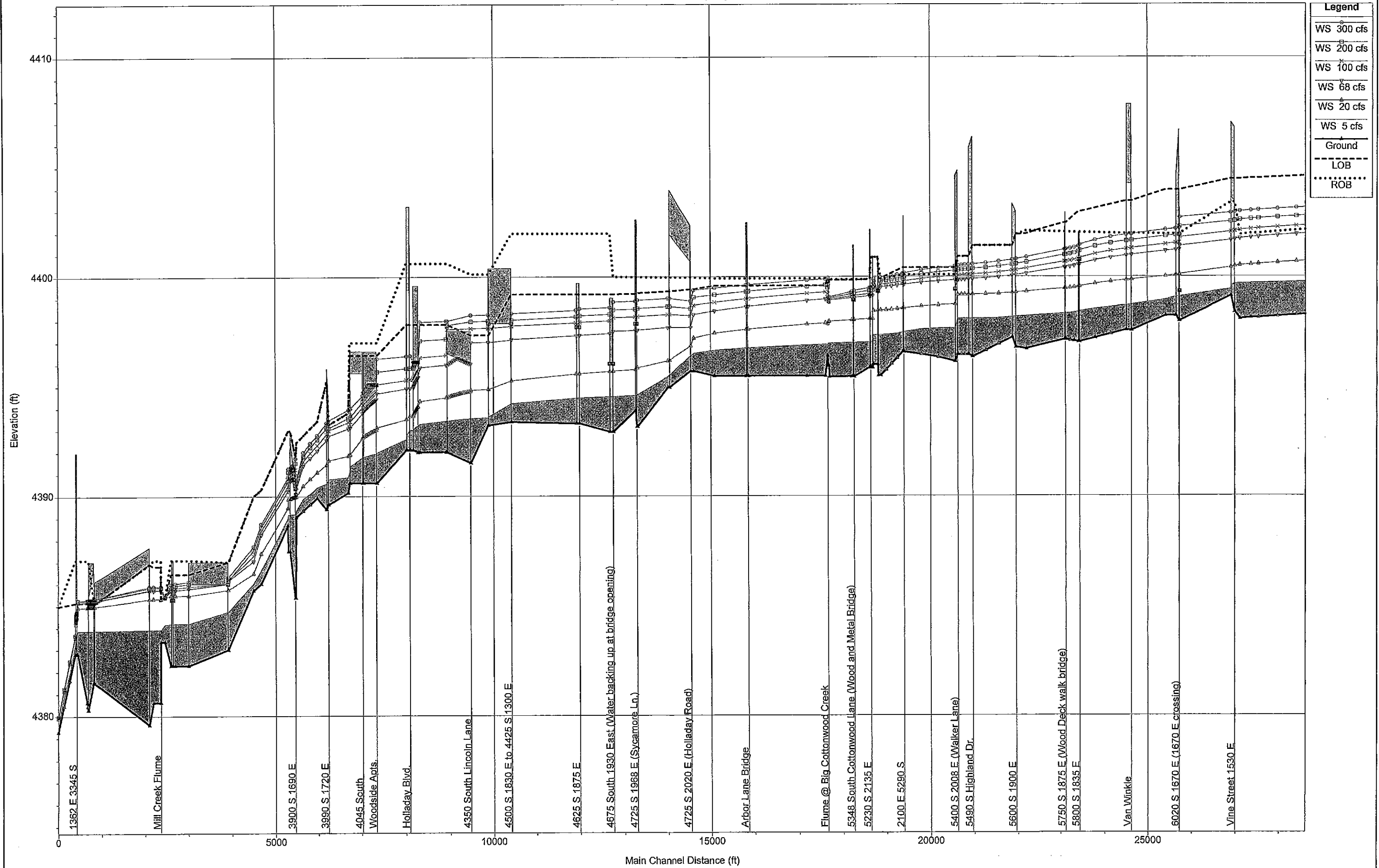


EASTSIDE CANAL STUDY
JORDAN & SALT LAKE CANAL
PLAN & PROFILE

FIGURE
C-2
14-09-100

JORDAN AND SALT LAKE CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

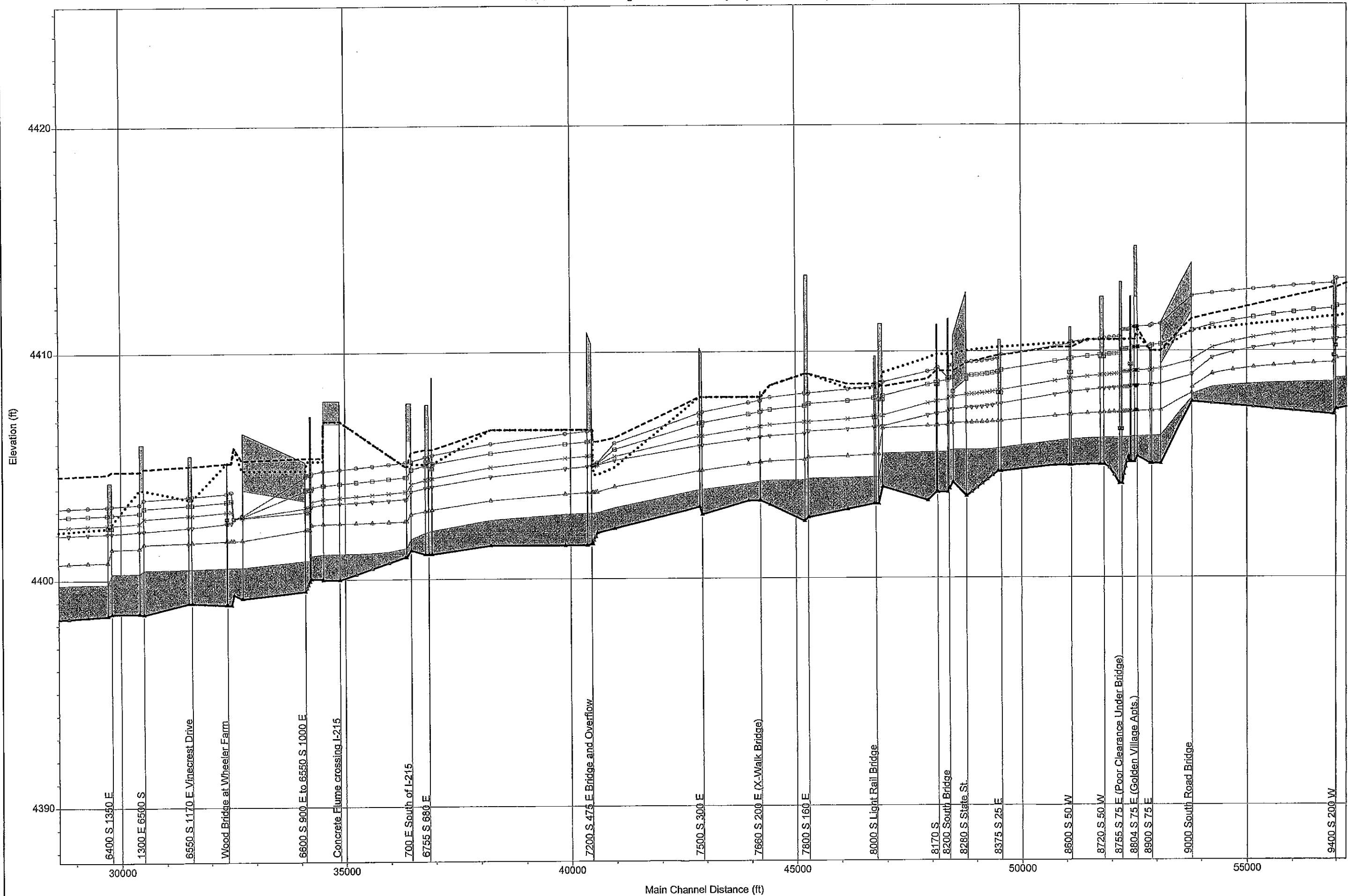


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△	WS 100 cfs
×	WS 68 cfs
*	WS 20 cfs
·	WS 5 cfs
—	Ground
- - -	LOB
· · ·	ROB

1 in Horiz. = 2000 ft 1 in Vert. = 4 ft

JORDAN AND SALT LAKE CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

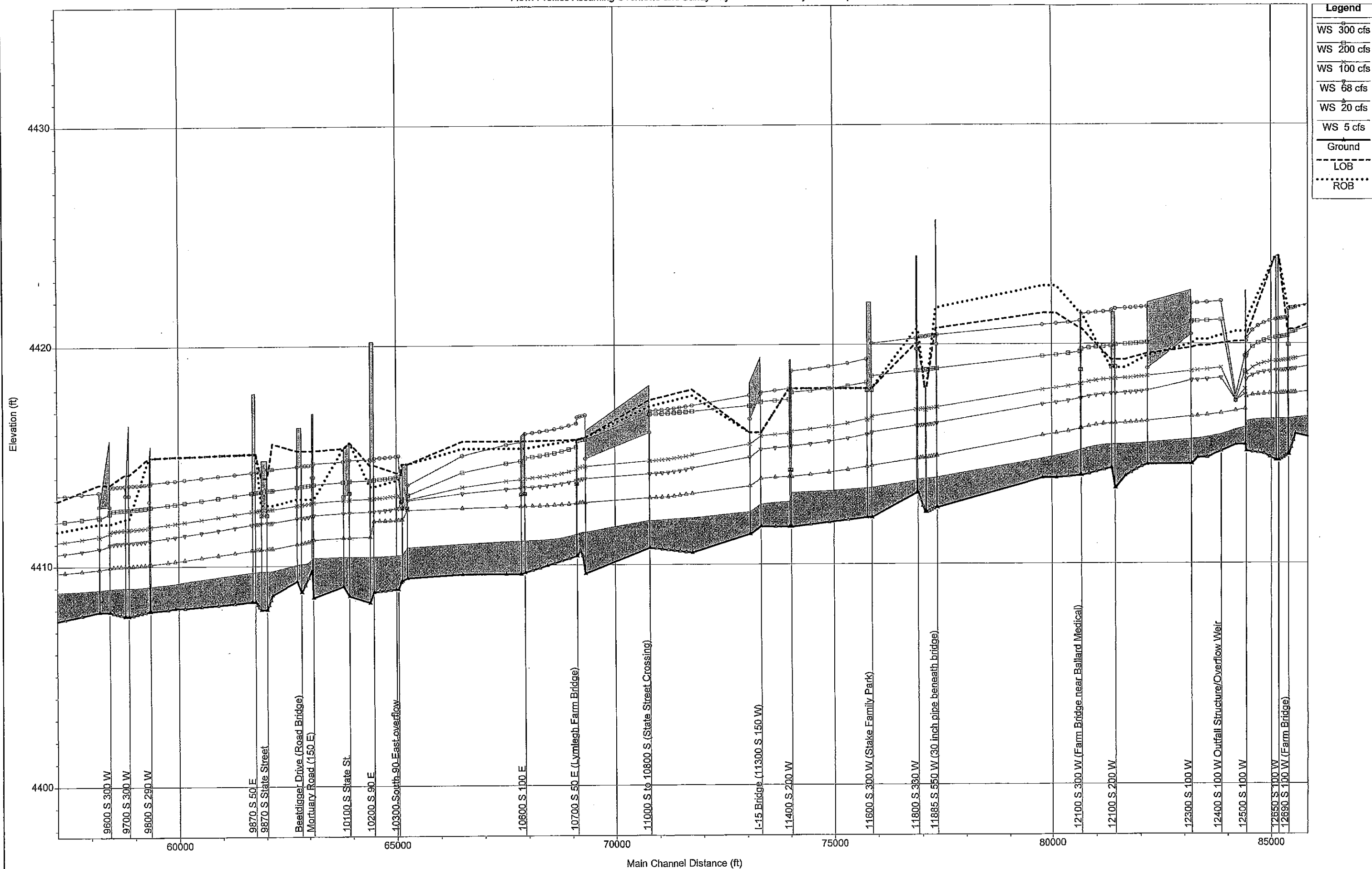


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□	WS 200 cfs
△	WS 100 cfs
▽	WS 68 cfs
▲	WS 20 cfs
▼	WS 5 cfs
—	Ground
- - -	LOB
⋯	ROB

1 in Horiz. = 2000 ft 1 in Vert. = 4 ft

JORDAN AND SALT LAKE CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete

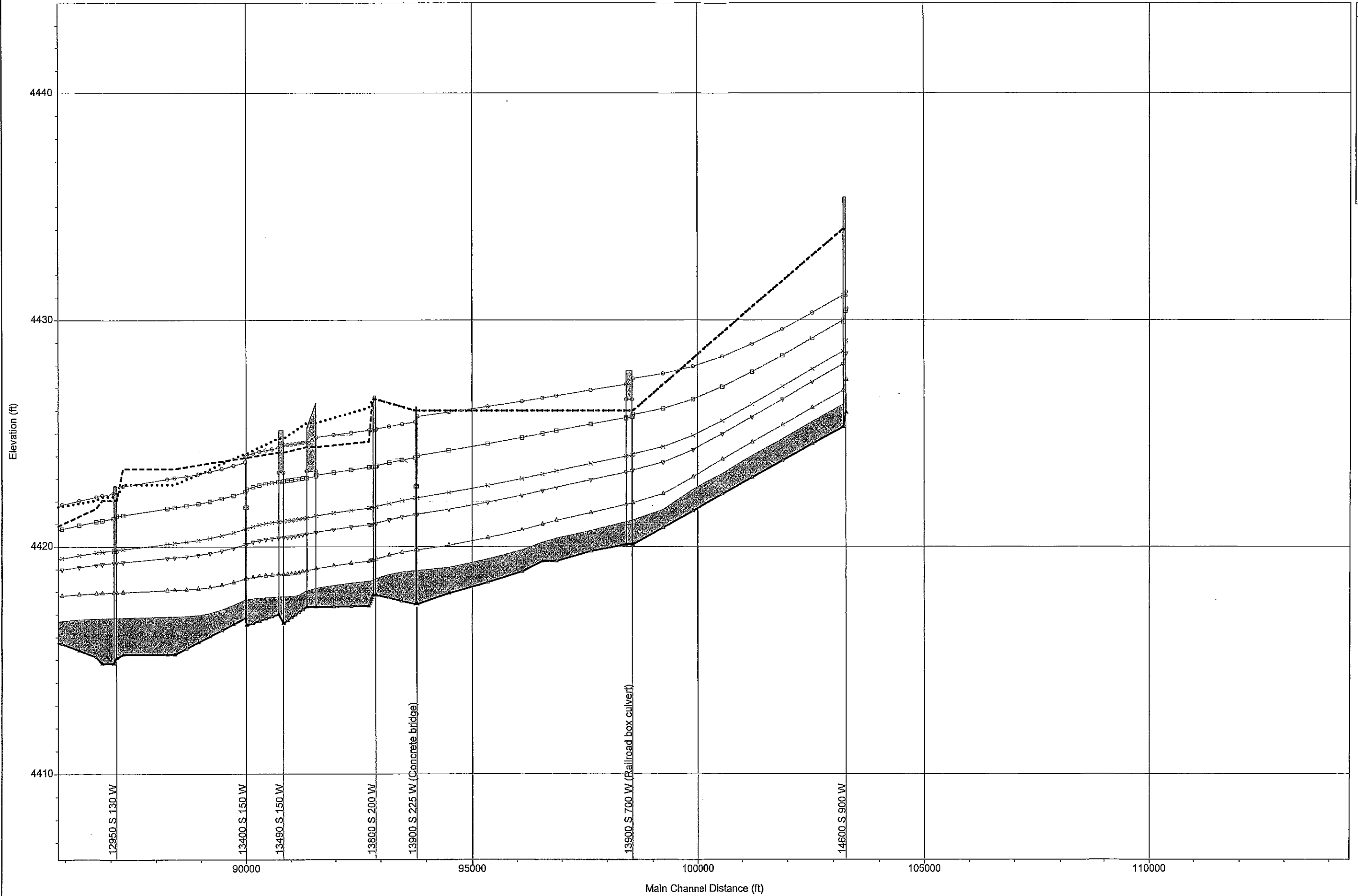


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▽	WS 100 cfs
△	WS 68 cfs
×	WS 20 cfs
•	WS 5 cfs
—	Ground
- - -	LOB
⋯	ROB

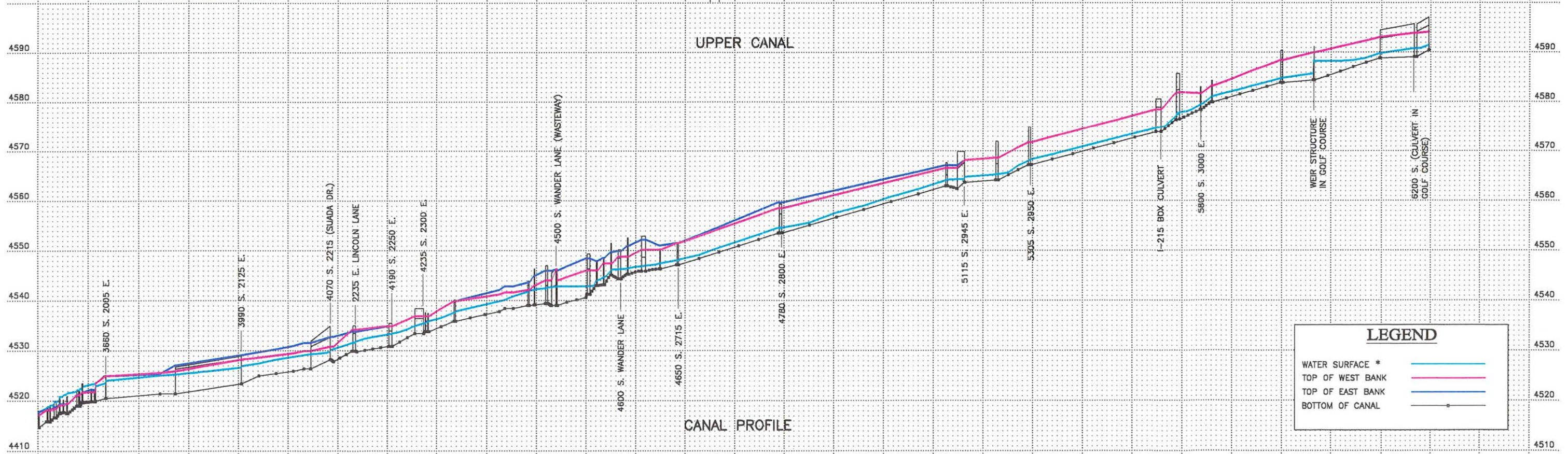
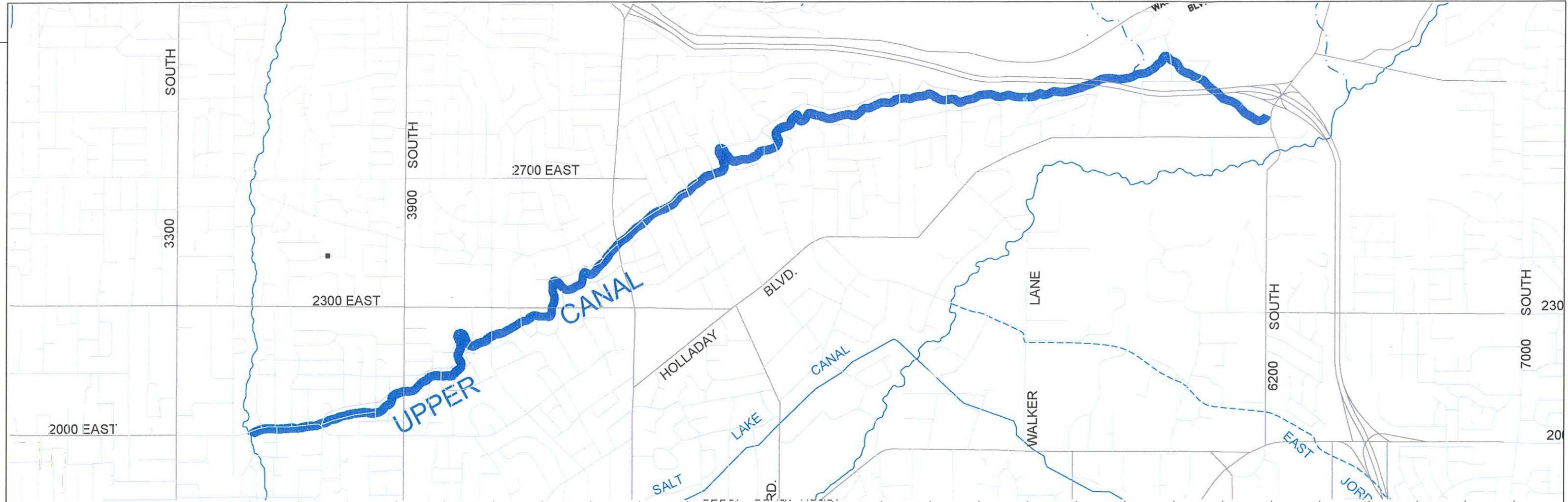
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JORDAN AND SALT LAKE CANAL

Flow: Profiles Assuming Overflows and Sandy City Master Plan Projects Complete



1 in Horiz. = 2000 ft 1 in Vert. = 4 ft



* THE WATER SURFACE PROFILE ASSUMES OVERFLOW MODIFICATIONS AND SANDY CITY STORM DRAIN MASTER PLAN PROJECTS ARE COMPLETE. FLOW IS THE COMBINATION OF NORMAL IRRIGATION BASE FLOW AND A 10-YEAR, 3-HOUR STORM EVENT FLOW.

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CHECKED	GJP	1	
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NO.	DATE	REVISIONS	BY	APVD.

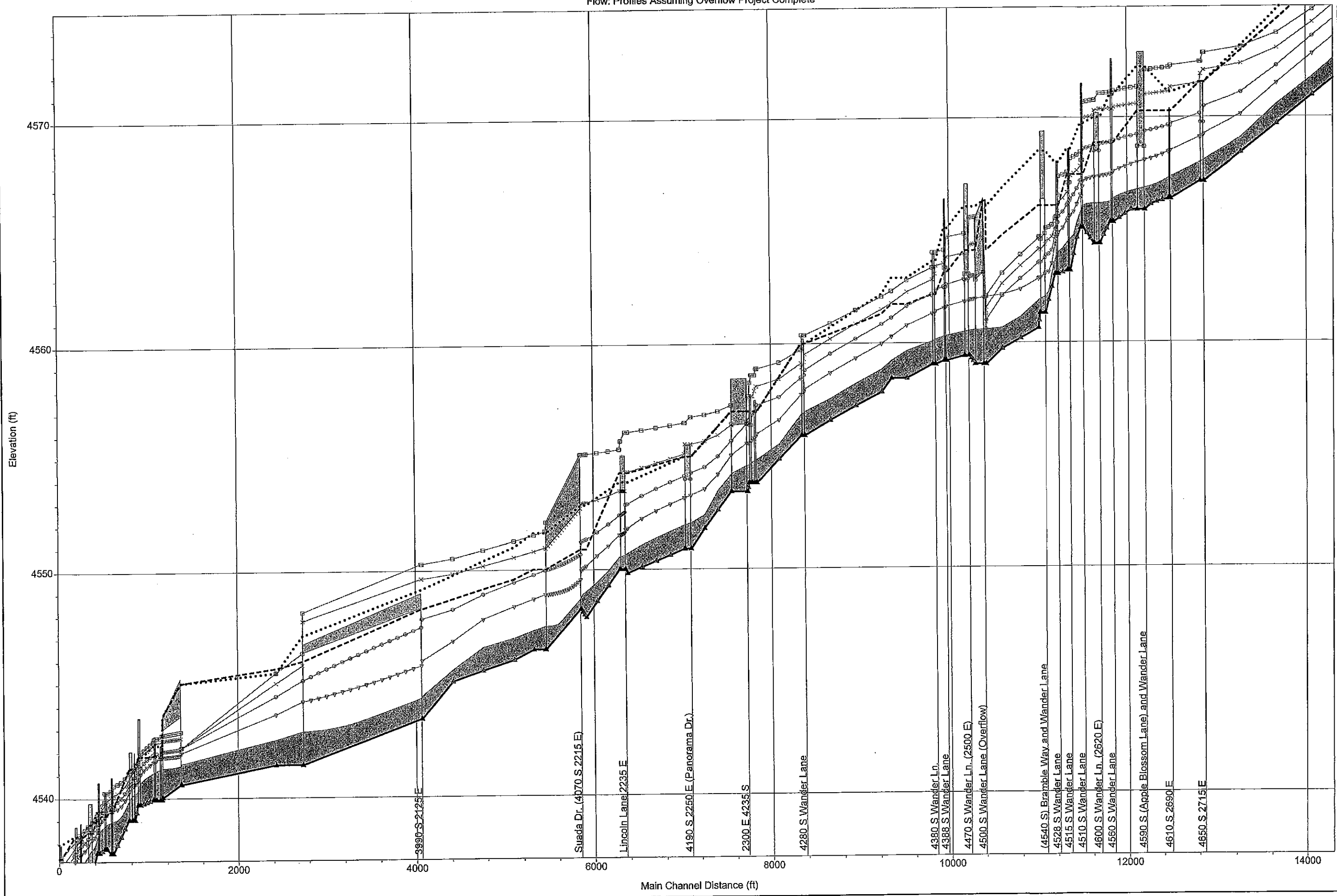
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EASTSIDE CANAL STUDY
UPPER CANAL
PLAN & PROFILE

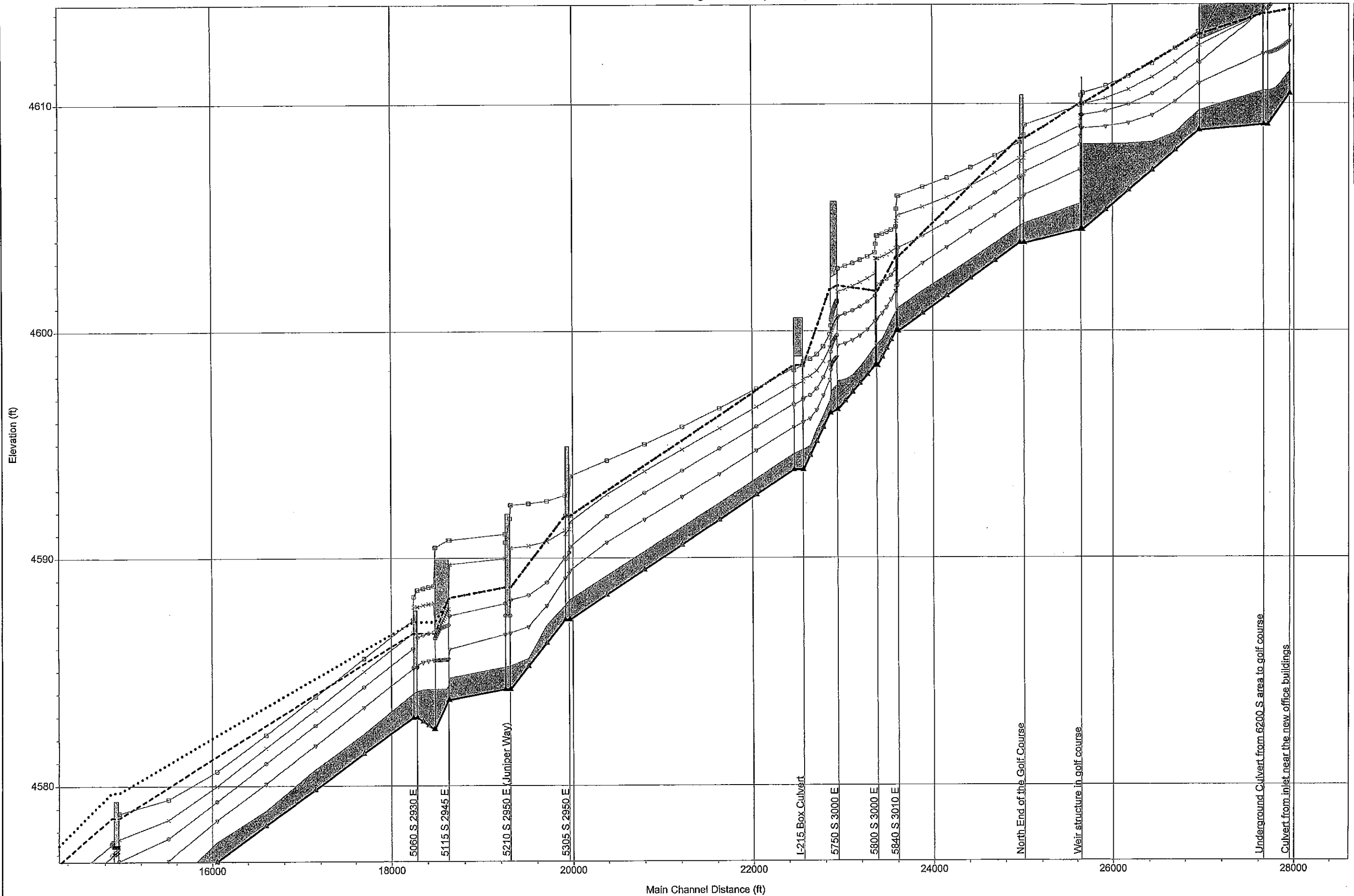
Upper Canal
 Flow: Profiles Assuming Overflow Project Complete



Legend	
□	WS 200 cfs
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△	WS 100 cfs
▽	WS 50 cfs
◇	WS 10 cfs
—▲—	Ground
- - -	LOB
.....	ROB

1 in Horiz. = 1000 ft 1 in Vert. = 4 ft

Upper Canal
Flow: Profiles Assuming Overflow Project Complete



Legend	
WS 200 cfs	□
WS 150 cfs	○
WS 100 cfs	△
WS 50 cfs	▽
WS 10 cfs	◇
Ground	—▲—
LOB	- - -
ROB	⋯

1 in Horiz. = 1000 ft 1 in Vert. = 4 ft

APPENDIX D

FIELD NOTES

Salt Lake County - Eastside Canal Study

Summary of stage measurements taken in the field on the East Jordan, Jordan and Salt Lake, and Upper canal on August 16 and 17, 2001.

Project Number: 014.09.100

East Jordan Canal

08/16/01

Identifier	Location	Time	Datum	Distance W.S to datum (ft)	Flow Depth (ft)	Comments
1	900 E	9:12 a.m.	depth indicator	NA	2.78	Flow measuring device located east of 900 E.
2	7800 S		top of concrete on bridge edge	4.3	2.3	
3	8600 S	9:40 a.m.	top of concrete on bridge edge	3.9	3	
4	9000 S		top of concrete on bridge edge	3.39	2.6	Wasteway at this location; Photo 45,44
5			Canal reach			Canal Photo 43; dense veg. above W.S.; about 1.5 ft of freeboard
6	Sego Lilly		top of concrete on bridge edge	10.9		Fast approach velocity and some debris in channel; Photo 42,41
7	Dry Creek Wasteway		Downstream concrete overflow	0.8		Photo 40,39,38
8	10600 S		top of concrete on bridge edge	6.07	3.4	Trash accumulation on center pier, old check with slots for boards, manually controlled wasteway, Photo 37,36
9	11400 S		top of concrete on bridge edge	4.28		Blow off above bridge; wide channel with slow flow; turnout both sides of bridge w/ wasteway on downstream end; Photo 35
10	700 E 12100 S		Top of rail	10.05		Pipeline crossing canal; 7.05' W.S. to top of cap; 3.62' bridge to top of rail; Photo 34
11	Slightly north of 13200 S		top of concrete on bridge edge	4.46	3.6	Flow depth near west side of the bridge; Photo 33
12	13540 S Fort St.	11:00 a.m.	top of wasteway	2.7	3.5	Wasteway to corner canyon creek; old corner canyon creek crossing; Photo 29,28
13	Stokes Ave. Bridge		top of concrete on bridge edge	3.15		1 - 1.5 ft of freeboard to bridge bottom; Photo 31,30
14	13800 S		top of concrete on bridge edge	5.51	3.5	Trees overgrowing canal upstream of bridge; Photo 25,24
15	East side of I-15		top of concrete on bridge edge	4.2		Dangerous slope into canal; downstream side of crossing; culvert into canal; Photo 23,22
16	14600 S	11:45 a.m.	top of concrete on bridge edge	7.56	3.8	Photo 21,20
17	13540 S Fort St.	1:30 p.m.	top of wasteway	2.7		Took Flow measurement at this location.

1	14600 S	3:42 p.m.	Top of bridge headwall	8.36	0.8	
2	13800 S	3:58 p.m.	Top of bridge headwall	10	1.3	
3	12650 S		Top of bridge surface	6.4	1.85	
4	Near Ballard Medical	4:30 p.m.	Small bridge, top at wood	3.7	2.8	Possible beaver dam downstream
5	11400 S		Top of rail	4.65	2.35	Bridge is flowing full. Couldn't see top of bridge
6	I-15 Crossing	4:45 p.m.	Top of cement	4.8	2.85	North barrel had needs to be cleaned out, had a flow of .8 - 1.8
7	10600 S	9:10 a.m.	Top of bridge in the middle	3.6	2.76	Very slow flow at edges because of weeds, bridge is 3.4' at west end
8	State St. (by Target)	9:30 a.m.	Top of concrete on bridge	3.91	3	Flow depth is 3' in the middle and 3.25' on north end. Upstream of bridge has thick tress along bank. Weeds and trees below flowline. Downstream of bridge is open on both sides with grassy park. A lot of pondweed in canal.
9	9400 S	9:50 a.m.	Top of concrete on bridge	3.68	1.6	Flow depth taken at deepest section toward west bank. Weeds generally above the flowline with some weeds in the flow. A few pondweeds.
10	8000 S	10:20 a.m.	Top of cement on bridge	4.78	2.6	Flow depth taken in the middle. Some seeds on band overlapping into the flow
11	7200 S	10:34 a.m.	Top of concrete on west side	5.84	2	Flow depth is 2' on westside and 2.5' in the middle. Channel is fairly wide, flow line mostly below weeds. Wasteway just before bridge.
12	700 E	10:50 a.m.	Top of concrete ledge	4.82	1.8	Flow depth taken in the middle.
13	Vine Street	11:05 a.m.	Top of concrete at west end	5.78	1.78	Flow depth was taken at the west end. Wide and slow through this section.
14	Walker Lane	11:20 a.m.	Top of concrete at west end	5.7	2.8	Flow depth was taken in the middle. Trees all around canal especially downstream.
15	4675 S	11:40 a.m.	Top of concrete	2.8	3.1	Flow depth taken in the middle. Bridge is flowing full with 1" above bottom of concrete submerged on both ends.
16	4350 S	11:54 a.m.	Top of steel grate and concrete	3	2.88	Flow depth taken in the center.
17	1300 E 3345 S	12:10 p.m.	Top of concrete	2.14	3	Water almost to top of bridge bottom. 3' in middle of flow depth. Wide channel on approach with thick vegetation on sides.

Upper Canal

08/17/01

1	2000 E 3660 S	1:00 p.m.	Top of concrete on main culvert	3.94	1.35	Flow depth taken at center of main culvert. Canal is very low. Canal bottom is free of weeds, wasteway at this location also. After this point, the canal parallels 2000 E is trapezoidal, lined, and crosses under all driveways.
2	Lincoln Lane	1:30 p.m.	East Side Wall, top of concrete	2.7	1.08	Flow depth taken in the middle. Slow flow clean lower banker above flowline higher banks have vegetation
3	2620 E 4600 S	1:45 p.m.	Top of concrete	5.06	0.74	Flow depth taken in the middle, very dense vegetation on banks
4	Casto Ln. 5060 S	2:00 p.m.	Top of concrete	3.44	1.12	Flow depth taken in the middle, flow is generally below vegetation on banks.
5	East of I-215		Top of concrete	3.46	0.9	Depth taken in middle, steep slop through this area with fast flow. A lot of rocks along channel bottom

East Jordan Canal Extension

08/17/01

1	Fardown Ave. & Shangri	2:50 p.m.	Top of wood bridge	2.76	0.9	Depth taken in the middle, 1.98' top of wood bridge to top of cement rail/side.
2	South Side of I-215 W	3:10 p.m.	Top of concrete on bridge	2.98	2.9	Wasteway/turnout with check slots in main channel
3	Fort Union & Park Center		Top of wall on west side	3.76	2.6	Flow depth on west side, main flow comes in along west side, dense willows along banks generally above flow.

APPENDIX E

ESTIMATED CONSTRUCTION COSTS

**Salt Lake County Eastside Canal Study
Eastside Canal Improvement Projects
Conceptual Cost Estimate and Comparison**

1. East Jordan Canal 11900 South 625 East
Build Up Canal Bank

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Clear and grub brush from banks.	0.3	Acre	\$5,000	\$1,286
2	Strip, stockpile, and spread soil from existing bank.	519	C.Y.	\$8.00	\$4,148
2	Raise west bank 1 foot.	1037	C.Y.	\$8.85	\$9,178
3	Revegetate bank.	28	M.S.F.	\$21.50	\$602
Estimated Construction Cost					\$15,214
Contingency (20%)					\$4,564
Subtotal					\$19,778
Design, Engineering, and Construction Management (15%)					\$2,967
Total Cost Estimate					\$22,744

2. East Jordan Canal 8750 South - 8720 South
Build Up Canal Bank

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Strip, stockpile, and spread soil from existing bank.	93	C.Y.	\$8.00	\$741
2	Raise east bank to west bank elevation.	185	C.Y.	\$8.85	\$1,639
3	Revegetate bank.	5	M.S.F.	\$21.50	\$108
Estimated Construction Cost					\$2,487
Contingency (20%)					\$746
Subtotal					\$3,233
Design, Engineering, and Construction Management (15%)					\$485
Total Cost Estimate					\$3,718

3. East Jordan Canal 6800 South
Build Up Canal Bank

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Strip, stockpile, and spread soil from existing bank.	333	C.Y.	\$8.00	\$2,667
2	Raise east bank to west bank elevation.	1,133	C.Y.	\$8.85	\$10,030
3	Revegetate bank.	18	M.S.F.	\$21.50	\$387
4	Remove and reset chain link fence.	575	L.F.	\$16.00	\$9,200
Estimated Construction Cost					\$22,284
Contingency (20%)					\$6,685
Subtotal					\$28,969
Design, Engineering, and Construction Management (15%)					\$4,345
Total Cost Estimate					\$33,314

4. East Jordan Canal 6070 South - 5850 South

Alternative 1 (Build Up Canal Bank)

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Clear and grub brush from banks.	1	Acre	\$2,975	\$2,185
2	Construct a temporary access road.	1280	L.F.	\$3.90	\$4,992
3	Strip, stockpile, and spread soil from existing bank.	1481	C.Y.	\$8.00	\$11,852
4	Raise east and west banks 1 ft.	2963	C.Y.	\$8.85	\$26,222
5	Revegetate bank.	80	M.S.F.	\$21.50	\$1,720
	Estimated Construction Cost				\$44,786
	Contingency (20%)				\$13,436
	Subtotal				\$58,222
	Design, Engineering, and Construction Management (15%)				\$8,733
	Total Cost Estimate				\$66,955

Alternative 2 (Replace Culvert at 5850 South)

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Demolition and removal of existing bridge, culvert, and roadway	1	L.S.	NA	\$5,000
2	8x5 box culvert.	52	L.F.	\$300.00	\$15,600
3	Headwall to culvert.	2	Ea.	\$5,750.00	\$11,500
4	Repair roadway.	9	S.Y.	\$15.00	\$133
	Estimated Construction Cost				\$32,233
	Contingency (20%)				\$6,447
	Subtotal				\$38,680
	Design, Engineering, and Construction Management (15%)				\$5,802
	Total Cost Estimate				\$44,482

Jordan and Salt Lake Canal 5250 South
Alternative 1 (Build Up East Canal Bank and Add Curb Along Road on West)

Item	Description	Quantity	Unit	Engineers Cost Estimate	
				Unit Cost	Amount
1	Strip, stockpile, and spread soil from existing bank.	204	C.Y.	\$8.00	\$1,630
2	Raise east bank 1 foot.	204	C.Y.	\$8.85	\$1,803
3	Revegetate bank.	11	M.S.F.	\$21.50	\$237
4	Install a roadside curb along west bank.	550	L.F.	\$2.20	\$1,210
	Estimated Construction Cost				\$4,879
	Contingency (20%)				\$976
	Subtotal				\$5,855
	Design, Engineering, and Construction Management (15%)				\$878
	Total Cost Estimate				\$6,733

Note: M.S.F. = thousand square feet.